

# WUSD RIVERBANK ES

## ESSR III

### 1100 CARRIE STREET

### WEST SACRAMENTO, CA 95605

## WASHINGTON UNIFIED SCHOOL DISTRICT

DSA File No. 57-31  
App. No. 02-122273  
PTN. 72694-123

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### DSA REQUIREMENTS

- ALL WORK SHALL CONFORM TO THE 2022 EDITION OF THE TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR).
- AS A FACILITY WHICH COMES UNDER THE APPROVAL AND AUTHORITY OF THE DIVISION OF THE STATE ARCHITECT (DSA), THIS PROJECT IS SUBJECT TO DRAWING AND JOB SITE REVIEW BY A REPRESENTATIVE OF DSA. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS AFFECTING FLS, SSS, AND/OR ACS SHALL BE MADE BY ADDENDA OR A CHANGE ORDER APPROVED BY THE DIVISION OF THE STATE ARCHITECT, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR AND DSA IR A-6.
- A DSA CERTIFIED PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY THE DIVISION OF THE STATE ARCHITECT SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR. GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS SHALL COMPLY WITH ALL LOCAL ORDINANCES.
- A COPY OF PART 1 TO PART 5 OF TITLE 24 SHALL BE KEPT AND BE AVAILABLE IN THE FIELD DURING CONSTRUCTION.
- DSA SHALL BE NOTIFIED OF THE START OF CONSTRUCTION AND PRIOR TO THE PLACEMENT OF CONCRETE PER SECTION 4-331, PART 1, TITLE 24, CCR. THE DIVISION OF THE STATE ARCHITECT IS EXEMPT FROM ARBITRATION OR MEDIATION PROCEDURES.
- SUPERVISION BY THE DIVISION OF THE STATE ARCHITECT IS PER SECTION 4-334, PART 1, TITLE 24, CCR.
- ADMINISTRATION OF CONSTRUCTION PER PART 1, TITLE 24, CCR:
  - VERIFIED REPORTS PER SECT 4-336, PART 1, TITLE 24 CCR
  - DUTIES OF ARCHITECT PER SECT 4-331, 4-341; PART 1, TITLE 24 CCR
  - DUTIES OF CONTRACTOR PER SECT 4-343, PART 1, TITLE 24 CCR
- TESTING AND INSPECTION:
  - INSPECTOR APPROVED BY DSA AS PER SECT. 4-333(D); PART 1, TITLE 24, CCR
  - TESTS AND TESTING LABORATORIES PER SECT 4-335
  - SPECIAL INSPECTION PER SECT. 4-333(C)
- CHANGES IN LEVEL FOR FLOOR FINISHES SHALL CONFORM WITH CBC SECTION 1120B.2 AND 1120B.3.
- ALL TESTS TO CONFORM TO REQUIREMENTS OF SECTION 4-335, PART 1, TITLE 24, CCR.
- TESTS OF MATERIALS AND TESTING LABORATORY SHALL BE IN ACCORDANCE WITH SECTION 4-335, PART 1, TITLE 24, CCR AND THE DISTRICT SHALL EMPLOY AND PAY THE DSA ACCEPTED LABORATORY. COSTS OF RE-TEST MAY BE BACK CHARGED TO THE CONTRACTOR.
- INSPECTOR SHALL BE APPROVED BY DSA. INSPECTION SHALL BE IN ACCORDANCE WITH SECTION 4-333(B).
- THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS IS THAT THE WORK OF THE ALTERATION, REHABILITATION OR RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24, CCR. SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHICH IS NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT (CCD) OR A SEPARATE SET OF PLANS AND SPECIFICATIONS, DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK. (SECTION 4-317(C), PART 1, TITLE 24, CCR).
- INSPECTOR OF RECORD REQUIREMENTS:
  - A. ONE OR MORE INSPECTORS EMPLOYED BY THE OWNER IN ACCORDANCE WITH THE REQUIREMENTS OF TITLE 24 OF THE CALIFORNIA CODE OF REGULATIONS WILL BE ASSIGNED TO THE WORK. THE INSPECTOR'S DUTIES ARE SPECIFICALLY DEFINED IN SECTION 4-342 OF SAID TITLE 24, PART 1, AND IN ADDITION, SHALL BE STIPULATED IN INTERPRETATION OF REGULATION DOCUMENT IR A-8.
  - B. INSPECTOR SHALL BE CERTIFIED AS A CLASS 2 INSPECTOR THROUGH THE DIVISION OF THE STATE ARCHITECT INSPECTOR EXAMINATION PROGRAM. INSPECTOR SHALL ALSO BE SPECIFICALLY APPROVED BY THE DIVISION OF THE STATE ARCHITECT FOR THIS PROJECT AT LEAST 10 DAYS PRIOR TO THE START OF ANY WORK FOR THIS PROJECT.

### DEFERRED APPROVALS

- NONE

### ADD ALTERNATES

- NONE

### PROJECT DESCRIPTION

APN: 014-580-002  
THE PROJECT INCLUDES NEW DRINKING FOUNTAINS, NEW FABRIC AND METAL SHADE STRUCTURE.

NOTE THAT DRINKING FOUNTAINS WILL BE INSTALLED DURING THE SCHOOL YEAR, WHILE ALL OTHER WORK WILL BE INSTALLED DURING THE SUMMER OF 2024.

FABRIC SHADE STRUCTURE TO HAVE 340FR FABRIC FOR FLAME RETARDANT, COMPLYING WITH TITLE 19, SECTION 315(a)

SITE IMPROVEMENTS, INCLUDING ACCESSIBLE PARKING SPACES, AND PLAYGROUND EQUIPMENT AND SURFACING

SITE SPECIFIC WIND = +93 MPH

SEISMIC FORCE CATEGORY = II

### STATEMENT OF GENERAL CONFORMANCE

FOR ARCHITECTS/ENGINEERS WHO UTILIZE PLANS, INCLUDING BUT NOT LIMITED TO SHOP DRAWINGS, PREPARED BY OTHER LICENSED DESIGN PROFESSIONALS AND/OR CONSULTANTS

Application No. 02-122273

File No. 57-31

- [X] The drawings or sheets listed on the cover or index sheet (all C, P and PC Drawings)  
[ ] This drawing, page of specifications/calculations

have been prepared by other design professionals or consultants who are licensed and/or authorized to prepare such drawings in this state. It has been examined by me for:

- design intent and appears to meet the appropriate requirements of Title 24, California Code of Regulations and the project specifications prepared by me, and
- coordination with my plans and specifications and is acceptable for incorporation into the construction of this project.

The Statement of General Conformance "shall not be construed as relieving me of my rights, duties, and responsibilities under Sections 17302 and 81138 of the Education Code and Sections 4-336, 4-341 and 4-344" of Title 24, Part 1, (Title 24, Part 1, Section 4-317 [b])

I find that:  
[X] All drawings or sheets listed on the cover or index sheet  
[ ] This drawing or page

[X] is/are in general conformance with the project design and  
[X] has/have been coordinated with the project plans and specifications

Signature Brian P. Whitmore 03/13/2024  
Date

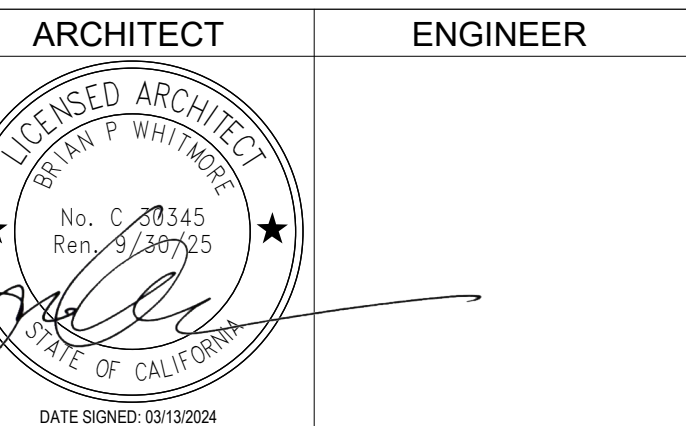
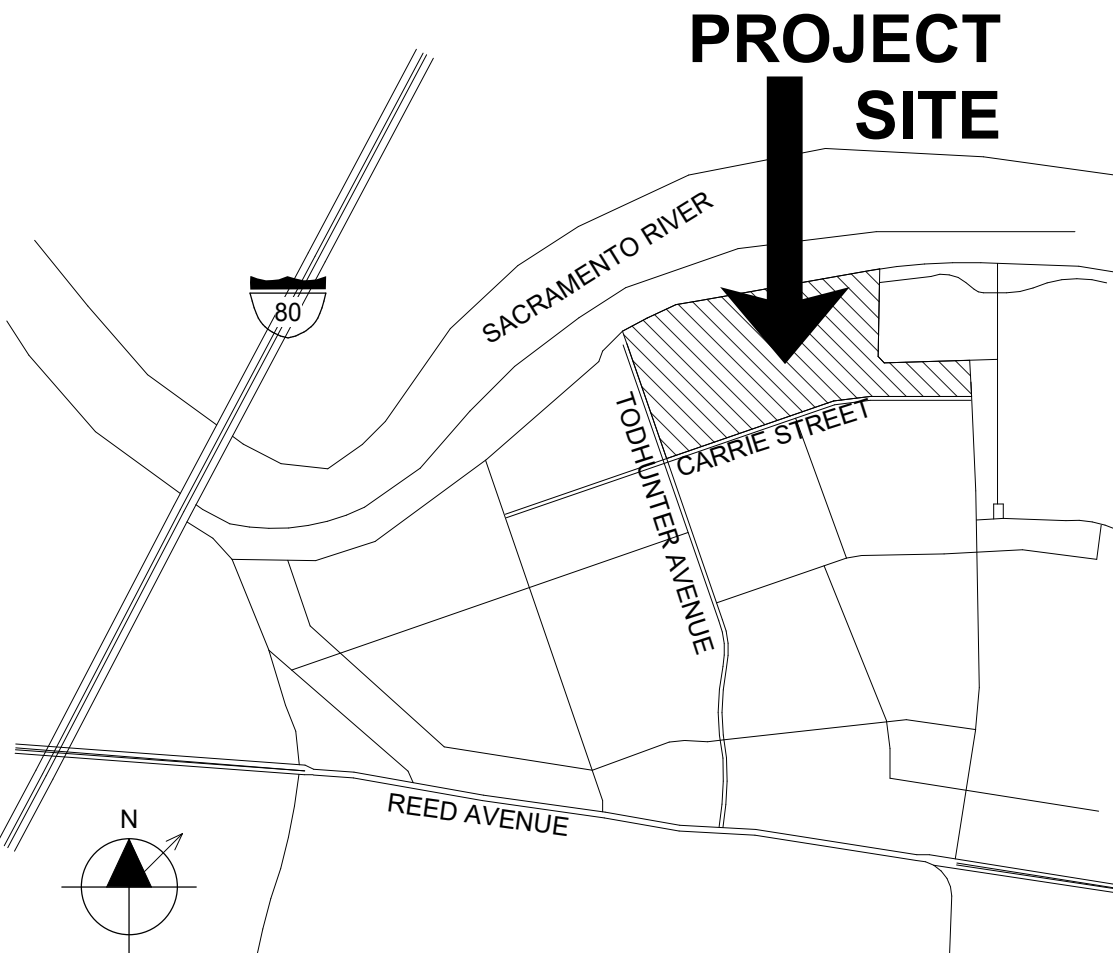
Architect or Engineer designated to be in general responsible charge.

Brian P. Whitmore  
Print Name

C 30345 09-30-2025  
License Number Expiration Date

STATEMENT OF GENERAL CONFORMANCE AND SIGNATURE BLOCK PER IR A-18

### VICINITY MAP

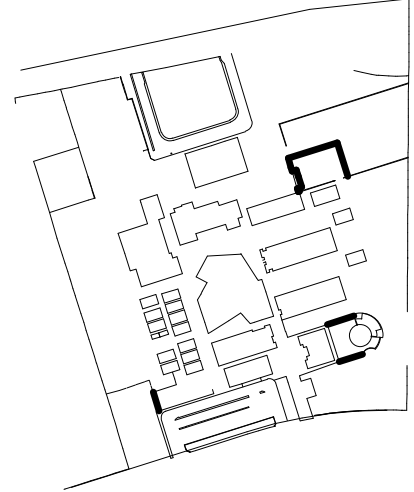


- GENERAL NOTES
- This sheet is part of a set and is not to be used alone.
  - This sheet is not to be used for construction unless the architect's stamp and signature appear on the drawings and the status box indicates drawings have been released for construction.
  - These plans and prints thereof, as instruments of service, are owned by the architect and are for use on this project only. Reproduction and/or distribution without the prior written consent of the architect is forbidden.
  - Copyright Studio W Associates, Inc. 2023.

NO.	REMARKS	DATE

DRAWING STATUS	DATE
<input type="radio"/> DSA PLAN CHECK	
<input type="radio"/> DSA BACK CHECK	
<input type="radio"/> BIDDING	
<input type="radio"/> CONSTRUCTION	

### KEY PLAN



WASHINGTON UNIFIED  
SCHOOL DISTRICT  
930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

### PROJECT STATUS

WUSD RIVERBANK ES  
ESSR III  
1100 CARRIE STREET  
WEST SACRAMENTO, CA 95605

### COVER SHEET

Date  
03/13/2024

Application Number  
02-122273

Drawn  
IO

Project Number  
22042

Drawing Number

A0.1

Checked  
IO

Checker



[illegible]



ARCHITECTURAL DRAWING ABBREVIATIONS

#	POUND OR NUMBER
&	AND
2X	ITEMS IDENTIFIED AS "NIC" ARE NOT PART OF THIS DSA APPROVAL
@	NOMINAL LUMBER SIZE (4X, 6X, 8X, ETC.)
⊥	PERPENDICULAR
A	
A/C	AIR CONDITIONING
A/E	ARCHITECT/ENGINEER
AB	ANCHOR BOLT
ABAN	ABANDON
ABC	AGGREGATE BASE COURSE
ABV	ABOVE
AC	ASPHALTIC CONCRETE
ACC	ACCESSIBLE
ACP	ALUMINUM COMPOSITE PANEL
ACST	ACOUSTICAL
ACT	ACOUSTICAL CEILING TILE
AD	AREA DRAIN
ADDUM	ADDENDUM
ADH	ADHESIVE
ADJ	ADJUSTABLE
ADJC	ADJACENT
AFT	ABOVE FINISH FLOOR
AFG	ABOVE FINISHED GRADE
AGGR	AGGREGATE
AHU	AIR HANDLING UNIT
ALS	ASSISTED LISTENING SYSTEM
ALT	ALTERNATE
ALUM./AL.	ALUMINUM
ANC	ANCHOR, ANCHORAGE
APLD	APPLIED
APPRX	APPROXIMATELY
ARCH	ARCHITECT(URAL)
ASC	ABOVE SUSPENDED CEILING
ASF	ABOVE STAGE FINISH
ASPH	ASPHALT
ASSY	ASSEMBLY
ASYM	ASYMMETRICAL
AUTO	AUTOMATIC
AV	AUDIO VISUAL
AWG	AMERICAN WIRE GAUGE
B	
B	BOLT
BC	BACK OF CURB
BOARD	
BITUM	BITUMINOUS
BLDG	BUILDING
BLK	BLOCK
BLKG	BLOCKING
BLW	BELOW
BLW CLG	BELOW CEILING
BLW FFLR	BELOW FINISH FLOOR
BM	BENCH MARK
BN	BOUNDARY NAILING
BO	BOTTOM OF
BOT	BOTTOM
BRCG	BRACING
BRDG	BRIDGING
BRG	BEARING
BRK	BRICK
BRKT	BRACKET
BRS	BRASS
BRZ	BRONZE
BS	BOTH SIDES
BSMT	BASEMENT
BTWN	BETWEEN
BUR	BUILT UP ROOFING
BW	BOTH WAYS
C	
C&G	CURB AND GUTTER
CAB	CABINET
CAD	CADMIUM
CB	CATCH BASIN
CBB	CEMENTITIOUS BACKER BOARD
CBC	CALIFORNIA BUILDING CODE
CEM	CEMENT
CER	CERAMIC
CFCI	CONTRACTOR FURNISHED CONTRACTOR INSTALLED
CFLG	COUNTERFLASHING
CFOI	CONTRACTOR FURNISHED OWNER INSTALLED
CG	CORNER GUARD
CHBD	CHALKBOARD
CHFR	CHAMFER
CI	CAST IRON
CIP	CAST IN PLACE
CIR	CIRCLE
CIRC	CIRCULAR, CIRCUMFERENCE
CJ	CONSTRUCTION JOINT
CL	CHAIN LINK OR CENTER LINE
CLG	CEILING
CLJ	CONTROL JOINT
CLKG	CAULKING
CLL	CONTRACT LIMIT LINE
CLOS	CLOSURE
CLR	CLEAR(ANCE)
CLRM	CLASSROOM
CMP	CORRUGATED METAL PANEL
CMPST	COMPOSITION
CMU	CONCRETE MASONRY UNIT
CNCL	CONCEALED
CNR	CORNER
CNTR	COUNTER
COL	COLUMN
COM	COMMON
COMB	COMBINATION
COMP	COMPOSITE
COMPT	COMPARTMENT
CONC	CONCRETE
CONF	CONFERENCE
CONN	CONNECTION
CONSTR	CONSTRUCTION
CONT	CONTINUOUS, CONTINUATION
CONTR	CONTRACT(OR)
COORD	COORDINATE
CORR	CORRIDOR
CPR	COPPER
CPRS	COMPRESSED(ED), (ION), (IBLE)
CPT	CARPET
CRS	COLD ROLLED STEEL
CS	CAST STONE
CSG	CASING
CSK	COUNTERSUNK
CSMT	CASEMENT
CSWK	CASEWORK
CT	CERAMIC TILE
CTB	CERAMIC TILE BASE
CTF	CERAMIC TILE FLOOR
CTG	COATING
CTR	CENTER
CURT	CURB FOOT
QUIN	CUBIC INCH
CUST	CUSTODIAN
CUYD	CUBIC YARD
CW	CURTAIN WALL
D	
D	DRAIN
d.	PENNYWEIGHT (NAILS)
DA	DOUBLE ACTING
DBL	DOUBLE
DEG	DEGREES
DEMO	DEMOLISH, DEMOLITION
DEP	DEPRESSED
DEPT	DEPARTMENT
DET	DETAIL
HLDN	HOLD DOWN
DF	DRINKING FOUNTAIN

DG	DECOMPOSED GRANITE
DH	DOUBLE HUNG
DIA	DIAMETER
DIAG	DIAGONAL
DIFF	DIFFUSER
DIM	DIMENSION
DISP	DISPENSER
DIV	DIVISION
DMPF	DAMP/PROOFING
DMT	DEMOUNTABLE
DN	DOWN
DR	DOOR
DRB	DRAINBOARD
DRLV	DOOR LOUVER
DS	ASPHALTIC CONCRETE
DSP	DRY STANDPIPE
DT	DRAIN TILE
DVTL	DOVETAIL
DW	DISHWASHER
DWG	DRAWING
DWL	DOWEL
DWR	DRAWER
E	
(E)	EXISTING
E	EAST
EA	EACH
EAR	EXHAUST AIR REGISTER
EB	EXPANSION BOLT
EE	EACH END
EF	EACH FACE
EFS	EXTERIOR FINISH SYSTEM
EHD	ELECTRIC HAND DRYER
EHS	EXTERIOR INSULATION AND FINISH SYSTEM
EJ	EXPANSION JOINT
EL	ELEVATION
ELAST	ELASTOMERIC
ELEC	ELECTRICAL
ELEV	ELEVATOR
EM	EXPANDED METAL
EMER	EMERGENCY
EN	EDGE NAILING
ENCL	ENCLOSURE
ENGR	ENGINEER
ENTR	ENTRANCE
EP	ELECTRICAL PANELBOARD
EQ	EQUAL
EQUIP	EQUIPMENT
ESC	ESCUTCHEON
ESCL	ESCALATOR
ESMT	EASEMENT
EW	EACH WAY
EWG	ELECTRIC WATER COOLER
EWL	ELECTRIC WATER HEATER
EWS	EYE WASH STATION
EXC	EXCAVATE
EXH	EXHAUST
EXP	EXPOSED
EXPN	EXPANSION
EXS	EXTRA STRONG
EXT	EXTERIOR
F	
(F)	FUTURE
F/F	FACE TO FACE
FA	FIRE ALARM
FAB	FABRIC
FBD	FIBERBOARD
FBK	FIRE BRICK
FCBRK	FACE BRICK
FD	FLOOR DRAIN
FDN	FOUNDATION
FE	FIRE EXTINGUISHER
FEC	FIRE EXTINGUISHER CABINET
FF	FINISH FLOOR
FFA	FROM FLOOR ABOVE
FFB	FROM FLOOR BELOW
FFEL	FINISHED FLOOR ELEVATION
FFL	FINISHED FLOOR LINE
FGL	FIBERGLASS
FHC	FIRE HOUSE CABINET
FHMB	FLAT HEAD MACHINE BOLT
FHMS	FLAT HEAD MACHINE SCREW
FHWS	FLATHEAD WOOD SCREW
FIN	FINISHED
FLJ	FLUSH JOINT
FLASH	FLASHING
FLDG	FOLDING
FLG	FLOORING
FLR	FLOOR
FLUORESC	FLUORESCENT
FN	FIELD NAILING
FOB	FACE OF BLOCK
FOC	FACE OF CONCRETE/CURB
FOF	FACE OF FINISH
FOG	FACE OF GRID
FOM	FACE OF MASONRY
FOS	FACE OF STUD
FPL	FIREPLACE
FRF	FIREPROOF(ING)
FR	FRAME(D), (ING)
FRG	FIBERGLASS REINFORCED GYPSUM
FRP	FIBERGLASS REINFORCED PLASTIC
FRTW	FIRE RETARDANT TREATED WOOD
FRZ	FREEZER
FS	FIRE SPRINKLER
FS	FAR SIDE
FSTN	FASTEN, FASTENER
FT	FOOT/FEET
FTG	FOOTING
FURG	FURRED, (ING)
FWC	FABRIC WALL COVERING
G	
GA	GAUGE
GAL	GALLON
GALV	GALVANIZED
GB	GRAB BAR
GFRG	GLASS FIBER REINFORCED CONCRETE
GI	GALVANIZED IRON
GL	GLASS
GLULAM	GLUE LAMINATED
GLZ	GLAZING
GLZCMU	GLAZED CONCRETE MASONRY UNIT
GND	GROUND
GPC	GYPSUM PLASTER CEILING
GR	GRADE
GRBM	GRADE BEAM
GRLN	GRADE LINE
GSSB	GYPSUM SHEATHING BOARD
GSM	GALVANIZED SHEET METAL
GSS	GALVANIZED STEEL SHEET
GST	GLAZED STRUCTURAL TILE
GUT	GROUT
GVL	GRAVEL
GYP	GYPSUM
GYP BD	GYPSUM BOARD
H	
HB	HOSE BIB
HC	HOLLOW CORE
HD	HEAVY DUTY
HDAS	HEADED ANCHOR STUD
HJUT	HEAD JOINT
HDR	HEADER
HDW	HARDWARE
HOWD	HARDWOOD
HEX	HEXAGONAL
HGR	HANGER
HLDN	HOLD DOWN
HM	HOLLOW METAL

HMD	HOLLOW METAL DOOR
HMF	HOLLOW METAL DOOR AND FRAME
HMF	HOLLOW METAL FRAME
HNDRL	HANDRAIL
HORZ	HORIZONTAL
HPT	HIGH POINT
HR	HOUR
HT	HEIGHT
HTG	HEATING
HVAC	HEATING, VENTILATING, AIR CONDITIONING
HWL	HOT WATER HEATER
I	
I	INSIDE DIAMETER
IN	INCH
INCL	INCLUDE(D), (ING)
INFO	INFORMATION
INSTL	INSTALL
INSUL	INSULATE(D), (ION)
INT	INTERIOR
INV	INVERT
IPS	IRON PIPE SIZE
ISA	INTERNATIONAL SYMBOL OF ACCESSIBILITY
J	
JAN	JANITOR
JST	JOIST
JT	JOINT
K	
KIT	KITCHEN
KO	KNOCKOUT
KPL	KICKPLATE
L	
LAB	LABORATORY
LAD	LADDER
LAM	LAMINATE
LAV	LAVATORY
LB(S)	POUND(S)
LBL	LABEL
LBR	LUMBER
LDR	LEADER
LF	LINEAL FOOT
LG	LENGTH, LONG
LH	LEFT HAND
LHR	LEFT HAND REVERSE
LKNT	LOCKNUT
LKR	LOCKER
LKWASH	LOCKWASHER
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LMST	LEAST
LNSCP	LANDSCAPE(D)
LNTL	LINTEL
LP	LIGHTPROOF
LPT	LOW POINT
LT	LIGHT
LTWT	LIGHTWEIGHT
LV	LOUVER VENT
LVL	LEVEL(ER)
LWC	LIGHTWEIGHT CONCRETE
LWIC	LIGHTWEIGHT INSULATING CONCRETE
M	
MAINT	MAINTAIN(ANCE)
MAS	MASONRY
MATL	MATERIAL
MAX	MAXIMUM
MB	MACHINE BOLT
MBR	MEMBER
MC	MEDICINE CABINET
MCB	METAL CORNER BEAD
MDO	MEDIUM DENSITY OVERLAD
MECH	MECHANICAL
MED	MEDIUM
MEMB	MEMBRANE
MEZZ	MEZZANINE
MFD	METAL FLOOR DECKING
MFR	MANUFACTURER
MH	MANHOLE
MH	FLAT HEAD MACHINE BOLT
MIRR	MIRROR
MISC	MISCELLANEOUS
ML	METAL LATH
MLDG	MOLDING
MLWK	MILLWORK
MO	MASONRY OPENING
MOD	MODULE(AR)
MR	MOISTURE RESISTANT
MRB	MARBLE
MRD	METAL ROOF DECKING
MS	MACHINE SCREW
MTD	MOUNTED
MTL	METAL
MTR	MORTAR
MULL	MULLION
N	
N	NEW
N	NORTH
NAT	NATURAL
NCOMBL	NONCOMBUSTIBLE
NE	NOT EXCEEDING
NF	NEAR FACE
NIC	NOT IN CONTRACT
NLB	NON-LOAD BEARING
NM	NONMETALLIC
NO	NUMBER
NOM	NOMINAL
NR	NOISE REDUCTION
NRC	NOISE REDUCTION COEFFICIENT
NRCA	NATIONAL ROOFING CONTRACTOR'S ASSOCIATION
NS	NEAR SIDE
NTS	NOT TO SCALE
O	
O	OVER
O/O	OUT TO OUT
OA	OVERALL
OBS	OBSCURE
OC	ON CENTER
OCC	OCCUPANTS OR OCCUPANCY
OD	OUTSIDE DIAMETER
OFICI	OWNER FURNISHED CONTRACTOR INSTALLED
OFF	OFFICE
OFOI	OWNER FURNISHED OWNER INSTALLED
OFS	OUTSIDE FACE OF STUD
OHMS	OHM/HEAD MACHINE SCREW
OHWS	OHM/HEAD WOOD SCREW
OI	OWNER INSTALLED
OPH	OPPOSITE HAND
OPNG	OPENING
OPP	OPPOSITE
OPQ	OPAQUE
OPR	OPERABLE
ORD	OVERFLOW ROOF DRAIN
OSB	ORIENTED STRAND BOARD
OVFL	OVERFLOW
OVHD	OVERHEAD
P	
P	PAINT
PA	PUBLIC ADDRESS
PAR	PARALLEL
PAT	PATTERN
PB	PANIC BAR
PBD	PARTICLE BOARD
PC	PORTLAND CEMENT
PCB	PRECAST CONCRETE
PCP	PORTLAND CEMENT PLASTER

PED	PEDESTAL
PERF	PERFORATED
PERIM	PERIMETER
PERP	PERPENDICULAR
PEGBD	PEGBOARD
PH	PHASE
PHS	PHILLIPS HEAD SCREW
PI	POINT OF INTERSECTION
PIV	POST INDICATOR VALVE
PL	PLATE, PROPERTY LINE
PLAM	PLASTIC LAMINATE
PLAS	PLASTER
PLYWD	PLYWOOD
PM	PRESSED METAL
PMF	PRESSED METAL FRAME
PNEU	PNEUMATIC
PNL	PANEL
PNT	PAINT(ED)
POL	POLISHED
POLY	POLYETHYLENE
PORC	PORCELAIN
PORT	PORTABLE
PR	PAIR
PRCST	PRECAST
PREFAB	PREFABRICATED
PREFIN	PREFINISHED
PREFMD	PREFORMED
PRKG	PARKING
PRML	PREFORMED
PROJ	PROJECT
PROP	PROPERTY
PSCONC	PRESTRESSED CONCRETE
PT	POINT
PTD	PAPER TOWEL DISPENSER
PTDF	PRESSURE TREATED DOUGLAS FIR
PTN	PARTITION
PTR	PAPER TOWEL RECEPTACLE
PVC	POLYVINYL CHLORIDE
PVE(D), (ING)	PAVE(D), (ING)
PVMT	PAVEMENT
Q	
QT	QUARRY TILE
QTB	QUARRY TILE BASE
QTF	QUARRY TILE FLOOR
QTR	QUARTER
QTY	QUANTITY
R	
R	RISER
RA	RETURN AIR
RAB	RABBIT
RAD	RADIUS
RB	RESILIENT BASE
RBR	RUBBER
RCP	REINFORCED CONCRETE PIPE
RCVR	RECEIVER
RD	ROOF DRAIN
RDWY	ROADWAY
REBAR	REINFORCING STEEL BARS
REC	RECESSED
RECT	RECTANGULAR
RECYL	RECYCLING
REF	REFERENCE
REFL	REFLECT(ED), (IVE), (OR)
REFR	REFRIGERATOR
REG	REGISTER
REIN	REINFORCED
REMO	REMOVE(ABLE)
REP	REPAIR
REPL	REPLACE
REQD	REQUIRED
RESIL	RESILIENT
RET	RETURN
REV	REVISION(S), REVISED
RF	RESILIENT FLOORING
RFG	ROOFING
RFH	ROOF HATCH
RGDINS	ROOF INSULATION
RH	RIGHT HAND
RHMS	ROUND HEAD MACHINE SCREW
RHR	RIGHT HAND REVERSE
RHWS	ROUND HEAD WOOD SCREW
RL	ROOF LEADER
RLG	RAILING
RM	ROOM
RND	ROUND
RO	ROUGH OPENING
ROW	RIGHT OF WAY
RR	RESTROOM
RS	ROUGH SAWN
RTF	RUBBER TILE FLOORING
RTU	ROOF TOP UNIT
RV	ROOF VENT
RVL	REVEAL
RVS	REVERSE (SIDE)
RVT	RIVET(ED)
RWD	REDWOOD
RWL	RAIN WATER LEADER
S	
S	SOUTH
S2S	SURFACED TWO SIDES
S4S	SURFACED FOUR SIDES
SA	SUPPLY AIR
SALV	SALVAGE
SAM	SELF-ADHERED MEMBRANE
SAT	SUSPENDED ACOUSTICAL TILE
SB	SPLASH BLOCK
SBSTR	SUBSTRATE
SC	SOLID CORE
SCD	SEAT COVER DISPENSER
SCHED	SCHEDULE
SCUP	SCUPPER
SCRN	SCREEN
SD	STORM DRAIN
SDBL	SANDBLAST
SEC	SECONDS
SECT	SECTION
SEP	SEPERATE OR SEPERATION
SF	SQUARE FEET, STOREFRONT
SGL	SINGLE
SHR	SHOWER
SHT	SHEET(ING)
SHTG	SHEATHING
SHV	SHELVES(ING)
SIM	SIMILAR
SK	SINK
SKLT	SKYLIGHT
SLD	SEALED
SLDG	SLIDE(ING)
SLDR	SOLDER
SLNT	SEALANT
SLV	SLEEVE
SM	SHEET METAL
SMACNA	SHEET METAL AND AIR CONDITIONING CONTRACTOR'S NATIONAL ASSOCIATION
SMLS	SEAMLESS
SMS	SHEET METAL SCREW
SND	SANITARY NAPKIN DISPENSER
SNDINS	SOUND INSULATION
SNDU	SANITARY NAPKIN DISPOSAL UNIT
SNT	SEALANT
SP	SPACES
SPC	SUSPENDED PLASTER CEILING
SPD	SOAP DISPENSER
SPEC	SPECIFICATION(S)
SPT	SUPPORT
SQ	SQUARE
SS	STAINLESS STEEL
SSK	SERVICE SINK

ST	STREET
STA	STATION
STAG	STAGGERED
STC	SOUND TRANSMISSION CLASS
STD	STANDARD
STG	SEATING
STIF	STIFFENER
STR	STIRUP
STL	STEEL
STOR	STORAGE
STR	STRAIGHT
STRUC	STRUCTURAL
STU	STRUCT
SUSP	SUSPENDED
SV	SHEET VINYL
SYMM	SYMMETRICAL
SYNTH	SYNTHETIC
SYS	SYSTEM
T	
T	TEMPERED, TOILET, TREAD
T24	TITLE 24
T&B	TOP AND BOTTOM
T&G	TONGUE & GROOVE
TB	THRU BOLT
TBE	THREADED BOTH ENDS
TD	TOWEL DISPENSER
TDR	TOWEL DISPENSER/RECEPTACLE
TEL	TELEPHONE
TEMP	TEMPORARY
TER	TERRAZZO
TER	TO FLOOR ABOVE
TFA	TO FLOOR BELOW
TFB	TO FLOOR BELOW
THD	THREADED(ED)
THERM	THERMAL
THK	THICK
THRES	THRESHOLD
THRU	THROUGH
TKBD	TACKBOARD
TMPD	TEMPERED
TO	TOP OF
TOB	TOP OF BEAM
TOC	TOP OF CURB OR TOP OF CONCRETE
TOF	TOP OF FOOTING
TOFF	TOP OF FINISH FLOOR
TOJ	TOP OF JOIST
TOL	TOLERANCE
TOM	TOP OF MASONRY
TOP	TOP OF PARAPET
TOPV	TOP OF PAVEMENT
TOS	TOP OF SHEATHING
TOSL	TOP OF SLAB
TOST	TOP OF STEEL
TOW	TOP OF WALL OR TOP OF WALK
TPD	TOILET PAPER DISPENSER
TPTN	TOILET PARTITION
TRANS	TRANSITION
TS	TUBE SHEET
TV	TELEVISION
TWLB	TOWEL BAR
TYP	TYPICAL
U	
UC	UNDERCUT
UGND	UNDERGROUND
UL	UNDERWRITER'S LABORATORY
UNFIN	UNFINISHED
UNON	UNLESS OTHERWISE NOTED
UR	URINAL
URM	UNREINFORCED MASONRY
UTIL	UTILITY
V	
VAR	VARIABLES
VB	VINYL BASE
VCT	VINYL COMPOSITION TITLE
VER	VERIFY
VERT	VERTICAL
VEST	VESTIBULE
VF	VINYL FABRIC
VFAT	VIBRATED ACOUSTIC TILE
VJ	V-JOINT(ED)
VNF	VERIFY IN FIELD
VNR	VENEER
VNR	VAPOR RETARDER
VTR	VENT THROUGH ROOF
VWC	VINYL WALL COVERING
W	
W	WEST
W.O.	WHERE OCCURS
W/	WITH
W/O	WITHOUT
WW	WALL TO WALL
WBL	WOOD BLOCKING
WC	WATER CLOSET
WD	WOOD
WDP	WOOD PANELING
WDW	WINDOW
WF	WIDE FLANGE
WFS	WOOD FURRING STRIP
WGL	WIRED GLASS
WH	WATER HEATER
WH	WALL HUNG
WI	WIRING
WD	WIDTH, WIDE
WLD	WELD(ED)
WM	WIRE MESH
WP	WATERPROOFING()
WPT	WORKING POINT
WR	WIRE ROPE
WS	WOOD SCREW
WSC	WAINSCOT
WT	WEIGHT
WWF	WEIGHTED WIRE FABRIC
X	
XBRACE	CROSS BRACE
XFORM	TRANSFORMER
XSECT	CROSS SECTION
Y	
YCO	YARD CLEANOUT
YD	YARD



GENERAL NOTES	ACC. PATH OF TRAVEL
EXISTING CONDITIONS 1. ALL (E) STRUCTURES AND ITEMS ON SITE ARE APPROXIMATE BASED ON DRAWINGS FROM OWNER.  BUILDING 1. ALL EXTERIOR OUTWARD SWINGING DOORS TO HAVE A MINIMUM 5'-0" LEVEL LANDING. 2. ALL BUILDING ENTRANCES AND EXTERIOR GROUND LEVEL EXITS SHALL BE ACCESSIBLE.  ACCESSIBLE PATH OF TRAVEL 1. SEE ACCESSIBLE PATH OF TRAVEL DEFINITION, THIS SHEET. 2. ALL SIDEWALKS ALONG THE ACCESSIBLE ROUTE TO BE A MINIMUM OF 4'-0" WIDE, AND THERE SHALL BE NO DROP-OFFS OVER 4" AT EDGE OF WALK OR LANDING, WHERE A 4" DROP-OFF DOES OCCUR, PROVIDING A 6" HIGH WARNING CURB OR GUARD OR HANDRAIL. (SEE CBC SECTION 11B-303.3) FOR GRATINGS LOCATED IN THE SURFACE OF ANY PEDESTRIAN WALKWAY IN THE PATH OF TRAVEL, GRID OPENINGS IN GRATINGS SHALL BE LIMITED TO 1/2" MAXIMUM IN THE DIRECTION OF TRAFFIC FLOW. 3. 36" WIDE CONTINUOUS DETECTABLE WARNING SHALL BE USED WHERE THE PEDESTRIAN PATH CROSSES OR ADJOINS A VEHICULAR WAY (SUCH AS A DRIVEWAY) TO WARN OF POTENTIAL HAZARDS AS PER CBC 11B-705. 4. SEE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE STATEMENT ON THIS SHEET FOR PATH OF TRAVEL REQUIREMENTS.  GATES 1. GATES ALONG ACCESSIBLE ROUTE SHALL MEET DOOR REQUIREMENTS PER CBC SECTION 11B-404 INCLUDING PANIC HARDWARE AND 10' MIN. SMOOTH BOTTOM OR KICK PLATE. 2. GATES IN PATH OF TRAVEL SHALL COMPLY WITH EXIT DOOR REQUIREMENTS WITH PROPER ACCESSIBLE LEVER HARDWARE AND KICK PLATES.	ACCESSIBLE PATH OF TRAVEL AS INDICATED ON PLANS IS A BARRIER FREE ACCESS ROUTE WITHOUT ANY ABRUPT LEVEL CHANGES EXCEEDING 1/2" IF BEVELED AT 1:2 MAXIMUM SLOPE OR VERTICAL LEVEL CHANGES NOT EXCEEDING 1/4" MAXIMUM AND AT LEAST 48" IN WIDTH. SURFACE IS STABLE, FIRM AND SLIP RESISTANT. CROSS SLOPE DOES NOT EXCEED 2% AND SLOPE IN THE DIRECTION OF TRAVEL IS LESS THAN 5% UNLESS OTHERWISE INDICATED. ACCESSIBLE PATH OF TRAVEL SHALL BE MAINTAINED FREE OF OVERHANGING OBSTRUCTIONS TO 80" MINIMUM AND PROTRUDING OBJECTS GREATER THAN 4" PROJECTION FROM WALL ABOVE 27" AND LESS THAN 80". ARCHITECT SHALL VERIFY THAT THERE ARE NO BARRIERS IN THE PATH OF TRAVEL.

DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE STATEMENT
(BASED ON DSA PROCEDURE PR 15-01)  THE P.O.T. IDENTIFIED IN THESE CONSTRUCTION DOCUMENTS MEETS THE REQUIREMENTS OF THE CURRENT APPLICABLE CALIFORNIA BUILDING CODE (CBC) ACCESSIBILITY PROVISIONS FOR PATH OF TRAVEL REQUIREMENTS FOR ALTERATIONS, ADDITIONS AND STRUCTURAL REPAIRS. AS PART OF THE DESIGN OF THIS PROJECT, THE P.O.T. WAS EXAMINED AND ANY ELEMENTS, COMPONENTS OR PORTIONS OF THE P.O.T. THAT WERE DETERMINED TO BE NONCOMPLIANT WITH THE CBC HAVE BEEN IDENTIFIED AND THE CORRECTIVE WORK NECESSARY TO BRING THEM INTO COMPLIANCE HAS BEEN INCLUDED WITHIN THE SCOPE OF THIS PROJECT'S WORK THROUGH DETAILS, DRAWINGS AND SPECIFICATIONS INCORPORATED INTO THESE CONSTRUCTION DOCUMENTS. ANY NONCOMPLIANT ELEMENTS, COMPONENTS OR PORTIONS OF THE P.O.T. THAT WILL NOT BE CORRECTED BY THIS PROJECT BASED ON VALUATION THRESHOLD LIMITATIONS OR A FINDING OF UNREASONABLE HARDSHIP ARE INDICATED IN THESE CONSTRUCTION DOCUMENTS.  DURING CONSTRUCTION, IF P.O.T. ITEMS WITHIN THE SCOPE OF THE PROJECT REPRESENTED AS CBC COMPLIANT ARE FOUND TO BE NONCONFORMING BEYOND REASONABLE CONSTRUCTION TOLERANCES, THE ITEMS SHALL BE BROUGHT INTO COMPLIANCE WITH THE CBC AS A PART OF THIS PROJECT BY MEANS OF A CONSTRUCTION CHANGE DOCUMENT.

ACCESSIBLE PARKING
BASED ON CBC TABLE 11B-208.2 "PARKING SPACES"  PARKING A STANDARD PARKING PROVIDED: 33 STALLS ACCESSIBLE PARKING PROVIDED: 01 STALLS + 1 VAN STALLS TOTAL PARKING PROVIDED: 36 STALLS  PARKING B STANDARD PARKING PROVIDED: 71 STALLS ACCESSIBLE PARKING PROVIDED: 02 STALLS + 1 VAN STALLS TOTAL PARKING PROVIDED: 74 STALLS

ADSA

810

FIRE & LIFE SAFETY SITE CONDITIONS SUBMITTAL

Division of the State Architect (DSA) documents referenced within this publication are available on the DSA Forms or DSA Publications webpages.  
To facilitate the Division of the State Architect's (DSA) fire and life safety plan review of project site conditions, DSA requires the design professional to provide the following information at time of project submittal for projects consisting of construction of a new campus, construction of new building(s), additions to existing buildings, and for site alternate design means for fire department emergency vehicle access, and fire suppression water supply. Information associated with compliance items 1 through 3 below is to be provided for all project types indicated above. Information associated with items 4 through 7 is to be completed when an alternate means is utilized. Acknowledgement by the school district and signature from the Local Fire Authority (LFA) is only required when an alternate design means is being requested.  
The Project Information and Fire & Life Safety Information sections are to be completed for all projects and signed into the fire access site plan. When an alternate design/means is proposed, all sections on pages 1 and 2 are to be completed and signed on the fire access site plan.  
For additional information refer to the instructions at the end of this form and DSA Policy PL 09-01: Fire Flow for Buildings.

PROJECT INFORMATION

School District/Owner: Washington Unified School District  
Project Name/School: Riverbank Elementary School  
Project Address: 1100 Carrie Street, West Sacramento, CA 95605

FIRE & LIFE SAFETY INFORMATION

1. Has a fire hydrant flow test been performed within the past 12 months?  
(If yes, provide a copy of the test data.)

Yes ☐ No ☒

2. Was the fire hydrant water flow test performed as part of this LFA review?

Yes ☐ No ☒

3. Is the project located within a designated fire hazard severity zone (FHSZ) as established by Cal-Fire? (If yes, indicate FHSZ classification below.)

Refer to the following website for FHSZ locations: [https://gis.ca.gov/arcgis/rest/services/California\\_Fire\\_Hazard\\_Severity\\_Zones/MapServer](https://gis.ca.gov/arcgis/rest/services/California_Fire_Hazard_Severity_Zones/MapServer)  
Wildland Interface Area (WIFA) (If any designations are checked, project design must meet the requirements of CBC Chapter 7A.)

Moderate ☐ High ☐ Very High ☐ WIFA ☐

DSA DSA 810 (revised 12/20/20)  
DIVISION OF THE STATE ARCHITECT

DEPARTMENT OF GENERAL SERVICES

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STATE OF CALIFORNIA

DSA 810

FIRE & LIFE SAFETY SITE CONDITIONS SUBMITTAL

CONDITION MEANS AND METHODS RESOLUTION

ALTERNATE ACCEPTED

4. Emergency vehicle access roadways do not meet CFC requirements.

Yes ☐ No ☐ N/A ☒ NCR ☐

5. Fire Hydrants: Number and spacing does not meet CFC requirements.

Yes ☐ No ☐ N/A ☒ NCR ☐

6. Fire Hydrants: Water flow and pressure are less than CFC minimum.

Yes ☐ No ☐ N/A ☒ NCR ☐

7a. Acceptable Alternate: The location of fire department connection serving the fire sprinkler system and/or standpipe system is acceptable for providing fire suppression and protection of life and property.

Yes ☐ No ☐ N/A ☒ NCR ☐

7b. Location of fire department connection(s) serving fire sprinkler system or standpipe system does not meet CFC requirements.

Yes ☐ No ☐ N/A ☒ NCR ☐

7c. Acceptable Alternate: The location of fire department connection serving the fire sprinkler system and/or standpipe system is acceptable for providing fire suppression and protection of life and property.

Yes ☐ No ☐ N/A ☒ NCR ☐

School District Acceptance of Acceptable Design Alternates  
By signing this form, the school district acknowledges and accepts the proposed design as an alternative to California Building Code (CBC) and California Fire Code (CFC) minimum requirements, as indicated by one or more of the conditions indicated at items 4a, 5a, 6a, 7a or 7b, for providing fire and life safety protection of life and property.  
Accepted by: \_\_\_\_\_ Title: \_\_\_\_\_  
Signature: \_\_\_\_\_ Date: \_\_\_\_\_  
LOCAL FIRE AUTHORITY (LFA) INFORMATION  
LFA Agency Name: West Sacramento Fire Department  
LFA Review Official: Bryan Johnson  
Title: Fire Marshal Work Phone: (916) 617-4008  
Work Email: bryan@cityofwestsacramento.org  
LFA Reviewer's Signature: \_\_\_\_\_ Digitally signed by Bryan Johnson Date: 2024.02.22 12:14:58 PST Date: 02/22/24  
DSA DSA 810 (revised 12/20/20)  
DIVISION OF THE STATE ARCHITECT

DEPARTMENT OF GENERAL SERVICES

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STATE OF CALIFORNIA

Code Analysis Site Plan showing buildings A through V, parking areas, gates, and fire hydrants. Includes a north arrow and scale bar.

	METAL SHADE STRUCTURE	FABRIC SHADE STRUCTURE
BUILDING CONDITION	NEW	NEW
OCCUPANCY (CBC SECTION 302)	A-2	E
ACTUAL BUILDING HEIGHT	15'-0"	15'-0"
ALLOWABLE BUILDING HEIGHT	55'-0"	55'-0"
ALLOWABLE AREA	9,500 SF	14,500 SF
ACTUAL AREA	1,527 SF	1,600 SF
NUMBER OF STORIES	1	1
TYPE OF CONSTRUCTION	II-B	II-B

**LOCATION:** PER DSA IR 31-1, SECTION 5.1: "SHADE STRUCTURES (SS) PROPOSED FOR LOCATION WITHIN THE FRONTAGE AREA OF A NEW OR EXISTING BUILDING DO NOT INCREASE THE FLOOR AREA OF THAT BUILDING. WHEN LOCATED WITHIN THE FRONTAGE AREA OF A BUILDING WHERE THE FRONTAGE HAS BEEN USED FOR AN AREA FACTOR INCREASE, THE SS SHALL NOT EXCEED 1/3 OF THE PROJECTED HORIZONTAL AREA OF THE FRONTAGE AREA WHERE LOCATED."

**FIRE SPRINKLERS:** PER DSA IR 31-1 SECTION 6, AN "AUTOMATIC FIRE SPRINKLER SYSTEM IS NOT REQUIRED FOR FREE-STANDING SHADE STRUCTURES. "...THEREFORE, NO SPRINKLERS HAVE BEEN ADDED TO NEW SHADE STRUCTURES.

**FIRE ALARM:** PER DSA IR 31-1, "OCCUPANTS OF SHADES STRUCTURE SHALL BE CAPABLE OF HEARING THE CAMPUS FIRE ALARM SIGNAL". REFER TO SITE PLAN FOR LOCATION OF FIRE ALARM NOTIFICATION APPLIANCES ADJACENT TO SHADE STRUCTURE.

REFER TO ENLARGED PLAN FOR ADDITIONAL INFORMATION ON SHADE TYPE OF CONSTRUCTION, OCCUPANCY TYPE, AND EGRESS.

BUILDING DSA APPLICATIONS	KEY NOTES
BUILDING ID	DSA APPLICATION NUMBER(S)
BUILDING A	20323, 02-112282
BUILDING B	20323, 02-112282
BUILDING C	20323, 30362, 02-100533, 02-112282
BUILDING D	20323, 02-112282
BUILDING E1	20323, 21450
BUILDING E2	20323, 21450, 02-112282
BUILDING F	20323, 26921, 02-112282
BUILDING G1	21450, 02-112282
BUILDING H	21450, 30382, 02-112282
BUILDING J1	23926, 02-112282
BUILDING J2	30382, 02-112282
BUILDING K	23926, 02-112282
BUILDING L	23926
BUILDING M	23926, 02-112282
BUILDING O	26921, 02-112282
BUILDING P1	52192, 02-117372
BUILDING P2	52192
BUILDING P3	52192, 02-117372
BUILDING P4	54165
BUILDING P5	54165, 02-117372
BUILDING P6	54165, 02-117372
BUILDING P7	54165, 02-117372
BUILDING P8	57947, 02-117372
BUILDING T1	02-102482
BUILDING T2	02-102482, 02-117372
BUILDING T3	02-102482, 02-117372
BUILDING T4	02-102482, 02-117372
BUILDING U	30382, 54165, 02-112282, 02-115821, 02-117372
BUILDING V	02-115491
SOLAR PANELS 1	02-112988
SOLAR PANELS 2	02-112988
SOLAR PANELS 3	02-112988
SHADE STRUCTURE 1	02-110525
SHADE STRUCTURE 2	02-114860
SHADE STRUCTURE 3	02-115485

LEGEND
<div><div></div><div>(E) BUILDING, NOT UNDER SCOPE OF WORK</div></div> <div><div></div><div>BUILDING UNDER SCOPE OF WORK</div></div> <div><div><div>X</div></div><div>ACCESSIBLE BATHROOM FACILITIES: (W) WOMENS (M) MENS (G) GIRLS (B) BOYS (S) ALL GENDER STAFF (SINGLE OCCUPANCY) (N) ALL GENDER STUDENT (SINGLE OCCUPANCY) (DF) DRINKING FOUNTAIN</div></div> <div><div><div>X</div></div><div>EXISTING BATHROOM FACILITIES: (W) WOMENS (M) MENS (G) GIRLS (B) BOYS (S) ALL GENDER STAFF (SINGLE OCCUPANCY) (N) ALL GENDER STUDENT (SINGLE OCCUPANCY) (E) DRINKING FOUNTAIN</div></div> <div><div>.....</div><div>ACCESSIBLE PATH OF TRAVEL, SEE DEFINITION ON THIS SHEET</div></div> <div><div>---</div><div>PROPERTY LINE</div></div> <div><div><div>+</div></div><div>FIRE HYDRANT AND 75' RADIUS CIRCLE</div></div> <div><div><div>+</div></div><div>LOCATION OF ACCESSIBLE EXTERIOR EXIT DOORS, ENTRANCES, AND EGRESS</div></div>

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122273 INC:  
REVIEWED FOR:  
SS ☒ FLS ☒ ACS ☒  
DATE: 05/03/2024

STUDIO W ARCHITECTS  
Studio W Architects  
1930 H Street  
Sacramento, California 95811  
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ARCHITECT  
ENGINEER  
No. C-2345  
Ren. 9/26/25  
DATE SIGNED: 01/05/24

GENERAL NOTES  
1. This sheet is part of a set and is not to be used alone.  
2. This sheet is not to be used for construction unless the architect's stamp and signature appear on the drawings and the status box indicated drawings have been released for construction.  
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REVISION HISTORY  
NO. REMARKS DATE

DRAWING STATUS  
DSA PLAN CHECK  
DSA BACK CHECK  
BIDDING  
CONSTRUCTION

KEY PLAN

WASHINGTON UNIFIED SCHOOL DISTRICT  
930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

PROJECT STATUS

WUSD RIVERBANK ES  
ESSR III  
1100 CARRIE STREET  
WEST SACRAMENTO, CA 95605

CODE ANALYSIS SITE PLAN

Date: 03/13/2024  
Application Number: 02-122273  
Drawn: \_\_\_\_\_  
Author: \_\_\_\_\_  
Checked: \_\_\_\_\_  
Checker: \_\_\_\_\_  
Project Number: 22042  
Drawing Number: A0.5

PRINT DATE: 5/20/2024 11:13:51 AM  
FILE PATH: BM\_390\_022942\_Washington USD Riverbank ES0204C-WUSD Riverbank Site A21.rvt



NOTE: NOT ALL ABBREVIATIONS  
MAY BE USED ON THESE PLANS.

AB	AGGREGATE BASE
AC	ASPHALTIC CONCRETE
AD	AREA DRAIN
APN	ASSESSOR'S PARCEL NUMBER
ARV	AIR RELEASE VALVE
ASB	AGGREGATE SUB-BASE
BO	BLOW-OFF VALVE
BV	BUTTERFLY VALVE
BW	BACK OF WALK
C/L	CENTERLINE
CB	CATCH BASIN
CL	CLASS
CMP	CORRUGATED METAL PIPE
CATV	CABLE TELEVISION
CO	CLEANOUT
COMM	COMMUNICATION
CONC.	CONCRETE
CONST.	CONSTRUCT
CR	CURB RETURN
CS	CONCRETE SURFACE
DC	DOUBLE CHECK VALVE
DDC	DOUBLE DETECTOR CHECK VALVE
DG	DECOMPOSED GRANITE
DI	DROP INLET
DIA	DIAMETER
DWP	DUCTILE IRON PIPE
DWG	DRAINING
DS	DOWNSPOUT
E	ELECTRIC
EP	EDGE OF PAVEMENT
ESMT	EASEMENT
EX	EXISTING
FS	FIRE SERVICE LINE
FDC	FIRE DEPARTMENT CONNECTION
FL	FLOW
FM	SANITARY SEWER FORCE MAIN
FF	FINISHED FLOOR ELEVATION
FH	FIRE HYDRANT
GA	GAS
GR	GRATE ELEVATION
GRD	GRADE ELEVATION
GV	GRADE VALVE
HB	HOSE BIBB
HBD	HEADER BOARD
HDPE	HIGH DENSITY POLYETHYLENE PIPE
HP	HIGH POINT
INV	PIPE INVERT ELEVATION
JNP	JOINT UTILITY POLE
LF	LINEAL FEET
LIP	LIP OF GUTTER
LT	LEFT
M	MOWSTRIP
NTS	NOT TO SCALE
OH	OVERHEAD
PCC	PORTLAND CEMENT CONCRETE
PD	PLANTER DRAIN
PV	POST INDICATOR VALVE
PL	PROPERTY LINE
PP	POWER POLE
PVE	PUBLIC UTILITY EASEMENT
PVC	POLYVINYL CHLORIDE
RCPC	REINFORCED CONCRETE PIPE
R	RADIUS
RIM	MANHOLE RIM ELEVATION (SOLID COVER)
RP	REDUCED PRESSURE BACKFLOW PREVENTER
SC	RIGHT OF WAY
SH	SCHEDULE
SD	STORM DRAIN
SDMH	STORM DRAIN MANHOLE
SE	SUBGRADE ELEVATION
SS	SANITARY SEWER
SSMH	SANITARY SEWER MANHOLE
ST	STANDARD
S/W	SIDEWALK
T	TELEPHONE
TC	TOP OF CURB
TD	TRENCH DRAIN
DCB	TRENCH DRAIN CATCH BASIN
TP	TELEPHONE POLE
TR	TOP OF RAMP ELEVATION
TRW	TOP OF RETAINING WALL
TSW	TOP OF SEAT WALL
TW	TOP OF WALK ELEVATION
U	UTILITY
UG	UNDERGROUND
UON	UNLESS OTHERWISE NOTED
VCP	VITRIFIED CLAY PIPE
W	WATER
W/	WITH
W/O	WITHOUT
WV	WATER VALVE

1. REFER TO ARCHITECTURAL, LANDSCAPE, ELECTRICAL AND PLUMBING PLANS FOR ADDITIONAL DEMOLITION ITEMS.
2. IN THE EVENT THAT ANY UNUSUAL CONDITIONS NOT COVERED BY THE GEOTECHNICAL INVESTIGATION REPORT OR ARE ENCOUNTERED DURING GRADING OPERATIONS THE GEOTECHNICAL ENGINEER AND THE ARCHITECT SHALL BE IMMEDIATELY NOTIFIED FOR DIRECTIONS.
3. ADDITIONAL DEMOLITION INFORMATION MAY BE SHOWN ON THE GRADING, DRAINAGE, AND UTILITY PLANS, AND THOSE PLANS PREPARED BY OTHER DISCIPLINES FOR THIS PROJECT.
4. ALL DEMOLISHED ITEMS SHALL BE DISPOSED OFFSITE AT A SUITABLE, LEGAL, DUMP SITE OR OTHER FACILITY.
5. ALL DISPOSED OF MATERIALS SHALL BE RECYCLED IF POSSIBLE.
6. THE SCHOOL DISTRICT SHALL HAVE SALVAGE RIGHTS TO ANY DEMOLISHED ITEMS SHOWN HEREON. THE CONTRACTOR SHALL GIVE THE DISTRICT NOTICE 7 DAYS PRIOR TO ANY ACT OF DEMOLITION. THE DISTRICT SHALL MOVE ANY RETAINED ITEMS OUT OF THE CONTRACTORS WORK AREA, UNLESS ANOTHER ARRANGEMENT IS MADE WITH THE CONTRACTOR. ANY REMAINING ITEMS BECOME THE PROPERTY OF THE CONTRACTOR AND SHALL BE REMOVED FROM THE PROJECT. ITEMS NOT SHOWN FOR REMOVAL SHALL REMAIN AND SHALL BE PROTECTED FROM DAMAGE DURING CONSTRUCTION TO A REASONABLE EXTENT.
7. EXISTING UTILITY STRUCTURES IN AREAS OF NEW PAVING SHALL BE REMOVED AND REPLACED WITH NEW BOX/COVER AT NEW GRADE UNLESS SPECIFICALLY NOTED OTHERWISE.
8. ITEMS OUTSIDE THE LIMITS OF DEMOLITION SHALL REMAIN AND BE PROTECTED FROM DAMAGE DURING CONSTRUCTION.
9. EXISTING UTILITY STRUCTURES AND PIPING NOT SHOWN ON DEMOLITION PLAN TO BE REMOVED SHALL REMAIN AND BE PROTECTED.
10. SAWCUTS AND SUBSEQUENT PATCH BACK OF CONCRETE WALKS, SHALL BE TO EXISTING CONCRETE JOINT BEYOND THE NEAREST LOCATION OF DEMOLITION. AS FOR PATCH BACK EFFORT HAS BEEN MADE TO LOCATE, SHOW AND COORDINATE WITH EXISTING JOINTS, HOWEVER IF FIELD CONDITIONS ARE OTHERWISE, IT IS UNDERSTOOD TO REMOVE AND PATCH BACK TO THE NEAREST JOINTS BEYOND DEMOLITION.
11. PRIOR TO THE START OF CONSTRUCTION, VERIFY AND POTHOLE ALL UTILITY POINTS OF CONNECTION FOR LOCATION, DEPTH, AND SIZE. IF CONFLICT IS FOUND, CONTACT THE ENGINEER IMMEDIATELY FOR DIRECTION.
12. WITHIN LANDSCAPE AREAS TO BE DEMOLISHED THERE MAY BE EXISTING IRRIGATION LINES NOT SHOWN ON THIS PLAN. CONTRACTOR SHALL REMOVE ALL LINES AND UNENCOUNTERED, MAIN LINES AND CONTROL WIRES MAY ONLY BE REMOVED PROVIDED THAT ROUTING IS KNOWN AND REMOVAL WILL NOT DEACTIVATE AN IRRIGATION SYSTEMS INTENDED TO REMAIN. IF CONFLICT IS FOUND, CONTACT THE ENGINEER FOR DIRECTION.
13. COORDINATE REMOVAL OF LANDSCAPE ITEMS WITH LANDSCAPE PLANS.

Know what's below.  
**Call** before you dig.

- THE TYPES, LOCATIONS, SIZES, AND/OR DEPTHS OF EXISTING UNDERGROUND UTILITIES AS SHOWN ON THESE PLANS WERE OBTAINED FROM SOURCES OF VARYING RELIABILITY. THE CONTRACTOR IS CAUTIONED THAT THE ONLY ACTUAL VERIFICATION OF THE TYPES, LOCATIONS, SIZES, LOCATIONS AND DEPTHS OF SUCH UNDERGROUND UTILITIES, A REASONABLE EFFORT HAS BEEN MADE TO LOCATE AND DELINEATE ALL KNOWN UNDERGROUND UTILITIES. THE CONTRACTOR, WARREN CONSULTING ENGINEERS CAN ASSUME NO RESPONSIBILITY FOR THE COMPLETENESS OR ACCURACY OF ITS DELINEATION OF SUCH UNDERGROUND UTILITIES, NOR FOR THE EXISTENCE OF OTHER BURIED OBJECTS OR UTILITIES WHICH MAY BE ENCOUNTERED BUT WHICH ARE NOT SHOWN ON THESE PLANS. THE CONTRACTOR OR ANY SUBCONTRACTOR FOR THIS CONTRACT SHALL NOTIFY MEMBERS OF UNDERGROUND SERVICE ALLIANCE (USA) TWO (2) WORKING DAYS IN ADVANCE OF PERFORMING ANY EXCAVATION WORK BY CALLING TOLL FREE 1-800-227-2600, OR 811.

2. WARREN CONSULTING ENGINEERS, INC. (WCE) ASSUMES NO RESPONSIBILITY FOR ERRORS IN PHYSICAL LOCATION OF IMPROVEMENTS. PARALLEL TO THE EXISTING ROADWAY WILL REVEAL THE TYPES, LOCATIONS, SIZES, LOCATIONS AND DEPTHS OF SUCH UNDERGROUND UTILITIES, A REASONABLE EFFORT HAS BEEN MADE TO LOCATE AND DELINEATE ALL KNOWN UNDERGROUND UTILITIES. THE CONTRACTOR, WARREN CONSULTING ENGINEERS CAN ASSUME NO RESPONSIBILITY FOR THE COMPLETENESS OR ACCURACY OF ITS DELINEATION OF SUCH UNDERGROUND UTILITIES, NOR FOR THE EXISTENCE OF OTHER BURIED OBJECTS OR UTILITIES WHICH MAY BE ENCOUNTERED BUT WHICH ARE NOT SHOWN ON THESE PLANS. THE CONTRACTOR OR ANY SUBCONTRACTOR FOR THIS CONTRACT SHALL NOTIFY MEMBERS OF UNDERGROUND SERVICE ALLIANCE (USA) TWO (2) WORKING DAYS IN ADVANCE OF PERFORMING ANY EXCAVATION WORK BY CALLING TOLL FREE 1-800-227-2600, OR 811.

3. IF SUBSURFACE CULTURAL RESOURCES, REMAINS, AND/OR ARTIFACTS ARE UNCOVERED DURING PROJECT CONSTRUCTION, ALL WORK IN THE VICINITY SHALL BE STOPPED UNTIL SUCH ITEMS CAN BE ASSESSED BY AN APPROPRIATE MEMBER OF THE COUNTY ENVIRONMENTAL IMPACT SECTION STAFF.

4. CONTRACTOR AGREES THAT HE/SHE SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONSTRUCTION DURING THE PROJECT. NO OTHER PERSONS, INCLUDING ANY CITY OF BEND OWNERS AND PROPERTY, THAT THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND SHALL NOT BE LIMITED TO NORMAL WORKING HOURS; AND THAT THE CONTRACTOR SHALL DEFEND, INDEMNIFY AND HOLD THE OWNER AND ENGINEER HARMLESS FROM AND AGAINST ALL SUCH CLAIMS, DAMAGES, LOSSES, AND EXPENSES, INCLUDING THE PERFORMANCE OF WORK ON THIS PROJECT, EXCEPTING FOR LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE OWNER OR ENGINEER.

5. THE CONTRACTOR SHALL OBTAIN AN EXCAVATION PERMIT FROM THE STATE OF CALIFORNIA DEPARTMENT OF INDUSTRIAL SAFETY FOR ALL EXCAVATIONS OF 5 FEET OR MORE IN DEPTH.

6. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO MAKE NECESSARY PRE-CONSTRUCTION SITE REVIEWS TO DETERMINE NECESSARY MEANS AND METHODS TO COMPLETE THE IMPROVEMENTS SHOWN ON THESE PLANS.

7. WHERE IMPROVEMENTS LIE WITHIN AN EXISTING DEVELOPED AREA, CONTRACTOR SHALL USE CAUTION WHEN ACCESSING THE SITE THROUGH THESE EXISTING IMPROVEMENTS. IT IS THE CONTRACTORS RESPONSIBILITY TO PROTECT ANY SUCH EXISTING IMPROVEMENTS OUTSIDE THE PROJECT BOUNDARY, OR EXISTING IMPROVEMENTS WITHIN THE BOUNDARY. IF ANY ARE NOT SHOWN ON PROPER PRECAUTIONS SHALL BE PROVIDED AND MAINTAINED THROUGHOUT CONSTRUCTION. ANY DAMAGE SHALL BE REPAIRED OR REPLACED TO THE SATISFACTION OF THE OWNER.

8. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO KEEP DETAILED RECORDS OF MINOR CHANGES OR ADJUSTMENTS MADE DURING CONSTRUCTION (WHICH WERE NOT FORMALLY ISSUED). UPON PROJECT COMPLETION, THESE RECORDS AND ALL INFORMATION SHALL BE PROVIDED TO THE OWNER AND WARREN CONSULTING ENGINEERS, INC. UNLESS AN OFFICIAL "AS-BUILT" SET OF PLANS IS A REQUIREMENT OF THE CONTRACT. IF AS-BUILT PLANS ARE A REQUIREMENT OF THE CONTRACT, REFER TO SPECIFICATIONS FOR AS-BUILT DELIVERABLE REQUIREMENTS.

9. IN VEHICULAR PARKWAYS, EXISTING ASPHALTIC AND/OR CONCRETE SURFACES SHALL BE CUT TO A NEAT AND STRAIGHT LINE, PARALLEL TO THE CENTERLINE OF THE VEHICULAR TRAVELED PATH. THIS IS TYPICALLY THE ROADWAY CENTERLINE, BUT MAY VARY. THAT SAWCUT EDGE SHALL BE PROTECTED FROM DAMAGE DURING CONSTRUCTION SO A CLEAN EDGE REMAINS FOR PATCH BACK. IF EDGE IS DAMAGED, A NEW SAW CUT MUST BE REQUIRED. THE EXPOSED EDGE SHALL BE "TACKED" WITH EMULSION PRIOR TO PAVING.

10. NO BURNING OR BLASTING SHALL BE ALLOWED ON SITE UNLESS SPECIFICALLY ADDRESSED ON PLANS, OR SPECIFICALLY APPROVED AND COORDINATED WITH THE ARCHITECT, ENGINEER, AND LOCAL AGENCY OR OTHER ADMINISTRATIVE AUTHORITY.

11. SUBGRADE AND RESULTING FINISHED GRADE SHALL BE CONSTRUCTED SMOOTH AND UNIFORM BETWEEN SPOT ELEVATIONS, CONTOURS OR OTHER STRUCTURE ELEVATIONS SHOWN ON GRADING OR OTHER PLANS. NO MOUNDS, RUTS, DEPRESSIONS OR OTHER GRADING DEFICIENCIES WILL BE ALLOWED UNLESS SPECIFICALLY SHOWN ON PLANS.

12. ON NEW WATER SYSTEMS, SERVICE LATERALS SHALL BE MADE USING APPROPRIATE "TEE" AND "WYE" FITTINGS. SADDLE TAPS WILL ONLY BE ALLOWED WHEN MAKING CONNECTIONS TO EXISTING WATER MAINS.

13. CURING COMPOUND SHALL BE APPLIED IN A CONTINUOUS SOLID WET FLOWING COAT. ANY "SPOTTY" APPLICATIONS SHALL BE RECOATED IMMEDIATELY. APPLICATION SHALL BE INSPECTED BY PROJECT INSPECTOR DURING APPLICATION.

14. EMBEDMENT OF FEATURES IN CONCRETE PAVING, CURBS, OR WALLS, SUCH AS SQUARE OR ROUND TUBING, POSTS, OR COLUMNS, LATERAL LINES AND HEADS ENCOUNTERED. MAIN LINES AND CONTROL WRES MAY ONLY BE REMOVED TO FILL OR TO FILL JOINTS WITH CONCRETE OR GRASS. ANY CRACKS OUTSIDE OF JOINTS WHICH WERE CONSTRUCTED LESS THAN 1" DEEP, SHALL BE CAUSE FOR CONCRETE SLAB(S) TO BE REMOVED AND REPLACE AT CONTRACTORS EXPENSE.

15. EMBEDMENT OF FEATURES IN CONCRETE PAVING, CURBS, OR WALLS, SUCH AS SQUARE OR ROUND TUBING, POSTS, OR COLUMNS, STEEL BOLTED PLATES, OR OTHER STRUCTURES, SHALL REQUIRE A MINOR ADJUSTMENT OF THE SLAB WITHIN CONCRETE TO ALLOW FOR SUCH STRUCTURE. THAT REAR ADJUSTMENT MAY NOT BE SPECIFICALLY SHOWN ON PLANS.

16. NO MORE THAN 1 GALLON OF WATER PER YARD OF CONCRETE CAN BE ADDED TO THE TRUCK AFTER ARRIVAL TO PROJECT SITE. THE ADDITION OF WATER CAN ONLY BE ADDED UNDER THE SUPERVISION OF THE CONCRETE INSPECTOR OR LABORATORY TECHNICIAN.

17. WHEN PUMPING CONCRETE FOR PLACEMENT, ABSOLUTELY NO WATER IS TO BE ADDED TO PUMP HOPPER. ANY WATER ADDED TO HOPPER WILL BE REASON FOR CONCRETE REJECTION AT THE CONTRACTORS EXPENSE.

18. ALL CONSTRUCTION/CONSTRUCTION JOINTS "CJ" SHALL BE 1/4 THE SLAB THICKNESS DEEP, BUT NO LESS THAN 1" FOR CONTROLLING OF CRACKING. CONTRACTOR SHALL EXERCISE CAUTION WHEN FINAL TROWELING OF CONCRETE TO BE ADDED TO FILL JOINTS WITH CONCRETE OR GRASS. ANY CRACKS OUTSIDE OF JOINTS WHICH WERE CONSTRUCTED LESS THAN 1" DEEP, SHALL BE CAUSE FOR CONCRETE SLAB(S) TO BE REMOVED AND REPLACE AT CONTRACTORS EXPENSE.

19. ANY SCREED BOARDS SET WITHIN CONCRETE SLABS SHALL BE AN "OVERHEAD SCREED" SO THERE IS NO INTERFERENCE WITH THE PLACEMENT AND ALIGNMENT OF SLAB REINFORCING.

20. 3-1/2" FELT JOINTS WILL NOT BE ACCEPTED. PROVIDE A 4" FELT JOINT FOR 4" SLAB CONSTRUCTION, AND A 6" FELT JOINT FOR A 6" SLAB SLAB CONSTRUCTION.

21. SHOULD ANY SHRINKAGE CRACKS OCCUR OUTSIDE OF EITHER THE EXPANSION JOINTS OR CRACK CONTROL JOINTS, THEN THE CONCRETE SLAB SHALL BE SAWCUT AT THE NEAREST JOINTS ON EACH SIDE OF THE CRACK AND FILL TO CONCRETE OR GRASS. JOINTS WITH CONCRETE OR GRASS TO BE REMOVED AND REPLACED. NEW CONCRETE SHALL BE DOWELED INTO EXISTING CONCRETE PER DRAWING DETAIL.

22. ALL AREAS DISTURBED BY GRADING OPERATIONS WHETHER SHOWN ON THE DRAWINGS OR NOT SHALL BE HYDROSEEDED UNLESS OTHERWISE NOTED. HYDRO SEEDING SHALL CONFORM TO LOCAL CITY/COUNTY STANDARDS.

23. REPAIR OR PATCHING OF GALVANIZED METALS, SUCH AS AFTER WELDING GALVANIZED COMPONENTS, SHALL BE MADE USING A ZINC COMPOUND "HOT STICK" APPLICATION PER ASTM A 780-01. GALVANIZING PAINTS WILL NOT BE ALLOWED.

24. AT LIMITS OF NEW PAVEMENT OR CURBS ADJACENT TO LANDSCAPING PROVIDE A 4:1 MINIMUM TRANSITION TO EXISTING GRADE WITH TOPSOIL. ADJUST EXISTING IRRIGATION HEADS TO FINISH GRADE AND PROVIDE SOD IN GRASS AREAS TO RESTORE TO EXISTING CONDITION.

25. WITHIN LIMITS OF WORK THERE MAY BE EXISTING IRRIGATION LINES NOT SHOWN ON THIS PLAN. CONTRACTOR SHALL REMOVE LATERAL LINES AND HEADS ENCOUNTERED. MAIN LINES AND CONTROL WRES MAY ONLY BE REMOVED PROVIDED THAT ROUTING IS KNOWN AND REMOVAL WILL NOT DEACTIVATE AN IRRIGATION SYSTEMS INTENDED TO REMAIN. IF CONFUSIT IS FOUND, CONTACT THE ARCHITECT FOR DIRECTION.

26. GENERAL CONTRACTOR IS REQUIRED TO HIRE A LANDSCAPE SUBCONTRACTOR TO PERFORM ALL LANDSCAPE AND IRRIGATION REPAIRS.

27. ALL TRANSITIONS TO EXISTING PAVEMENT SHALL BE A SMOOTH AND LEVEL TRANSITION.

28. WIDTH OF NEW SIDEWALKS SHALL MATCH WIDTH OF EXISTING, ADJACENT, SIDEWALKS.

29. SEE ARCHITECTURAL PLANS FOR PAVEMENT AND CONTROL JOINT LAYOUT.

30. ADJUST TO FINISH GRADE ALL UTILITY BOXES, FRAMES, COVERS, SLEEVES, POST HOLES GRATES, ETC. FOUND IN AREA OF WORK, WHETHER SHOWN OR NOT. CLEAN OR REPLACE AS NECESSARY TO ENSURE PROPER SEATING.

31. FOR ACCESSIBLE PATH OF TRAVEL REQUIREMENTS SEE ARCHITECTURAL SHEETS.

32. PERCENT OF SLOPE SHOWN ON ARROWS ARE MAXIMUM SLOPES AND NOT INTENDED TO SUPERCEDE SLOPES 0.0% MAX

33. WITHIN THE LIMITS OF ACCESSIBLE PARKING AREA AND ACCESSIBLE DROP OFF ZONE THE SLOPE OF PAVEMENT SHALL NOT EXCEED 1.8% IN ANY DIRECTION.

34. TRANSITIONS BETWEEN CONCRETE AND OR ASPHALT SURFACES SHALL BE FLUSH, UNLESS NOTED OTHERWISE BY CURB OR STEP.


35. TRANSITION BETWEEN PAVED SURFACES AND LANDSCAPE AREAS SHALL BE NO GREATER THAN 1", UNLESS NOTED OTHERWISE.

36. THE MINIMUM SLOPE AWAY FROM THE BUILDING ON PAVED SURFACES SHALL BE 1%.



C0.1	TOPOGRAPHIC SURVEY
C0.2	UTILITY SURVEY
C1.1	DEMOLITION PLAN
C1.2	DEMOLITION PLAN
C2.1	GRADING PLAN
C2.2	GRADING PLAN
C3.1	UTILITY PLAN
C3.2	UTILITY PLAN
C4.1	PAVING AND STRIPING PLAN
C4.2	PAVING AND STRIPING PLAN
C5.1	DETAILS AND SECTIONS



ARCHITECT	ENGINEER
	

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DRAWING STATUS	DATE
<input type="radio"/> DSA PLAN CHECK	
<input type="radio"/> DSA BACK CHECK	
<input type="radio"/> BIDDING	
<input type="radio"/> CONSTRUCTION	

WASHINGTON UNIFIED  
SCHOOL DISTRICT  
930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

CONSTRUCTION DOCUMENTS

WUSD RIVERBANK ES  
ESSR III  
1100 CARRIE STREET  
WEST SACRAMENTO, CA 95605

# CIVIL GENERAL NOTES AND ABBREVIATIONS

Date	Project Number
11/20/2023	22042
Application Number	Drawing Number
-	
Drawn	<b>C0.0</b>
Checked	
AT	
AT	



EXISTING TOPOGRAPHY

- PROPERTY LINE
- CENTERLINE
- EASEMENT
- PROPERTY CORNER FOUND AS NOTED
- PROPERTY CORNER NOTHING FOUND OR SET
- TEMPORARY BENCHMARK (SEE TBM LIST FOR INFO)
- SWALE OR DRAINAGE FLOW
- DRAINAGE FLOW
- FENCE (TYPE NOTED)
- TREE (SIZE/TYPE INDICATED)
- SLOPE
- CONTOUR
- CONCRETE SURFACE
- EDGE OF ASPHALT
- EDGE OF BUILDING
- SIGN
- POST OR BOLLARD
- GROUND ELEVATION
- HARD SURFACE ELEVATION

ABBREVIATIONS

- NOTE: NOT ALL ABBREVIATIONS MAY BE USED ON THESE PLANS.
- ACC ACCESSIBLE
  - ACU AIR CONDITIONING UNIT
  - AD AREA DRAIN
  - APN ASSESSOR'S PARCEL NUMBER
  - ARV AIR RELEASE VALVE
  - BBALL BASKETBALL POLE
  - BCM BRASS CAP MONUMENT
  - BFP BACK FLOW PREVENTER
  - BLK BLOCK
  - BLDG BUILDING
  - BOLL BOLLARD
  - BOV BLOW-OFF VALVE
  - BRK BRICK
  - BWF BARBED WIRE FENCE
  - CA TV COMMUNICATION
  - CIP CABLE TELEVISION
  - C/L CAPPED IRON PIPE
  - C/L CENTERLINE
  - C/L CHAIN LINK FENCE
  - CMP CORRUGATED METAL PIPE
  - CO CLEANOUT
  - COL COLUMN
  - CONC CONCRETE
  - COND CONDENSATE
  - CPF CONTROL POINT FOUND
  - CPS CONTROL POINT SET
  - CS CONCRETE SURFACE
  - DF DRINKING FOUNTAIN
  - DG DECOMPOSED GRANITE
  - DI DROP INLET
  - DRWY DRIVEWAY
  - DS DOWNSPOUT
  - DWG DRAWING
  - E ELECTRIC
  - EP EDGE OF PAVEMENT
  - ESMT EASEMENT
  - FA FIRE ALARM
  - FDC FIRE DEPARTMENT CONNECTION
  - FFE FINISHED FLOOR ELEVATION
  - FH FIRE HYDRANT
  - FL FLOWLINE
  - FO FIBER OPTIC
  - FS FIRE SERVICE
  - G GAS
  - GB GRADE BREAK
  - GRATE
  - GRB GROUND ROD BOX
  - GROD GROUND ROD
  - GV GAS VALVE
  - H HYDRONICS
  - HB HOSE BIBB
  - HBB HOSE BIBB BOX
  - HED HEAD BOARD
  - HP HIGH PRESSURE
  - HVE HANDRAIL
  - HWF HIGH VOLTAGE ELECTRIC
  - ICV IRRIGATION CONTROL VALVE
  - ICP IRRIGATION CONTROL PANEL
  - INV INVERT ELEVATION
  - IRR IRRIGATION
  - JP JOINT UTILITY POLE
  - JT JOINT TRENCH
  - LDNG LANDING
  - LVE LOW VOLTAGE ELECTRIC
  - M METAL
  - MH MANHOLE
  - MS MOW STRIP
  - MSC METAL STORAGE CONTAINER
  - MULTI MULTIPLE
  - OH OVERHEAD
  - OHANG OVERHANG
  - OIP OPEN IRON PIPE
  - PA PLANTER AREA
  - PB PARKING BUMPER
  - PH POSTHOLE
  - PIV POST INDICATOR VALVE
  - PP POWER POLE
  - PIR PUBLIC RECORD INFORMATION
  - PUE PUBLIC UTILITY EASEMENT
  - PV PAVERS
  - PVC POLYVINYL CHLORIDE
  - R RUBBER
  - RG ROLLING GATE
  - RM MANHOLE RM ELEVATION
  - RR RUBBER RAMP
  - RWD RETAINING WALL
  - RWL REDWOOD
  - RS RAIN WATER LEADER
  - SB SIGNAL BOX
  - SDM STORM DRAIN
  - SDMH STORM DRAIN MANHOLE
  - SL SIGN
  - SLB STREET LIGHT BOX
  - SLB SANITARY SEWER
  - SSCO SANITARY SEWER CLEANOUT
  - SMH SANITARY SEWER MANHOLE
  - STL STEEL
  - TELEPHONE
  - TBALL TETHER BALL POLE
  - TBM TEMPORARY BENCHMARK
  - TC TOP OF CURB
  - TD TRENCH DRAIN
  - TP TELEPHONE POLE
  - TRW TOP OF RETAINING WALL
  - UG UNDERGROUND
  - UNK UNKNOWN
  - VENT
  - VBALL VOLLEYBALL
  - W WATER
  - W/ WITH
  - WO WOOD
  - WF WOOD FENCE
  - WLF WROUGHT IRON FENCE
  - WRF WOOD RAIL FENCE
  - XF TRANSFORMER
  - XWALK CROSSWALK
  - YD YARD DRAIN

IDENTIFICATION STAMP  
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APP: 02-122273 INC:  
REVIEWED FOR  
SS FLS ACS  
DATE: 05/03/2024



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NO.	REMARKS	DATE

REVISION HISTORY

NO.	REMARKS	DATE

DRAWING STATUS

☐ DSA PLAN CHECK

☐ DSA BACK CHECK

☐ BIDDING

☐ CONSTRUCTION

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1100 CARRIE STREET  
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TOPOGRAPHIC  
SURVEY

Date 11/20/2023	Project Number 22042
Application Number	Drawing Number
Drawn AT	Checked AT

C0.1

TBM LIST

NUMBER	DESCRIPTION	NORTHING	EASTING	ELEVATION
2	CPF STREET MONUMENT	-432.06	-23.64	14.97
3	CPF STREET MONUMENT	-385.19	283.70	15.15
41	CPS CHISELED "+"	-341.23	91.97	17.80
42	CPS CHISELED "+"	-197.11	38.99	21.44
43	CPS CHISELED "+"	-185.98	75.86	21.37
44	CPS CHISELED "+"	-138.88	22.08	19.84
46	CPS CHISELED "+"	-27.01	-8.39	21.38
47	CPS CHISELED "+"	0.60	-136.85	21.31
48	CPS CHISELED "+"	50.77	120.68	21.57
52	CPS CHISELED "+"	-107.06	-29.17	20.00
53	CPS CHISELED "+"	-44.10	182.66	21.31
54	CPS CHISELED "+"	18.34	228.70	21.40
55	CPS CHISELED "+"	100.16	-39.24	21.53
56	CPS CHISELED "+"	183.32	-37.90	21.78
57	CPS CHISELED "+"	25.98	-211.58	21.23
58	CPS CHISELED "+"	-66.08	-223.91	21.46
59	CPS CHISELED "+"	-207.40	-206.24	20.04
60	CPS CHISELED "+"	-263.72	-315.50	19.54
61	CPS CHISELED "+"	-50.53	-171.64	21.34
62	CPS CHISELED "+"	196.78	-250.21	21.33
63	CPS PK-WASHER	204.82	180.11	19.89

BENCHMARK NO. C11-02 ELEV. 16.87

FD A 3 1/2" BRASS DISK SET IN A STANDARD MONUMENT BOX  
STAMPED: CITY OF WEST SACRAMENTO GEODETIC CONTROL  
SURVEY STATION C11-02, RCE 30639, 1993. LOCATED ALONG  
THE APPROXIMATE CENTERLINE OF TODHUNTER AVE. 438'  
SOUTHERLY OF THE CENTERLINE OF CARRIE ST.

GRAPHIC SCALE

20' 0 10' 20' 40'

(IN FEET) 1 inch = 20 feet

THIS DRAWING MAY HAVE BEEN ENLARGED OR REDUCED.

NORTH

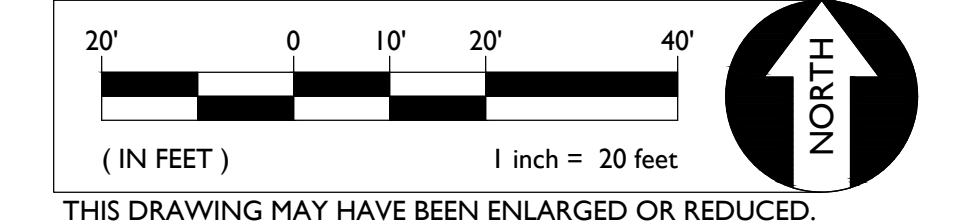




# EXISTING UTILITIES

- 12" SD = STORM DRAIN LINE (SIZE & DIRECTION OF FLOW)
- 12" SD = STORM DRAIN LINE (RECORD INFORMATION)
- 12" SD = STORM DRAIN LINE (UNDERGROUND LOCATING)
- ⊙ = STORM DRAIN MANHOLE
- = STORM DRAIN CLEANOUT
- = DROP INLET
- = AREA DRAIN
- ° RML = RAIN WATER LEADER
- ° DS = DOWNSPOUT
- 12" SS = SANITARY SEWER LINE (SIZE & DIRECTION OF FLOW)
- 12" SS = SANITARY SEWER LINE (RECORD INFORMATION)
- 12" SS = SANITARY SEWER LINE (UNDERGROUND LOCATING)
- ⊙ = SANITARY SEWER MANHOLE
- = SANITARY SEWER CLEANOUT
- W — = WATER LINE (SIZE INDICATED)
- W — = WATER LINE (RECORD INFORMATION)
- W — = WATER LINE (UNDERGROUND LOCATING)
- ⊙ = WATER MANHOLE
- ⊙ = WATER VALVE
- ⊙ = WATER METER
- ⊙ = WATER BOX
- ⊙ = IRRIGATION CONTROL VALVE
- ⊙ = FIRE HYDRANT
- ⊙ = BACKFLOW PREVENTER
- ⊙ = SPRINKLER
- ⊙ = HOSE BIBB
- OH — = OVERHEAD ELECTRIC LINE
- E — = UNDERGROUND ELECTRIC LINE
- E — = UNDERGROUND ELECTRIC LINE (RECORD INFORMATION)
- E — = UNDERGROUND ELECTRIC LINE (UNDERGROUND LOCATING)
- ⊙ = ELECTRIC MANHOLE
- ⊙ = UTILITY POLE (WITH GUY WIRE)
- ⊙ = ELECTRIC METER
- ⊙ = ELECTRIC BOX
- ⊙ = STREET LIGHTING BOX
- ⊙ = LIGHT STANDARD
- ⊙ = SIGNAL LIGHT
- ⊙ = FLOOD LIGHT
- ⊙ = ELECTRICAL OUTLET
- G — = GAS LINE (SIZE INDICATED)
- G — = GAS LINE (RECORD INFORMATION)
- G — = GAS LINE (UNDERGROUND LOCATING)
- ⊙ = GAS MANHOLE
- ⊙ = GAS VALVE
- ⊙ = GAS METER
- T — = TELEPHONE LINE
- T — = TELEPHONE LINE (RECORD INFORMATION)
- T — = TELEPHONE LINE (UNDERGROUND LOCATING)
- ⊙ = STORM DRAIN BOX
- ⊙ = TRAFFIC SIGNAL BOX

## GRAPHIC SCALE



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APP: 02-122273 INC.  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☒  
DATE: 05/03/2024



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ARCHITECTS

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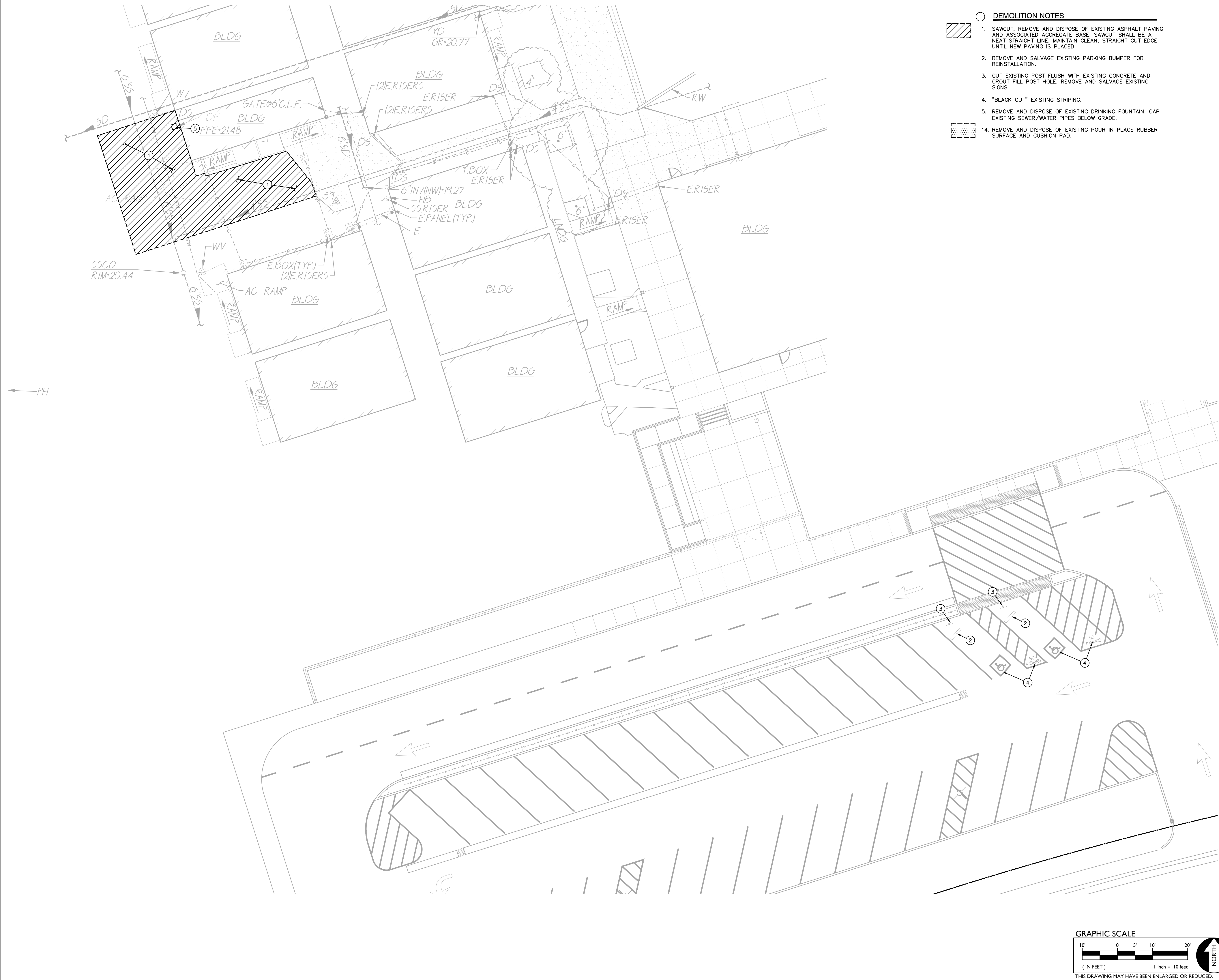
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UTILITY SURVEY

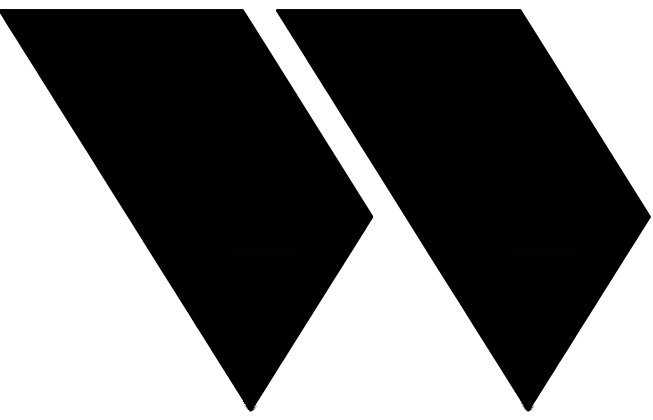
Date 11/20/2023	Project Number 22042
Application Number -	Drawing Number C0.2
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- DEMOLITION NOTES**
1. SAWCUT, REMOVE AND DISPOSE OF EXISTING ASPHALT PAVING AND ASSOCIATED AGGREGATE BASE. SAWCUT SHALL BE A NEAT STRAIGHT LINE, MAINTAIN CLEAN, STRAIGHT CUT EDGE UNTIL NEW PAVING IS PLACED.
  2. REMOVE AND SALVAGE EXISTING PARKING BUMPER FOR REINSTALLATION.
  3. CUT EXISTING POST FLUSH WITH EXISTING CONCRETE AND GROUT FILL POST HOLE. REMOVE AND SALVAGE EXISTING SIGNS.
  4. "BLACK OUT" EXISTING STRIPING.
  5. REMOVE AND DISPOSE OF EXISTING DRINKING FOUNTAIN. CAP EXISTING SEWER/WATER PIPES BELOW GRADE.
  14. REMOVE AND DISPOSE OF EXISTING POUR IN PLACE RUBBER SURFACE AND CUSHION PAD.

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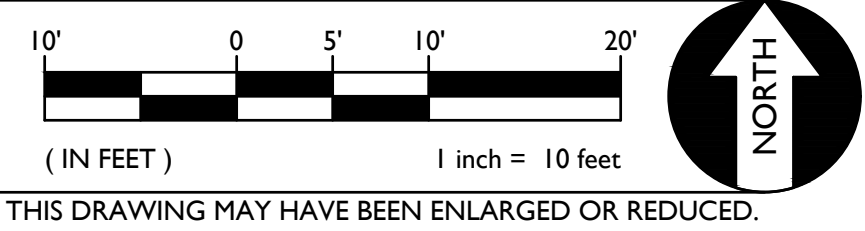
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**DEMOLITION PLAN**

GRAPHIC SCALE



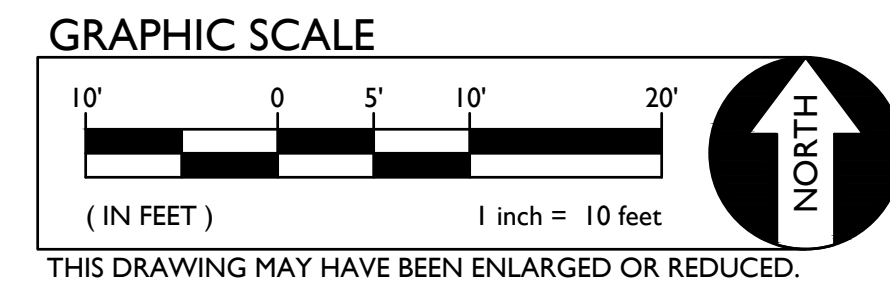
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- DEMOLITION NOTES**
1. SAWCUT, REMOVE AND DISPOSE OF EXISTING ASPHALT PAVING AND ASSOCIATED AGGREGATE BASE. SAWCUT SHALL BE A NEAT STRAIGHT LINE, MAINTAIN CLEAN, STRAIGHT CUT EDGE UNTIL NEW PAVING IS PLACED.
  5. REMOVE AND DISPOSE OF EXISTING DRINKING FOUNTAIN. CAP EXISTING SEWER/WATER PIPES BELOW GRADE.
  6. SAWCUT, REMOVE AND DISPOSE OF EXISTING CONCRETE PAVING TO NEAREST JOINT AND ASSOCIATED AGGREGATE BASE. SAWCUT SHALL BE A NEAT STRAIGHT LINE, MAINTAIN CLEAN, STRAIGHT CUT EDGE UNTIL NEW PAVING IS PLACED.
  7. REMOVE AND DISPOSE OF EXISTING LANDSCAPING, TURF AND ASSOCIATED IRRIGATION PIPING/SPRINKLERS WITHIN AREAS OF WORK. CUT AND CAP ANY MAINLINES NEAR WHERE THEY ENTER THE BOUNDARY OF THE PROJECT. MARK ALL CAPPED LINES WITH AN IRRIGATION VALVE BOX. ALL EXISTING IRRIGATION AREAS OUTSIDE THE PROJECT WORK AREA SHALL BE PRESERVED AND OPERATIONAL. INTEGRITY SHALL BE MAINTAINED WITH PROPER SPRINKLER COVERAGE TO TURF AREAS TO REMAIN.
  8. REMOVE AND DISPOSE OF EXISTING BARK MATERIAL AND PLAY APPARATUS. CONFIRM WITH DISTRICT IF PLAY APPARATUS IS TO BE RETURNED TO DISTRICT.
  9. REMOVE AND DISPOSE OF EXISTING CONCRETE CURB.
  10. REMOVE AND SALVAGE EXISTING FENCE MESH AND RAILS FOR REINSTALLATION. REMOVE AND DISPOSE OF EXISTING POSTS AND ASSOCIATED FOOTINGS.
  11. EXISTING TREE TO REMAIN.
  12. REMOVE AND DISPOSE OF EXISTING AREA DRAIN.
  13. REMOVE AND DISPOSE OF EXISTING BENCH.
  15. REMOVE AND DISPOSE OF EXISTING SIGN, POST AND ASSOCIATED FOOTING.



FILENAME: I:\23-115\QVLDWG\23-115-011-C12.DWG

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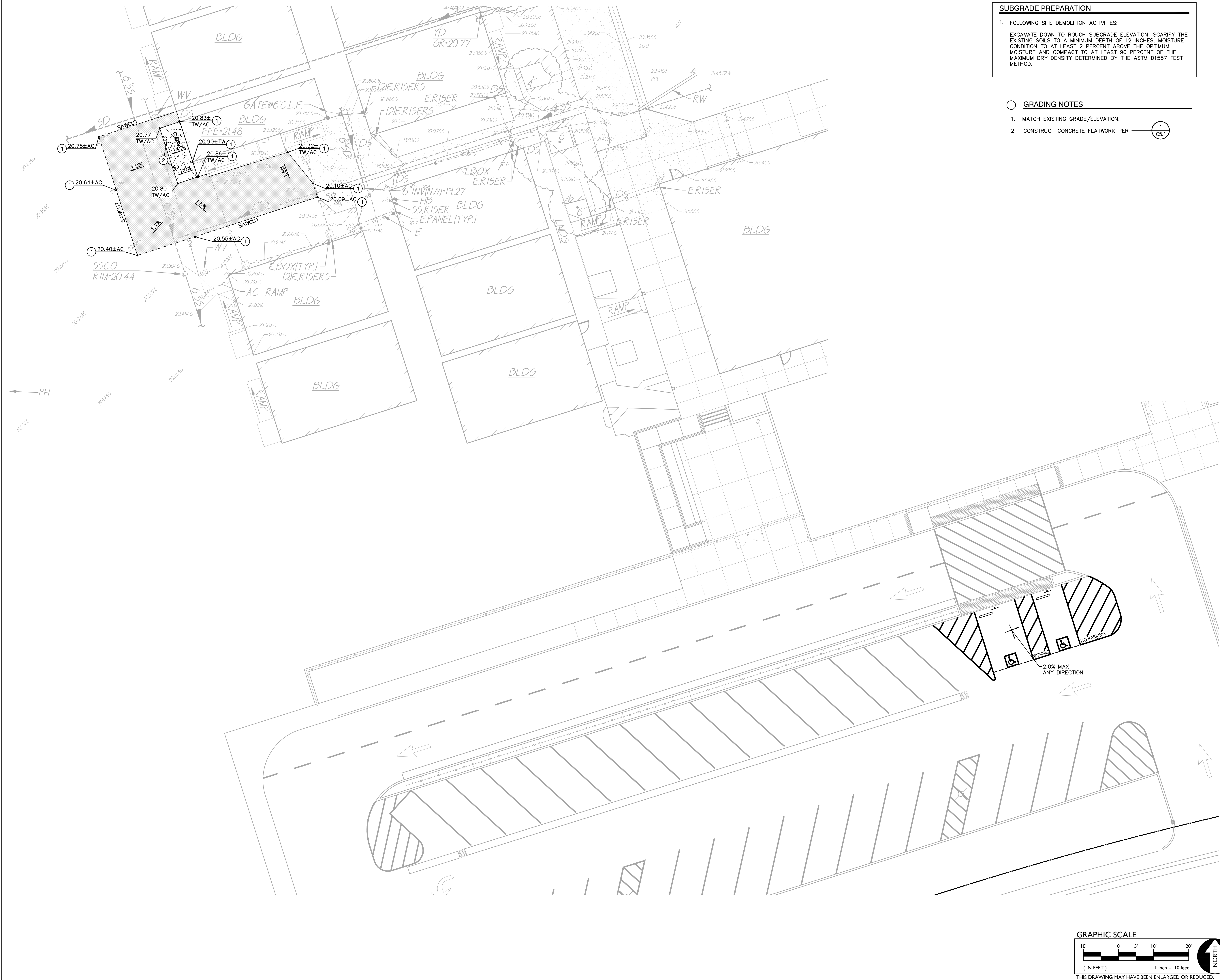
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**DEMOLITION PLAN**

Date 11/20/2023	Project Number 22042
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**C1.2**





SUBGRADE PREPARATION

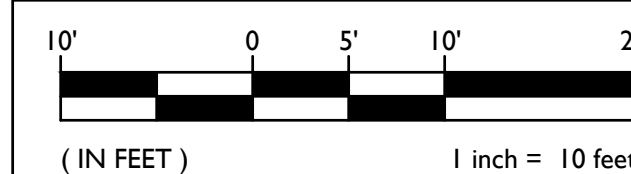
1. FOLLOWING SITE DEMOLITION ACTIVITIES:

EXCAVATE DOWN TO ROUGH SUBGRADE ELEVATION. SCARIFY THE EXISTING SOILS TO A MINIMUM DEPTH OF 12 INCHES. MOISTURE CONDITION TO AT LEAST 2 PERCENT ABOVE THE OPTIMUM MOISTURE AND COMPACT TO AT LEAST 90 PERCENT OF THE MAXIMUM DRY DENSITY DETERMINED BY THE ASTM D1557 TEST METHOD.

GRADING NOTES

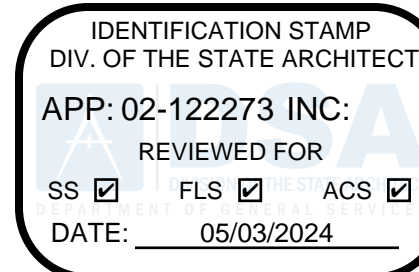
- MATCH EXISTING GRADE/ELEVATION.
- CONSTRUCT CONCRETE FLATWORK PER 1  
C5.1

GRAPHIC SCALE



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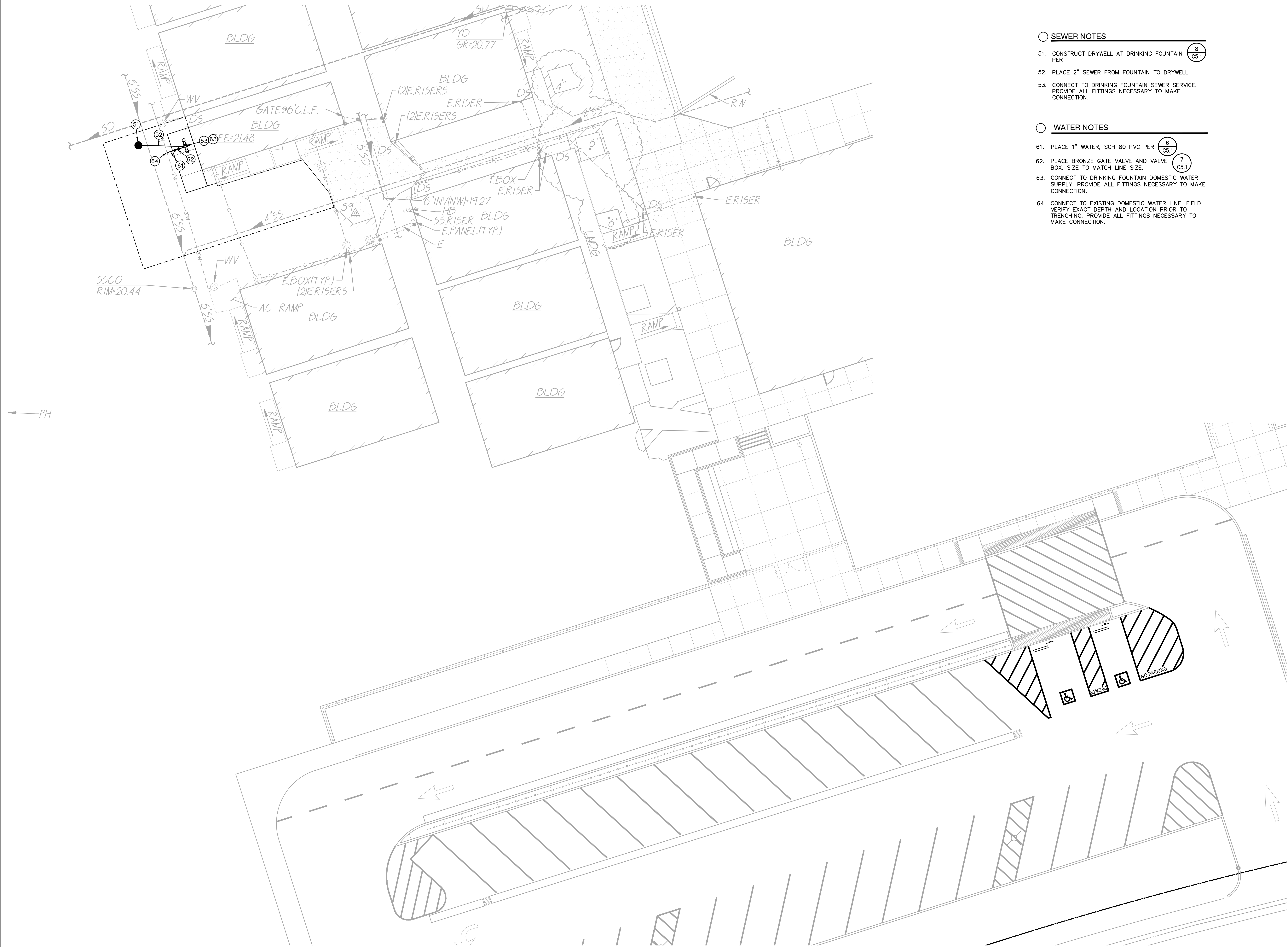
GRADING PLAN

Date 11/20/2023	Project Number 22042
Application Number -	Drawing Number C2.1
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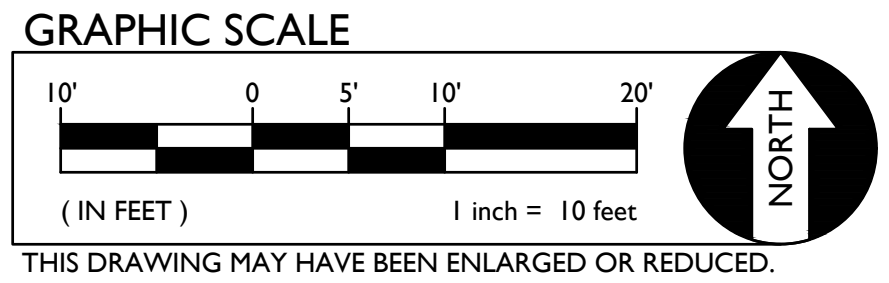








- SEWER NOTES**
- CONSTRUCT DRYWELL AT DRINKING FOUNTAIN PER
  - PLACE 2" SEWER FROM FOUNTAIN TO DRYWELL.
  - CONNECT TO DRINKING FOUNTAIN SEWER SERVICE. PROVIDE ALL FITTINGS NECESSARY TO MAKE CONNECTION.
- WATER NOTES**
- PLACE 1" WATER, SCH 80 PVC PER
  - PLACE BRONZE GATE VALVE AND VALVE BOX. SIZE TO MATCH LINE SIZE.
  - CONNECT TO DRINKING FOUNTAIN DOMESTIC WATER SUPPLY. PROVIDE ALL FITTINGS NECESSARY TO MAKE CONNECTION.
  - CONNECT TO EXISTING DOMESTIC WATER LINE. FIELD VERIFY EXACT DEPTH AND LOCATION PRIOR TO TRENCHING. PROVIDE ALL FITTINGS NECESSARY TO MAKE CONNECTION.



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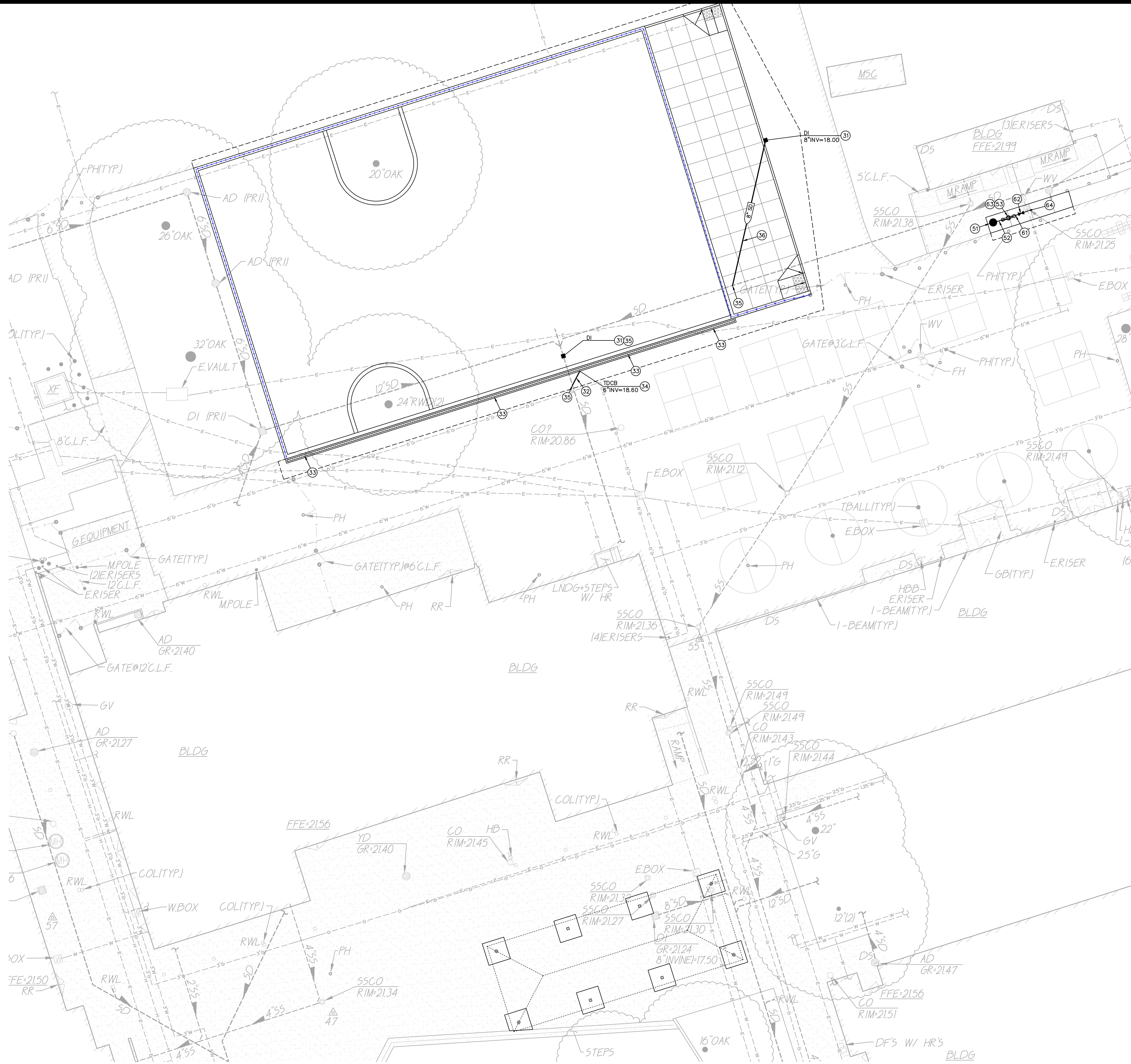
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UTILITY PLAN

Date 11/20/2023	Project Number 22042
Application Number -	Drawing Number C3.1
Drawn AT	Checked AT

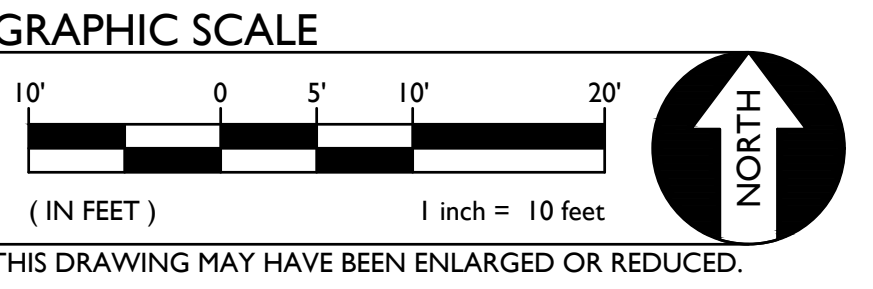




- DRAINAGE NOTES**
- 31. CONSTRUCT DROP INLET PER 3 C5.1
  - 32. PLACE 6" STORM DRAIN PER 4 C5.1
  - 33. CONSTRUCT TRENCH DRAIN PER 5 C5.1
  - 34. CONSTRUCT TRENCH DRAIN CATCH BASIN PER 4 C5.1
  - 35. CONNECT TO EXISTING STORM DRAIN. FIELD VERIFY EXACT DEPTH AND LOCATION PRIOR TO TRENCHING. PROVIDE ALL FITTINGS NECESSARY TO MAKE CONNECTION.
  - 36. PLACE 8" STORM DRAIN PER 4 C5.1

- SEWER NOTES**
- 51. CONSTRUCT DRYWELL AT DRINKING FOUNTAIN PER 8 C5.1
  - 52. PLACE 2" SEWER FROM FOUNTAIN TO DRYWELL.
  - 53. CONNECT TO DRINKING FOUNTAIN SEWER SERVICE. PROVIDE ALL FITTINGS NECESSARY TO MAKE CONNECTION.

- WATER NOTES**
- 61. PLACE 1" WATER, SCH 80 PVC PER 6 C5.1
  - 62. PLACE BRONZE GATE VALVE AND VALVE BOX. SIZE TO MATCH LINE SIZE.
  - 63. CONNECT TO DRINKING FOUNTAIN DOMESTIC WATER SUPPLY. PROVIDE ALL FITTINGS NECESSARY TO MAKE CONNECTION.
  - 64. CONNECT TO EXISTING DOMESTIC WATER LINE. FIELD VERIFY EXACT DEPTH AND LOCATION PRIOR TO TRENCHING. PROVIDE ALL FITTINGS NECESSARY TO MAKE CONNECTION.



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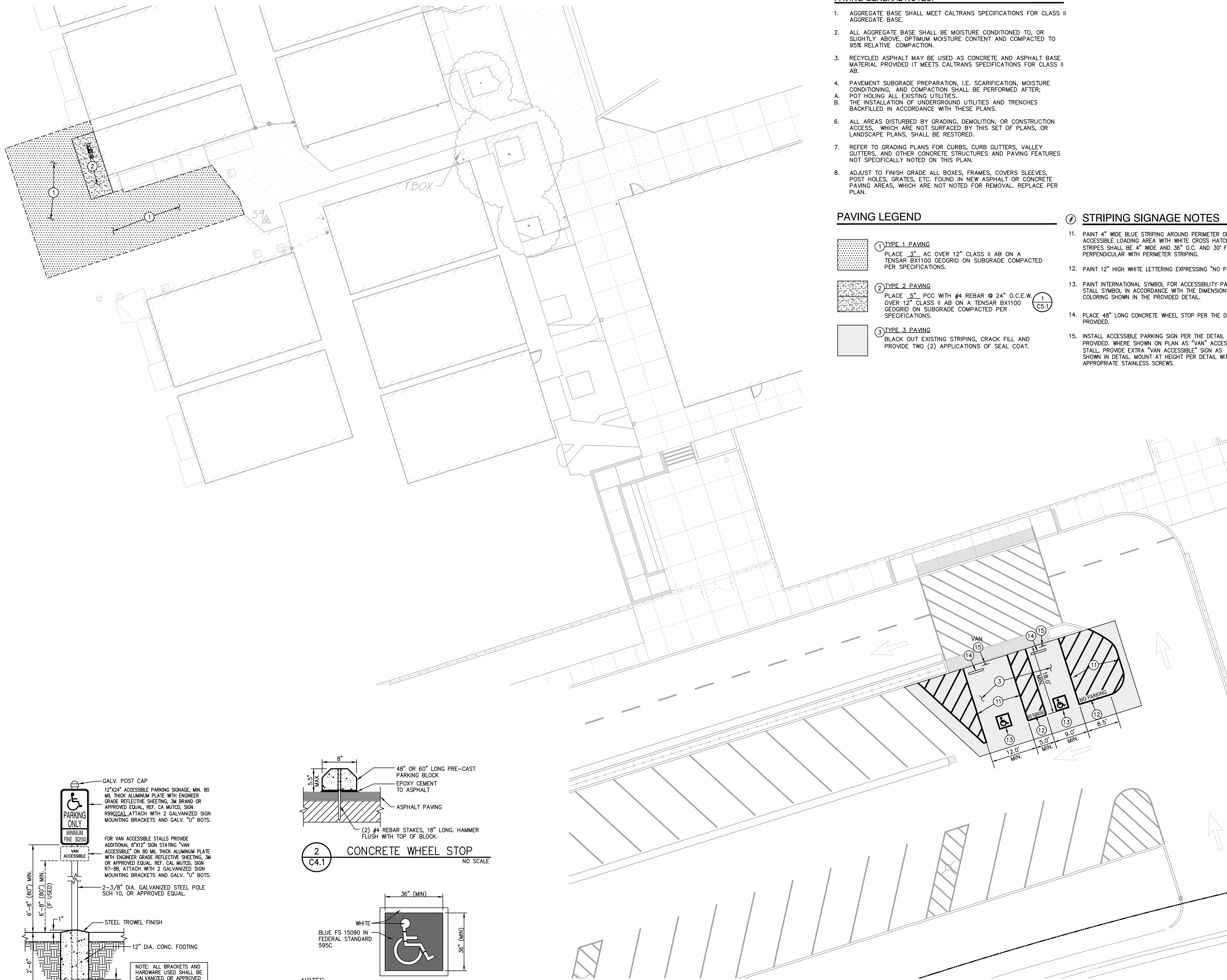
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UTILITY PLAN

Date 11/20/2023	Project Number 22042
Application Number .	Drawing Number <b>C3.2</b>
Drawn AT	Checked AT

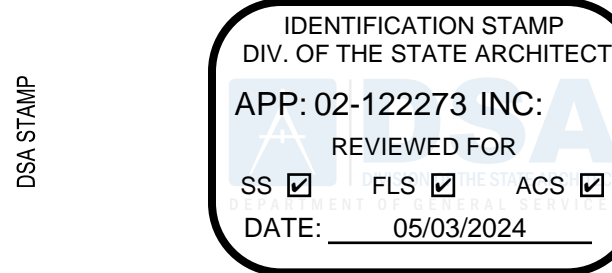
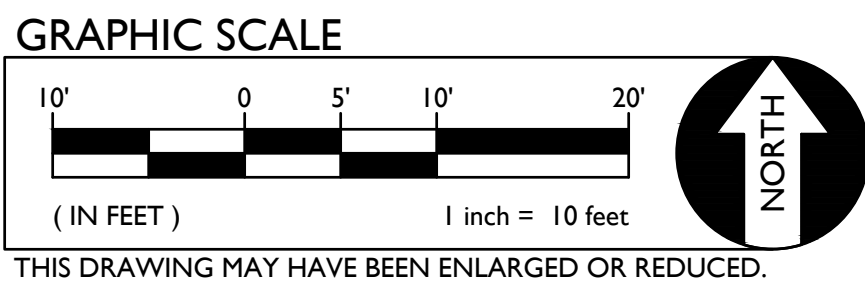
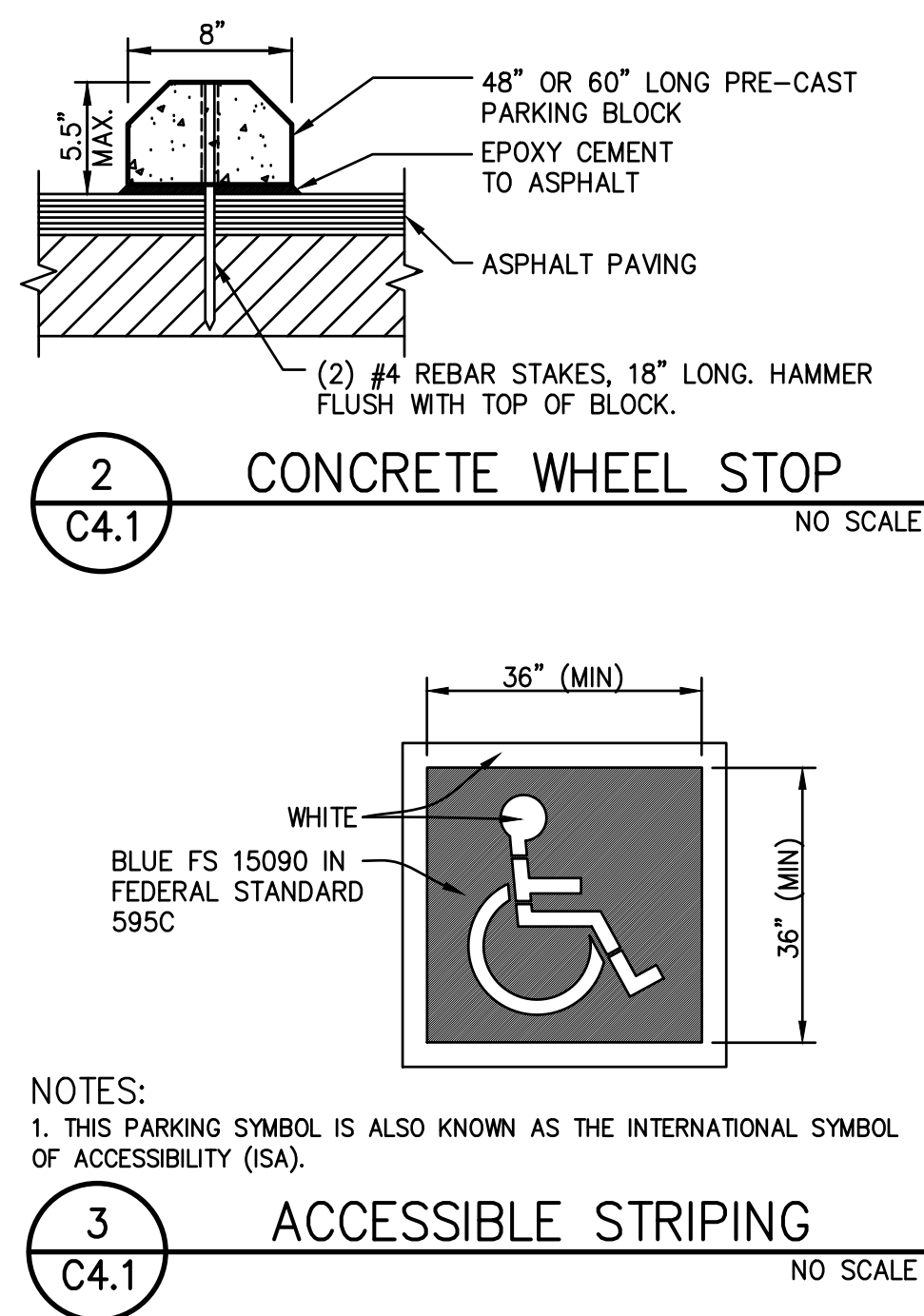
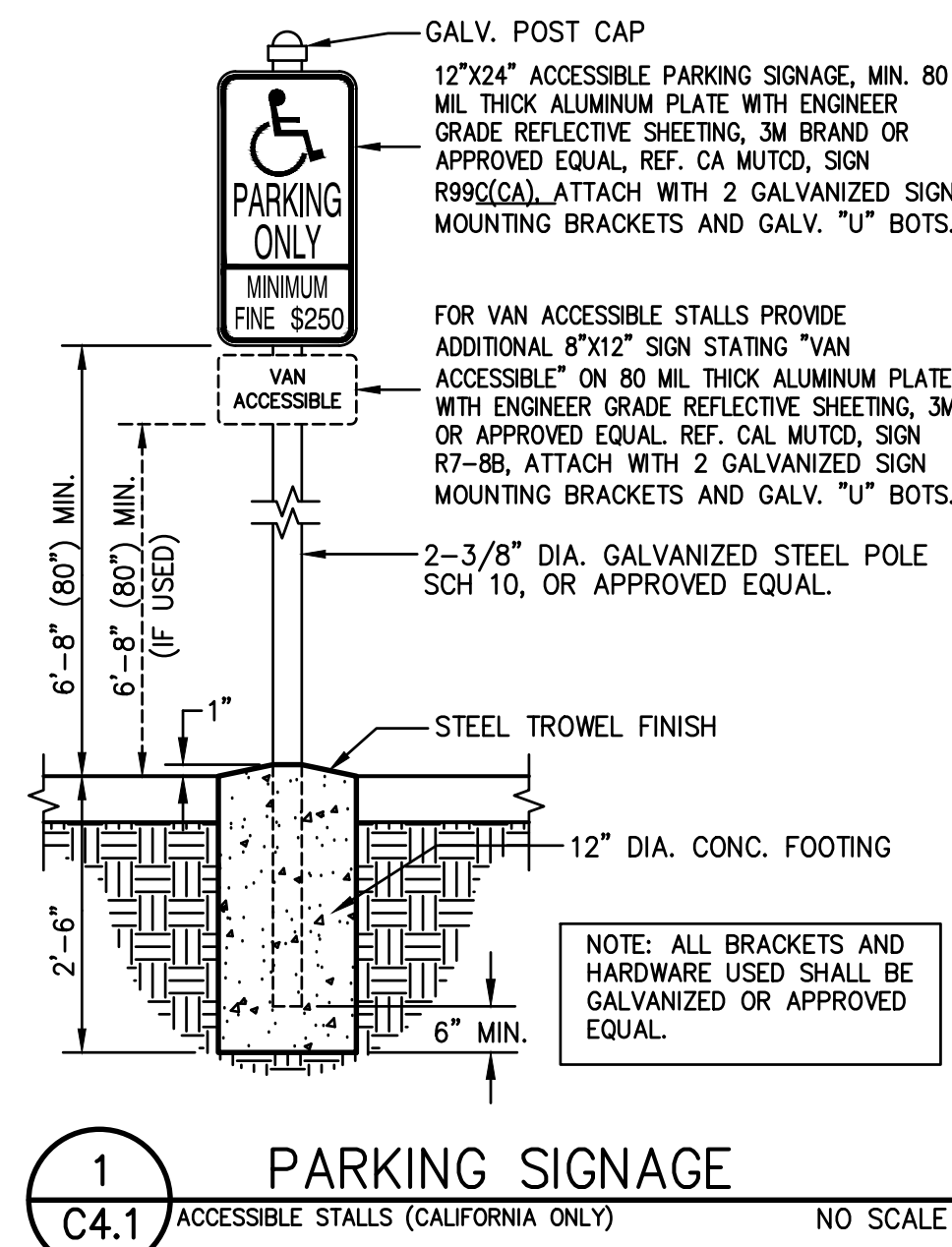




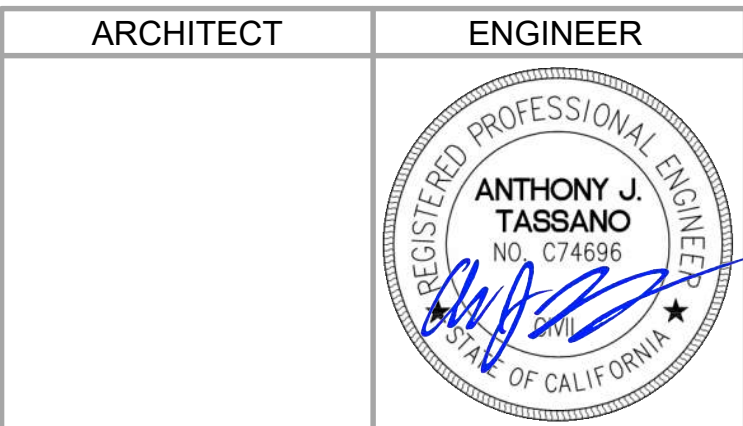
- PAVING GENERAL NOTES:**
1. AGGREGATE BASE SHALL MEET CALTRANS SPECIFICATIONS FOR CLASS II AGGREGATE BASE.
  2. ALL AGGREGATE BASE SHALL BE MOISTURE CONDITIONED TO, OR SLIGHTLY ABOVE, OPTIMUM MOISTURE CONTENT AND COMPACTED TO 95% RELATIVE COMPACTION.
  3. RECYCLED ASPHALT MAY BE USED AS CONCRETE AND ASPHALT BASE MATERIAL PROVIDED IT MEETS CALTRANS SPECIFICATIONS FOR CLASS II AB.
  4. PAVEMENT SUBGRADE PREPARATION, I.E. SCARIFICATION, MOISTURE CONDITIONING, AND COMPACTION SHALL BE PERFORMED AFTER;  
A. POT HOLES, ALL EXISTING UTILITIES.  
B. THE INSTALLATION OF UNDERGROUND UTILITIES AND TRENCHES BACKFILLED IN ACCORDANCE WITH THESE PLANS.
  6. ALL AREAS DISTURBED BY GRADING, DEMOLITION, OR CONSTRUCTION ACCESS, WHICH ARE NOT SURFACED BY THIS SET OF PLANS, OR LANDSCAPE PLANS, SHALL BE RESTORED.
  7. REFER TO GRADING PLANS FOR CURBS, CURB GUTTERS, VALLEY GUTTERS, AND OTHER CONCRETE STRUCTURES AND PAVING FEATURES NOT SPECIFICALLY NOTED ON THIS PLAN.
  8. ADJUST TO FINISH GRADE ALL BOXES, FRAMES, COVERS SLEEVES, POST HOLES, GRATES, ETC. FOUND IN NEW ASPHALT OR CONCRETE PAVING AREAS, WHICH ARE NOT NOTED FOR REMOVAL. REPLACE PER PLAN.

- PAVING LEGEND**
- ① **TYPE 1 PAVING**  
PLACE 3" AC OVER 12" CLASS II AB ON A TENSAR BX1100 GEOGRID ON SUBGRADE COMPACTED PER SPECIFICATIONS.
  - ② **TYPE 2 PAVING**  
PLACE 5" PCC WITH #4 REBAR @ 24" O.C.E.W. OVER 12" CLASS II AB ON A TENSAR BX1100 GEOGRID ON SUBGRADE COMPACTED PER SPECIFICATIONS.
  - ③ **TYPE 3 PAVING**  
BLACK OUT EXISTING STRIPING, CRACK FILL AND PROVIDE TWO (2) APPLICATIONS OF SEAL COAT.

- STRIPING SIGNAGE NOTES**
11. PAINT 4" WIDE BLUE STRIPING AROUND PERIMETER OF ACCESSIBLE LOADING AREA WITH WHITE CROSS HATCH STRIPING. STRIPES SHALL BE 4" WIDE AND 36" O.C. AND 30" FROM PERPENDICULAR WITH PERIMETER STRIPING.
  12. PAINT 12" HIGH WHITE LETTERING EXPRESSING "NO PARKING".
  13. PAINT INTERNATIONAL SYMBOL FOR ACCESSIBILITY PARKING STALL SYMBOL IN ACCORDANCE WITH THE DIMENSIONS AND COLORING SHOWN IN THE PROVIDED DETAIL.
  14. PLACE 48" LONG CONCRETE WHEEL STOP PER THE DETAIL PROVIDED.
  15. INSTALL ACCESSIBLE PARKING SIGN PER THE DETAIL PROVIDED. WHERE SHOWN ON PLAN AS "VAN" ACCESSIBLE STALL, PROVIDE EXTRA "VAN ACCESSIBLE" SIGN AS SHOWN IN DETAIL. MOUNT AT HEIGHT PER DETAIL WITH APPROPRIATE STAINLESS SCREWS.



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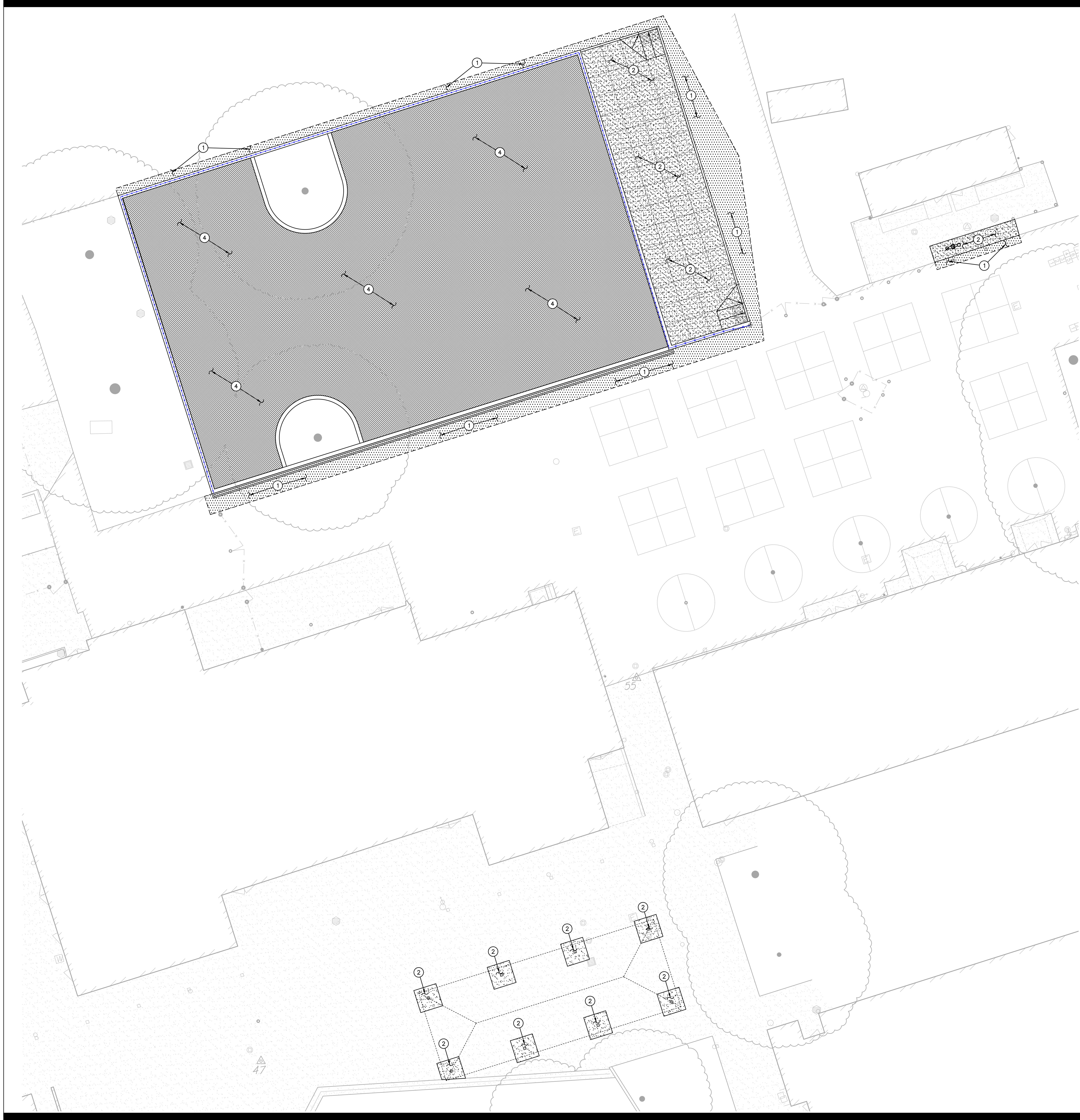
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PAVING AND  
STRIPING PLAN

Date 11/20/2023	Project Number 22042
Application Number -	Drawing Number C4.1
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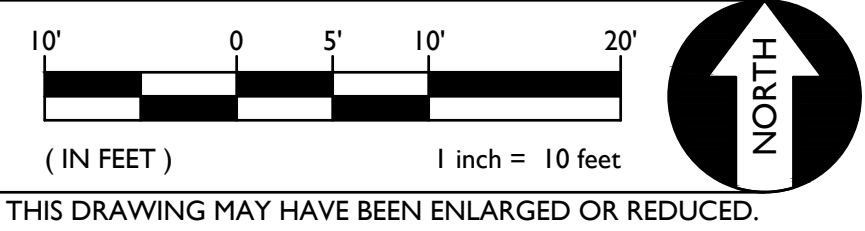
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3. RECYCLED ASPHALT MAY BE USED AS CONCRETE AND ASPHALT BASE MATERIAL PROVIDED IT MEETS CALTRANS SPECIFICATIONS FOR CLASS II AB.
4. PAVEMENT SUBGRADE PREPARATION, I.E. SCARIFICATION, MOISTURE CONDITIONING, AND COMPACTION SHALL BE PERFORMED AFTER;  
A. POT HOLE ALL EXISTING UTILITIES.  
B. THE INSTALLATION OF UNDERGROUND UTILITIES AND TRENCHES BACKFILLED IN ACCORDANCE WITH THESE PLANS.
6. ALL AREAS DISTURBED BY GRADING, DEMOLITION, OR CONSTRUCTION ACCESS, WHICH ARE NOT SURFACED BY THIS SET OF PLANS, OR LANDSCAPE PLANS, SHALL BE RESTORED.
7. REFER TO GRADING PLANS FOR CURBS, CURB GUTTERS, VALLEY GUTTERS, AND OTHER CONCRETE STRUCTURES AND PAVING FEATURES NOT SPECIFICALLY NOTED ON THIS PLAN.
8. ADJUST TO FINISH GRADE ALL BOXES, FRAMES, COVERS SLEEVES, POST HOLES, GRATES, ETC. FOUND IN NEW ASPHALT OR CONCRETE PAVING AREAS, WHICH ARE NOT NOTED FOR REMOVAL. REPLACE PER PLAN.

PAVING LEGEND

- ① TYPE 1 PAVING**  
PLACE 3" AC OVER 12" CLASS II AB ON A TENSAR BX1100 GEOGRID ON SUBGRADE COMPACTED PER SPECIFICATIONS.
- ② TYPE 2 PAVING**  
PLACE 5" PCC WITH #4 REBAR @ 24" O.C.E.W. OVER 12" CLASS II AB ON A TENSAR BX1100 GEOGRID ON SUBGRADE COMPACTED PER SPECIFICATIONS.
- ④ TYPE 4 PAVING**  
PLACE 1/2" POUR IN PLACE RUBBER WEAR COURSE OVER 3" SBR CUSHION LAYER ON 12" OF CL2 AGGREGATE BASE ON A TENSAR BX1100 GEOGRID ON SUBGRADE COMPACTED PER SPECIFICATIONS.
- NOTE: POUR IN PLACE SURFACING TO COMPLY WITH 11B-1008.2.6 FOR ACCESSIBILITY AND USE ZONES.

GRAPHIC SCALE



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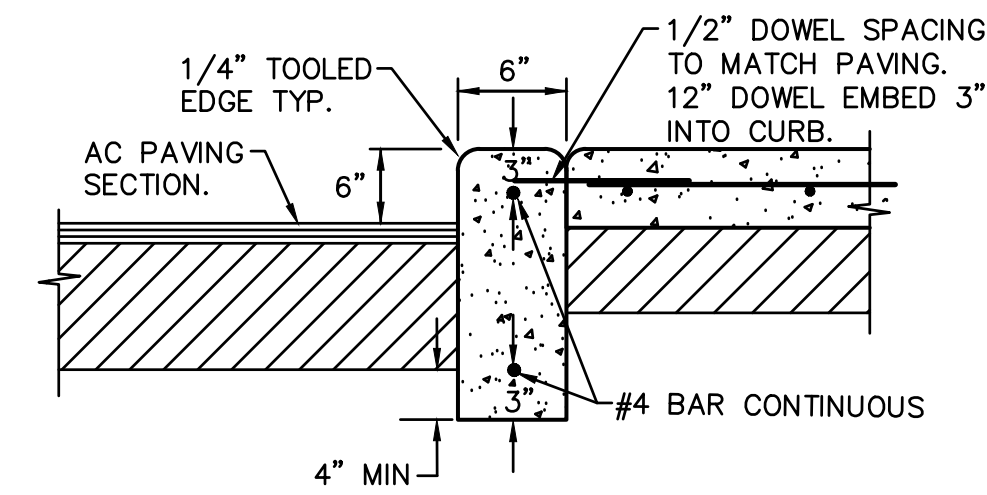
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PAVING AND  
STRIPING PLAN

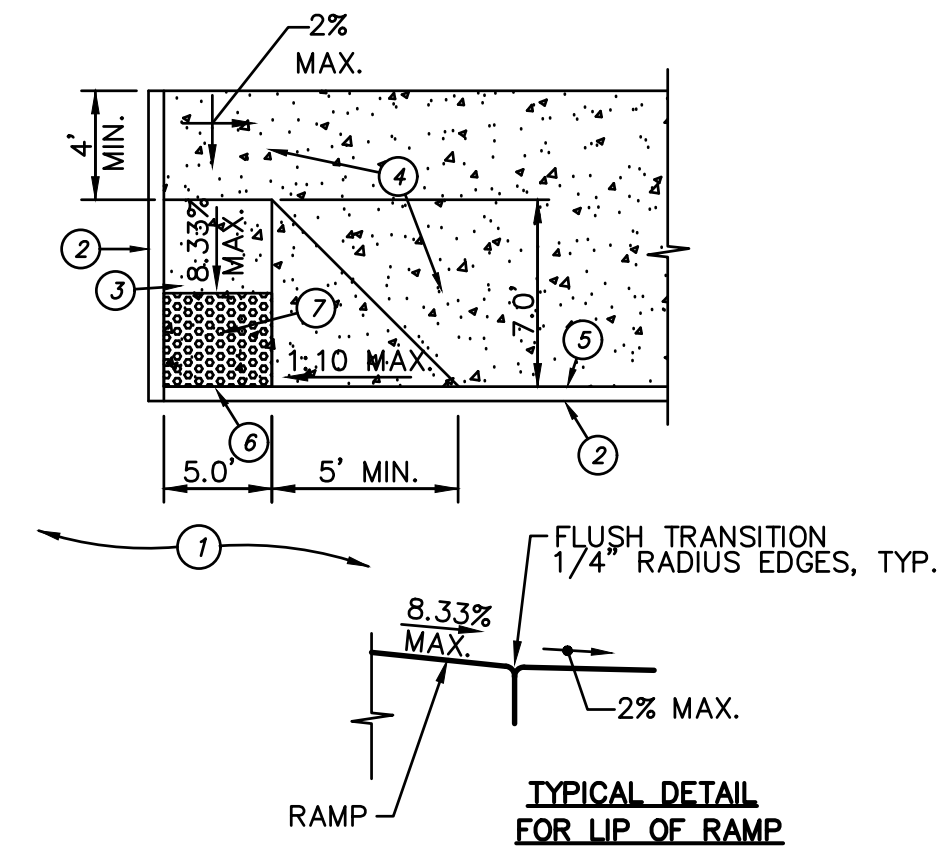
Date 11/20/2023	Project Number 22042
Application Number -	Drawing Number C4.2
Drawn AT	Checked AT





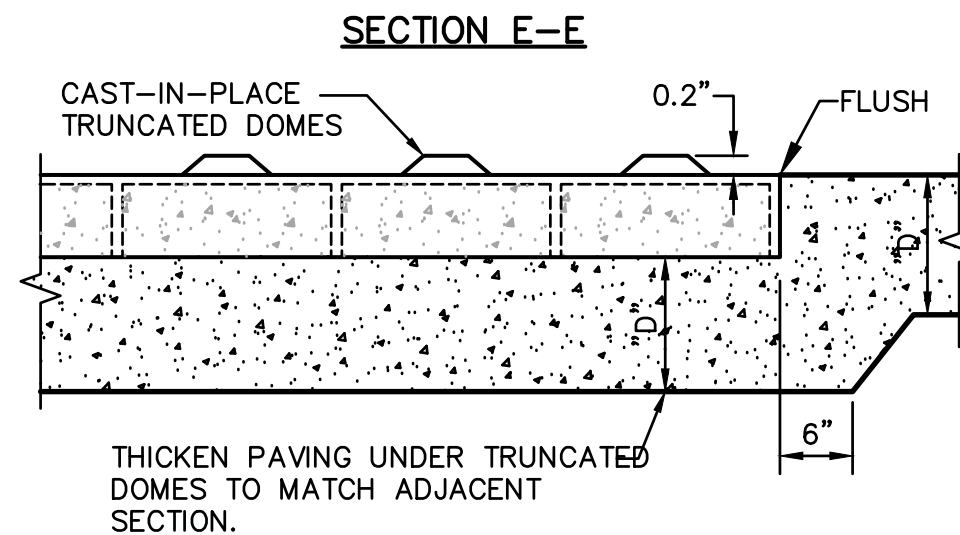
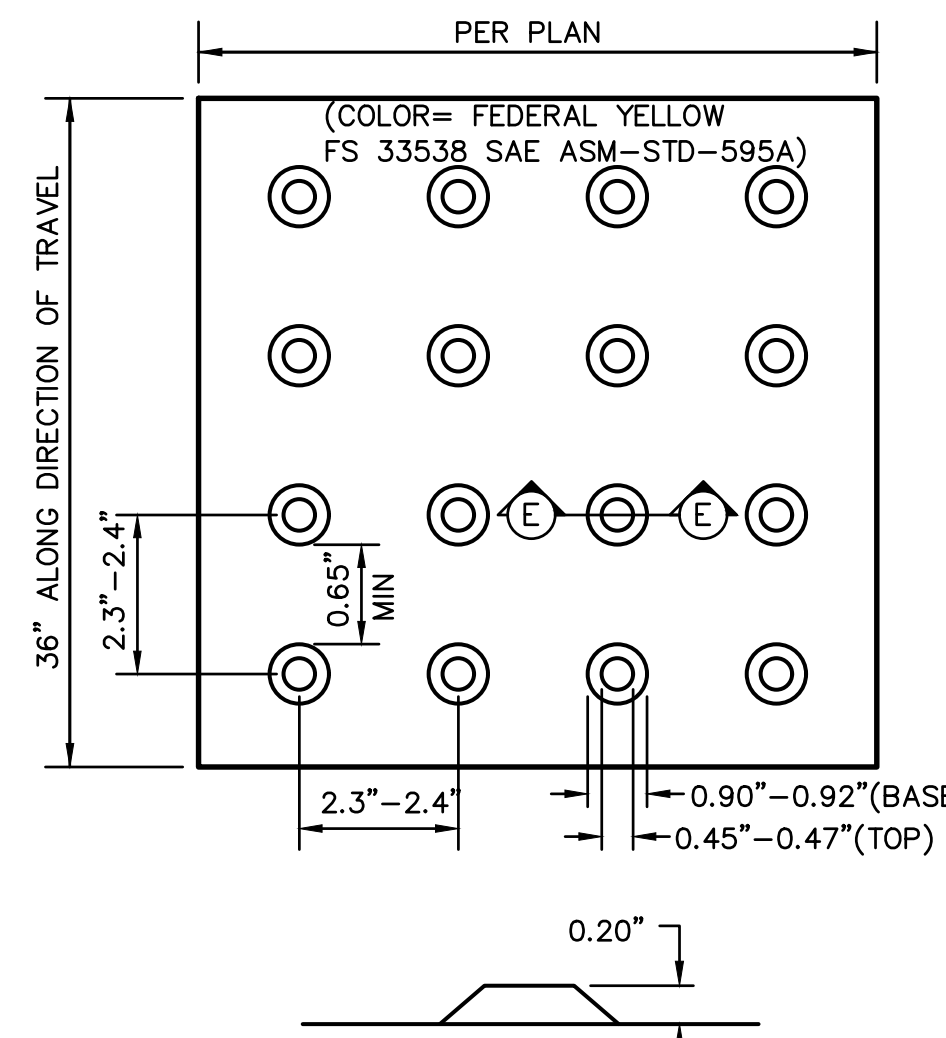
- NOTES:
1. PROVIDE FELT EXPANSION JOINTS (E.J.) AT 60 FEET O.C. MAXIMUM PROVIDE CONTROL JOINTS AT 10 FEET O.C. MAXIMUM, EXCEPT WHEN PLACING ADJACENT TO CONCRETE WALKS THE EXPANSION JOINTS SHALL ALIGN WITH THE EXPANSION JOINTS SHOWN FOR THE CONCRETE WALKS.
  2. AT E.J. USE 1/2"x24" SMOOTH DOWELS, ALIGN WITH REBAR, GREASE 1/2 THE LENGTH BEFORE CONCRETE PLACEMENT.

9 CONCRETE CURB  
C5.1 NO SCALE

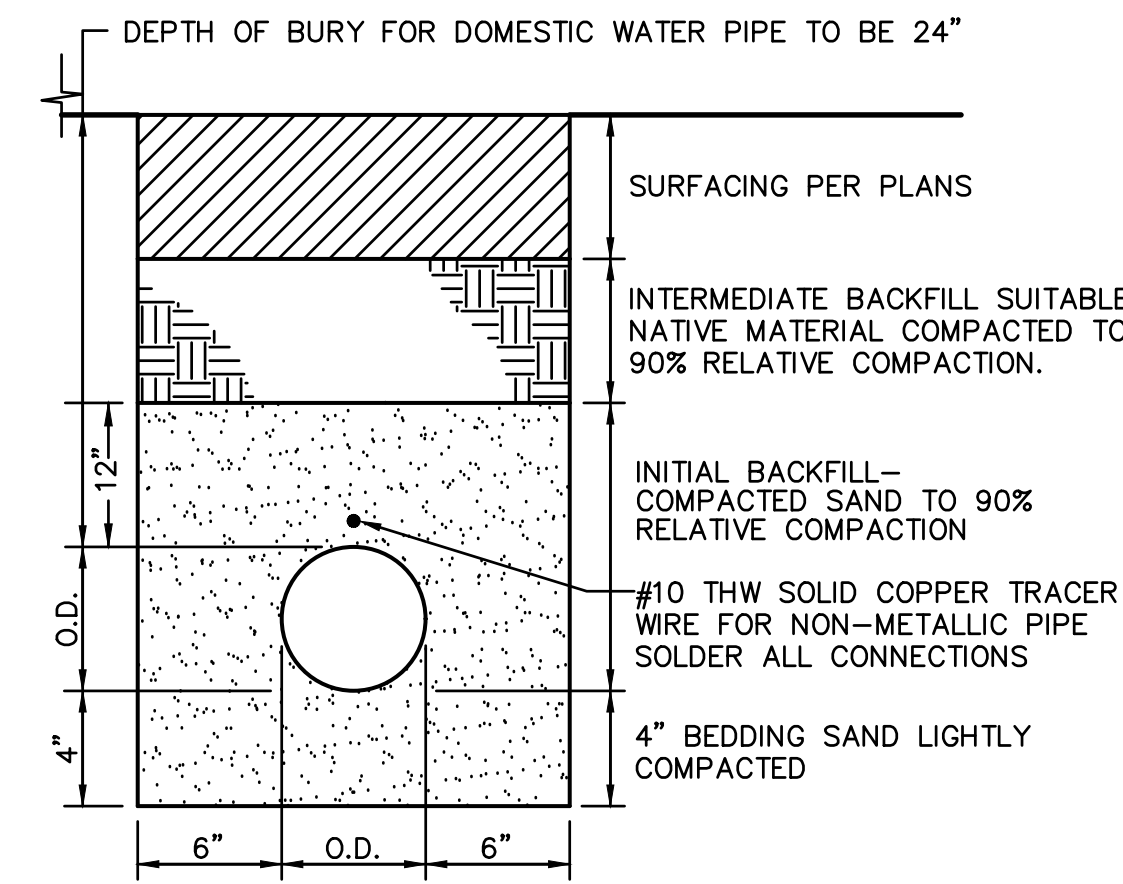


- LEGEND
1. PAVEMENT.
  2. TOP FACE OF ROLLED CURB, STANDARD 4" HIGH.
  3. 8.33% (1:12) MAXIMUM SLOPE, 2% MAX CROSS SLOPE LENGTH=6.5'.
  4. LANDING SHALL BE 48" MIN. AND SLOPE SHALL NOT EXCEED 1:48 MAX PER CALIFORNIA BUILDING CODE, TITLE 24, SECTION 11B-406.5.3
  5. SCORE MARK, 6" BACK OF CURB.
  6. TRANSITION SHALL BE FLUSH AND FREE OF ABRUPT CHANGE WITH A COUNTER SLOPE NOT EXCEEDING 1:20 MAX FOR 24" PER CALIFORNIA BUILDING CODE, TITLE 24, SECTION 11B-406.5.8.
  7. PLACE 36" WIDE PREFABRICATED CAST IN PLACE DETECTABLE WARNING TILE BY ARMOR-TILE OR APPROVED EQUAL. DETECTABLE WARNINGS AT CURB RAMP SHALL EXTEND 36 INCHES IN THE DIRECTION OF TRAVEL. DETECTABLE WARNINGS SHALL EXTEND THE FULL WIDTH OF THE RAMP RUN LESS 2 INCHES MAXIMUM ON EACH SIDE, EXCLUDING ANY FLARED SIDES. DETECTABLE WARNINGS SHALL BE LOCATED SO THE EDGE NEAREST THE CURB IS 6 INCHES MINIMUM AND 8 INCHES MAXIMUM FROM THE LINE AT THE FACE OF THE CURB MARKING THE TRANSITION BETWEEN THE CURB AND THE GUTTER OR STREET.

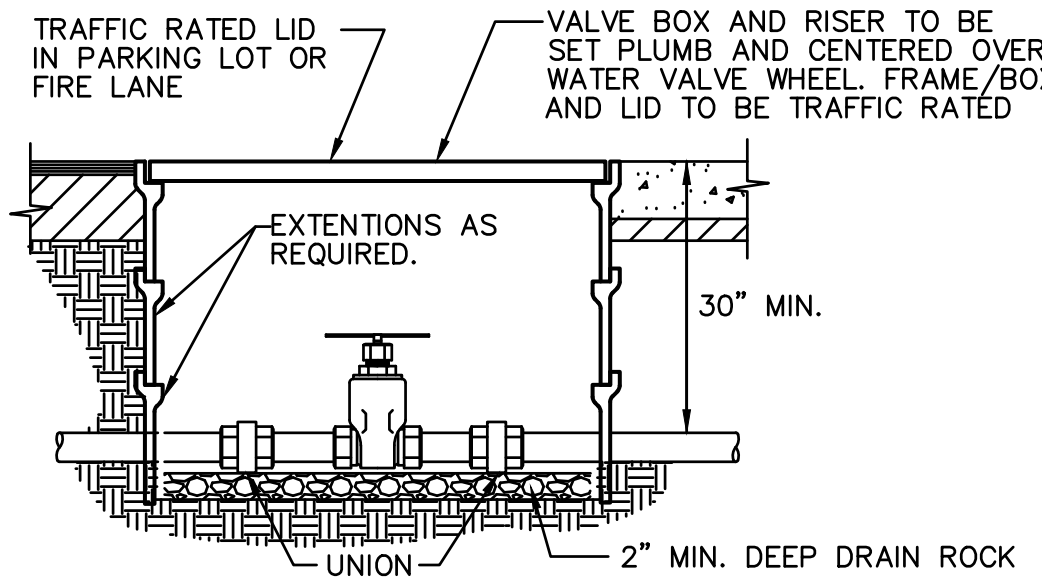
10 ACCESSIBLE CURB RAMP  
C5.1 NO SCALE



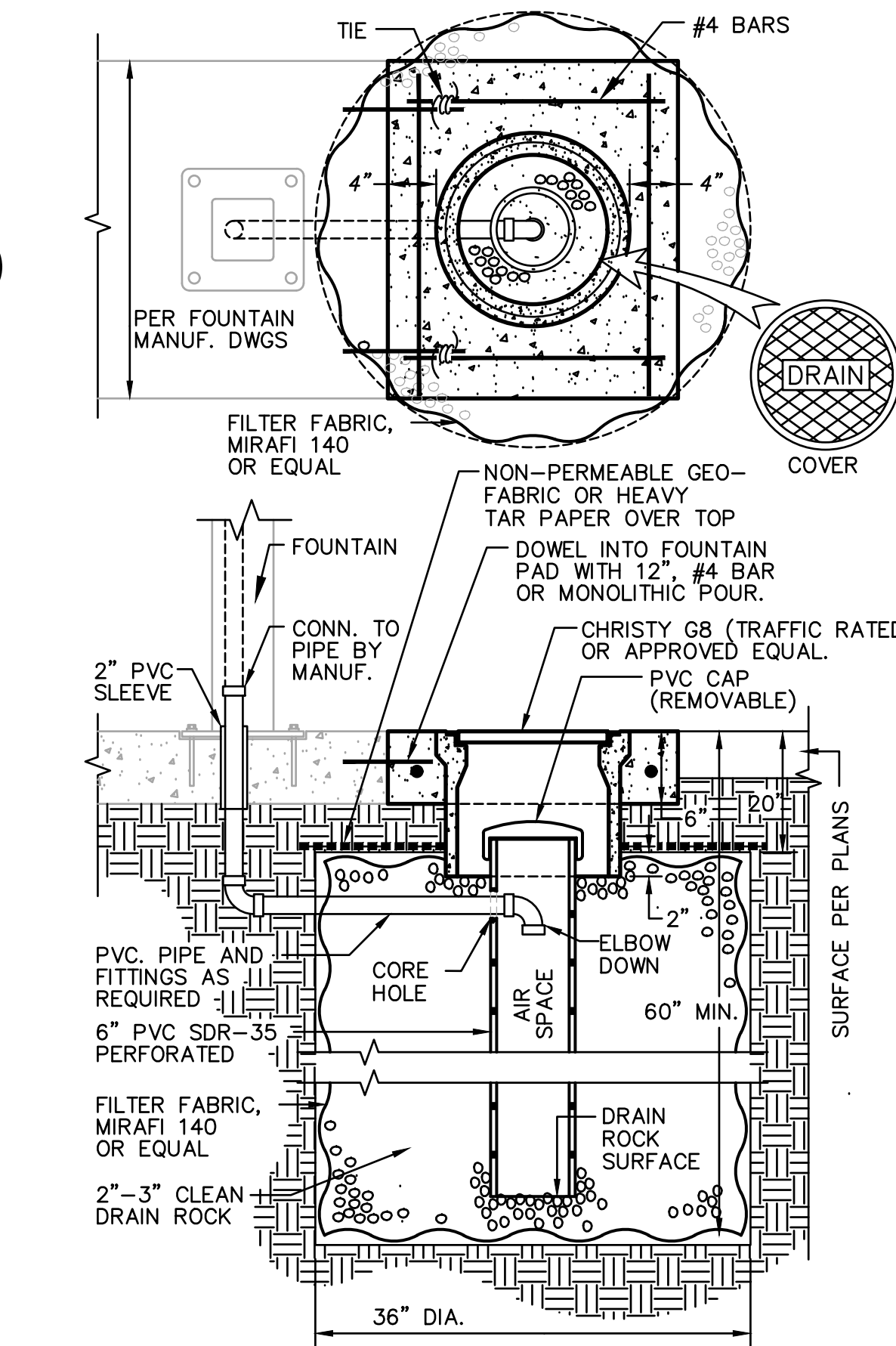
11 TRUNCATED DOMES  
C5.1 NO SCALE



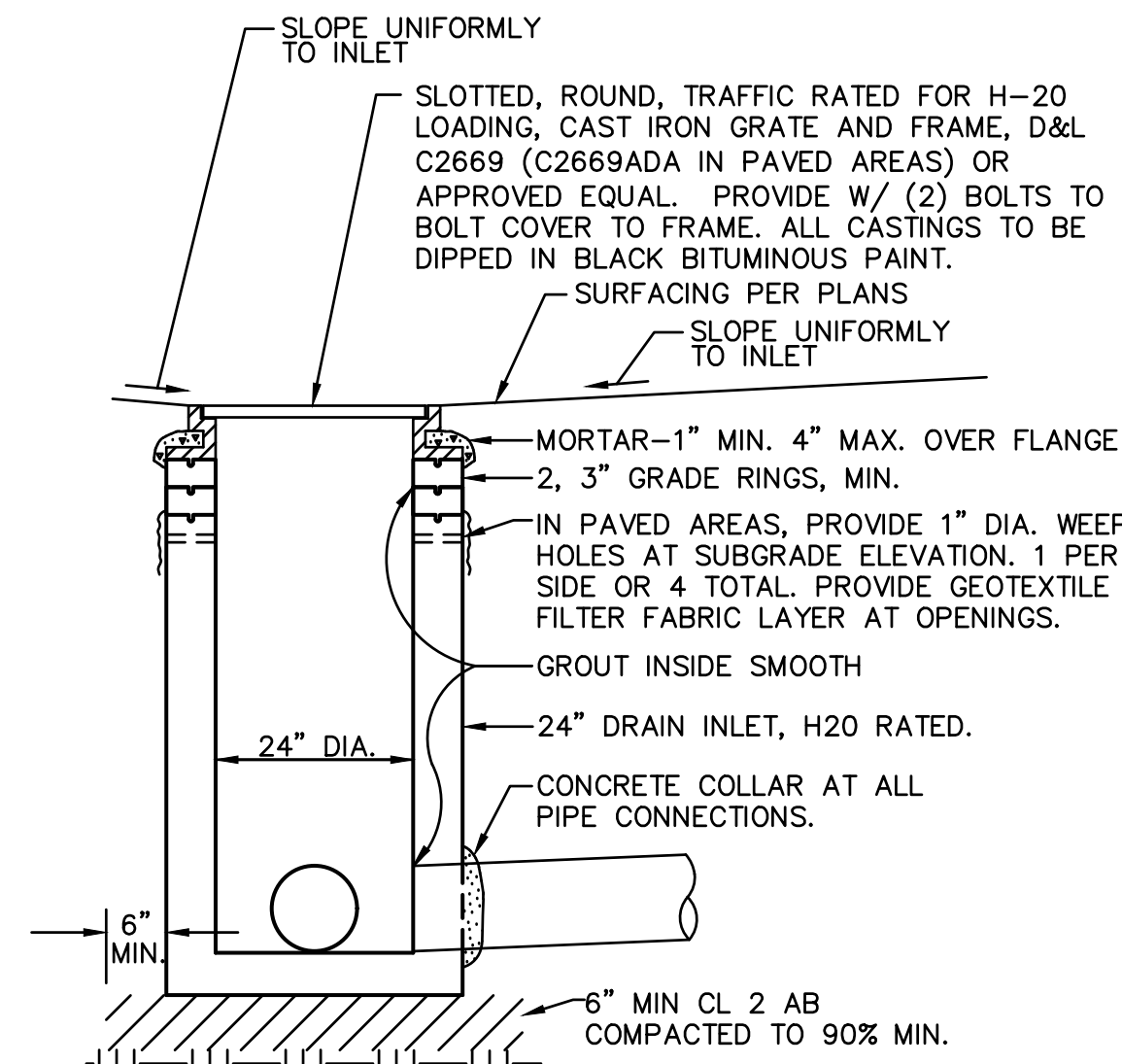
6 WATER TRENCH  
C5.1 NO SCALE



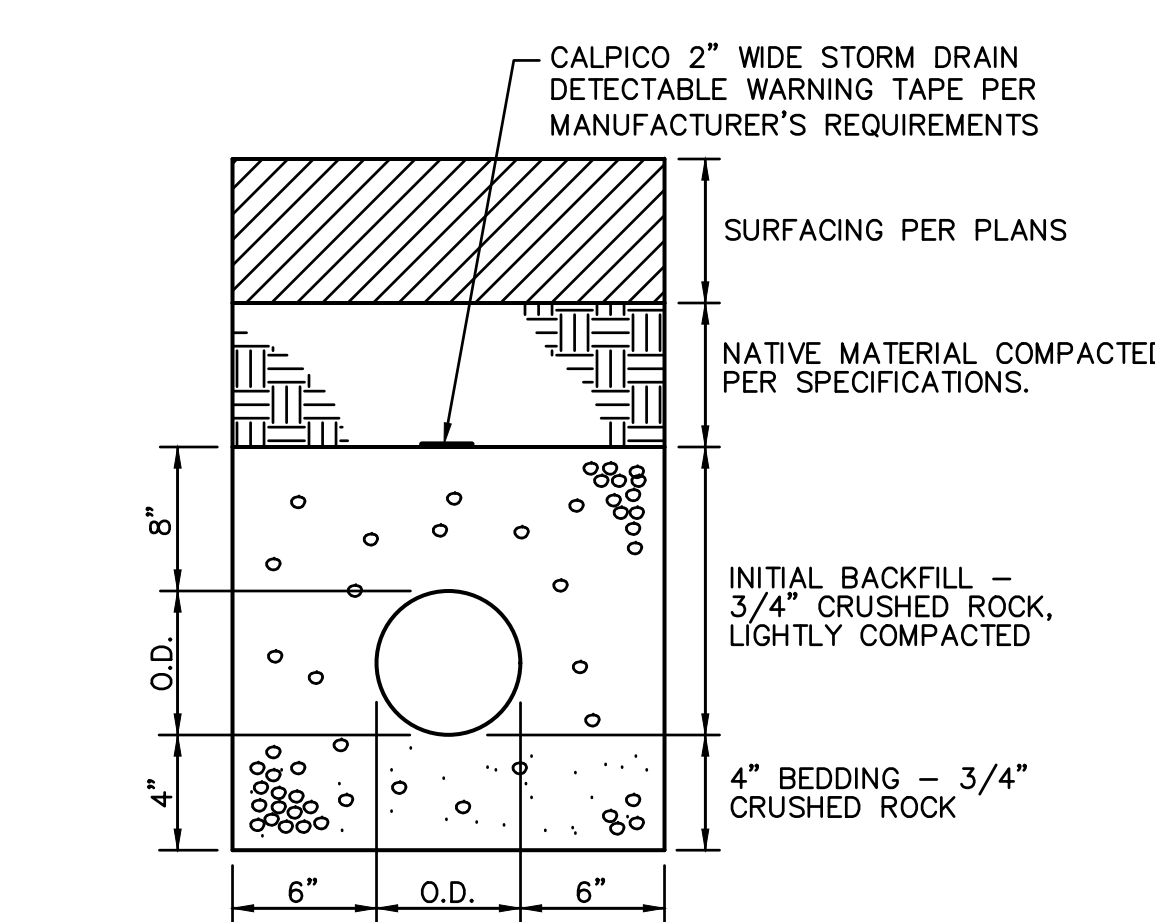
7 WATER VALVE  
C5.1 1/2" TO 3" PIPE NO SCALE



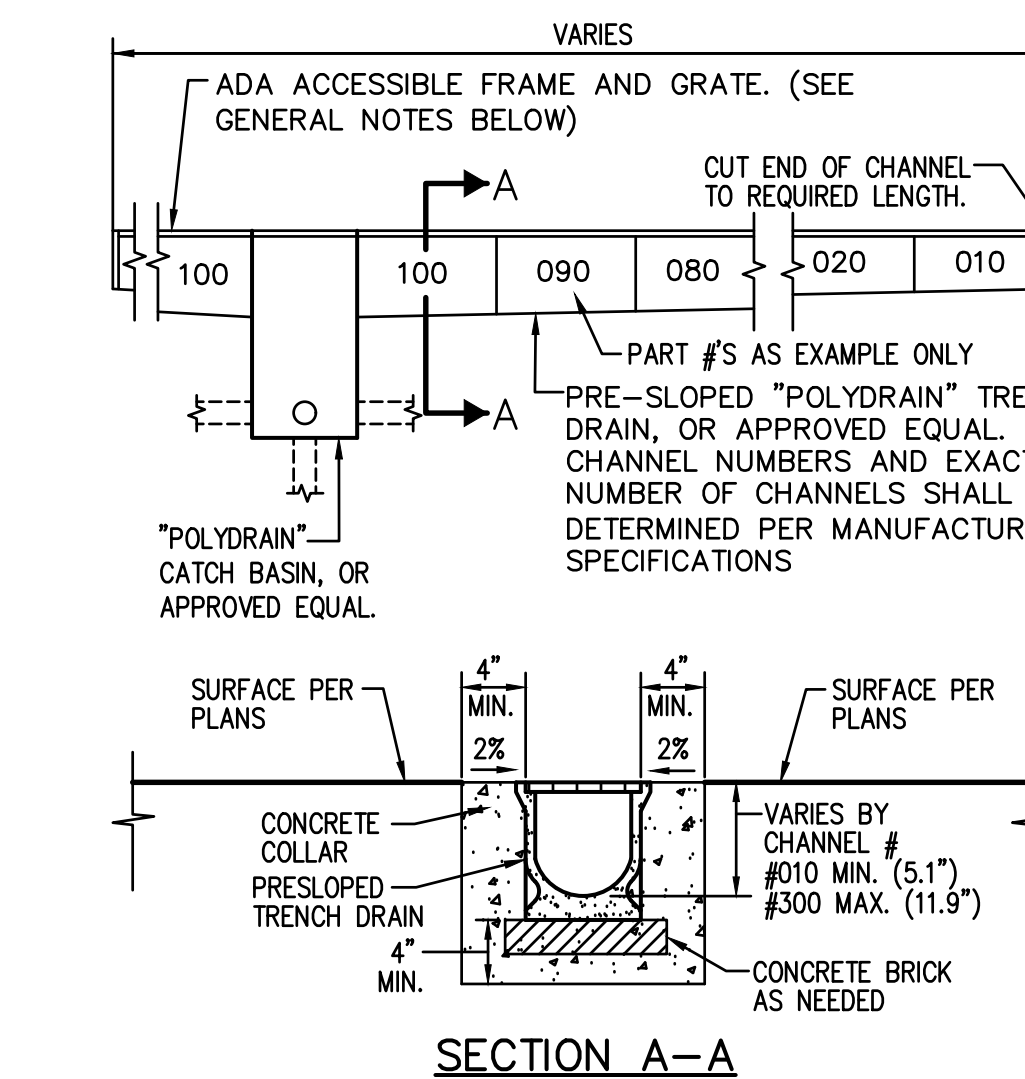
8 DRINKING FOUNTAIN DRYWELL  
C5.1 FOR DRINKING FOUNTAIN ONLY NO SCALE



3 DROP INLET  
C5.1 NO SCALE

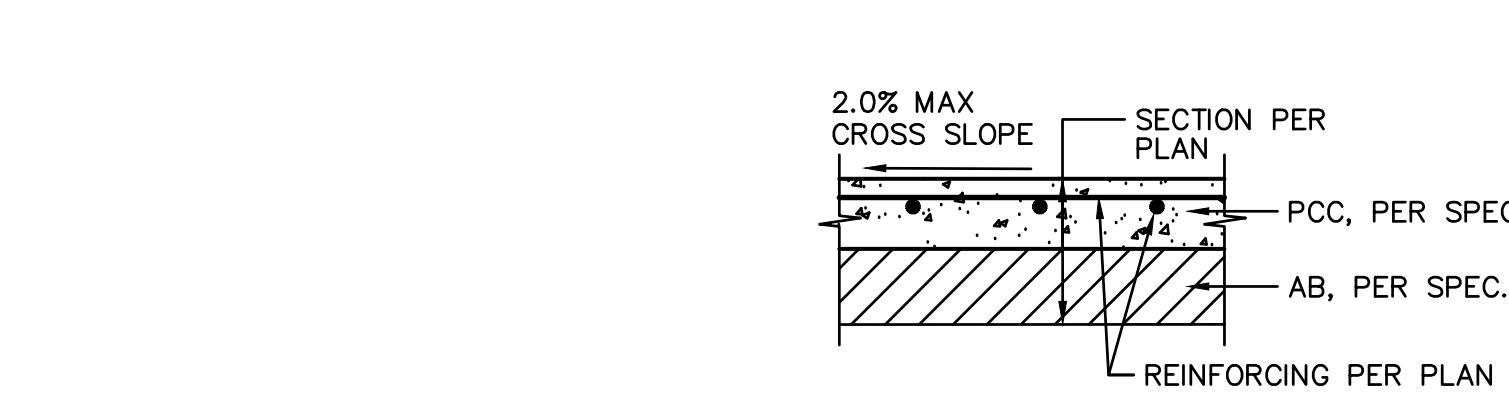


4 STORM DRAIN TRENCH  
C5.1 NO SCALE

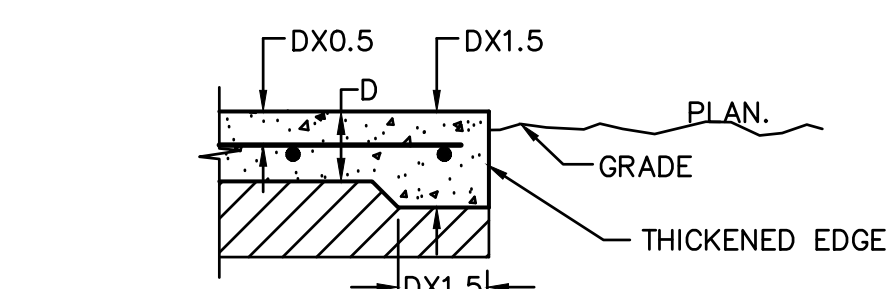


- GENERAL NOTES:
1. GRATE SHALL ADA ACCESSIBLE WITH 1/2" MAX SPACING, POLYDRAIN MODEL 2412, OR APPROVED EQUAL. IF PLACED IN FIRE LANE OR AREA DESIGNATED FOR VEHICLE TRAFFIC PROVIDE POLYDRAIN MODEL 2506.
  2. IF TRENCH DRAIN IS PLACED IN FIRE LANE OR AREA DESIGNATED FOR VEHICLE TRAFFIC PROVIDE GALVANIZED STEEL "OVERLAY RAILS" AS SUPPLIED BY POLYDRAIN, OR APPROVED EQUAL.
  3. CONTRACTOR SHALL FURNISH AND INSTALL A MODEL 2811B LOCKING DEVICE, OR APPROVED EQUAL, FOR ALL TRENCH DRAIN GRATES.
  4. CONTRACTOR SHALL FURNISH AND INSTALL A TRASH BUCKET, MODEL 2900, IN ALL TRENCH DRAIN CATCH BASINS.
  5. CONTRACTOR SHALL PURCHASE AND FURNISH THE MAINTENANCE/OPERATIONS DEPARTMENT OF THE SCHOOL WITH 2 MODEL 2231 TRENCH DRAIN SHOVEL HEADS, WITH STANDARD WOOD, OR COMPOSITE HANDLES.
  6. ALL MITERED JOINTS SHALL BE SEALED WITH POLYDRAIN "POLYSEAL" CAULKING OR APPROVED EQUAL.

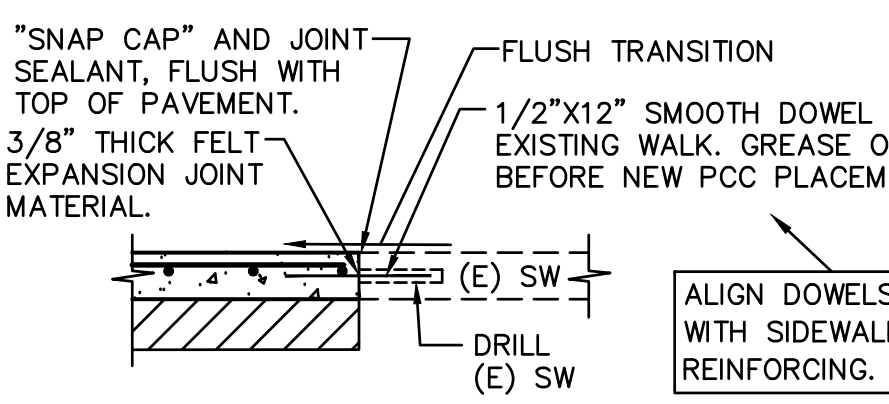
5 TRENCH DRAIN DETAIL  
C5.1 NO SCALE



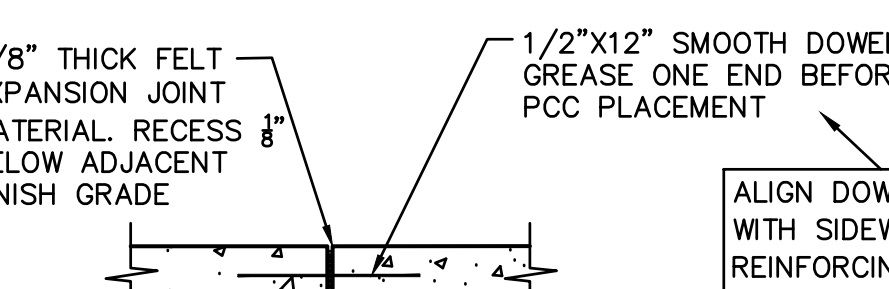
TYPICAL SECTION



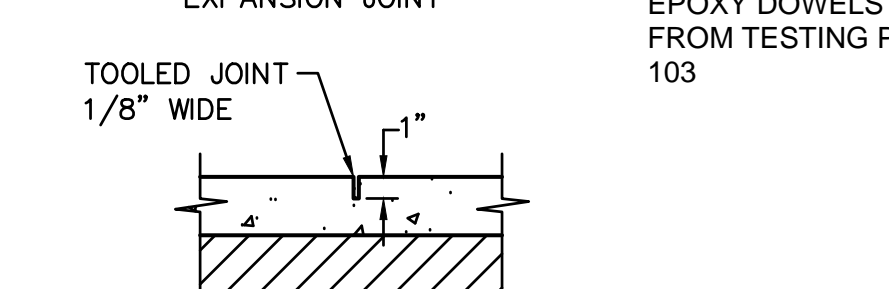
TYPICAL THICKENED EDGE



CONNECTION TO (E) CONCRETE



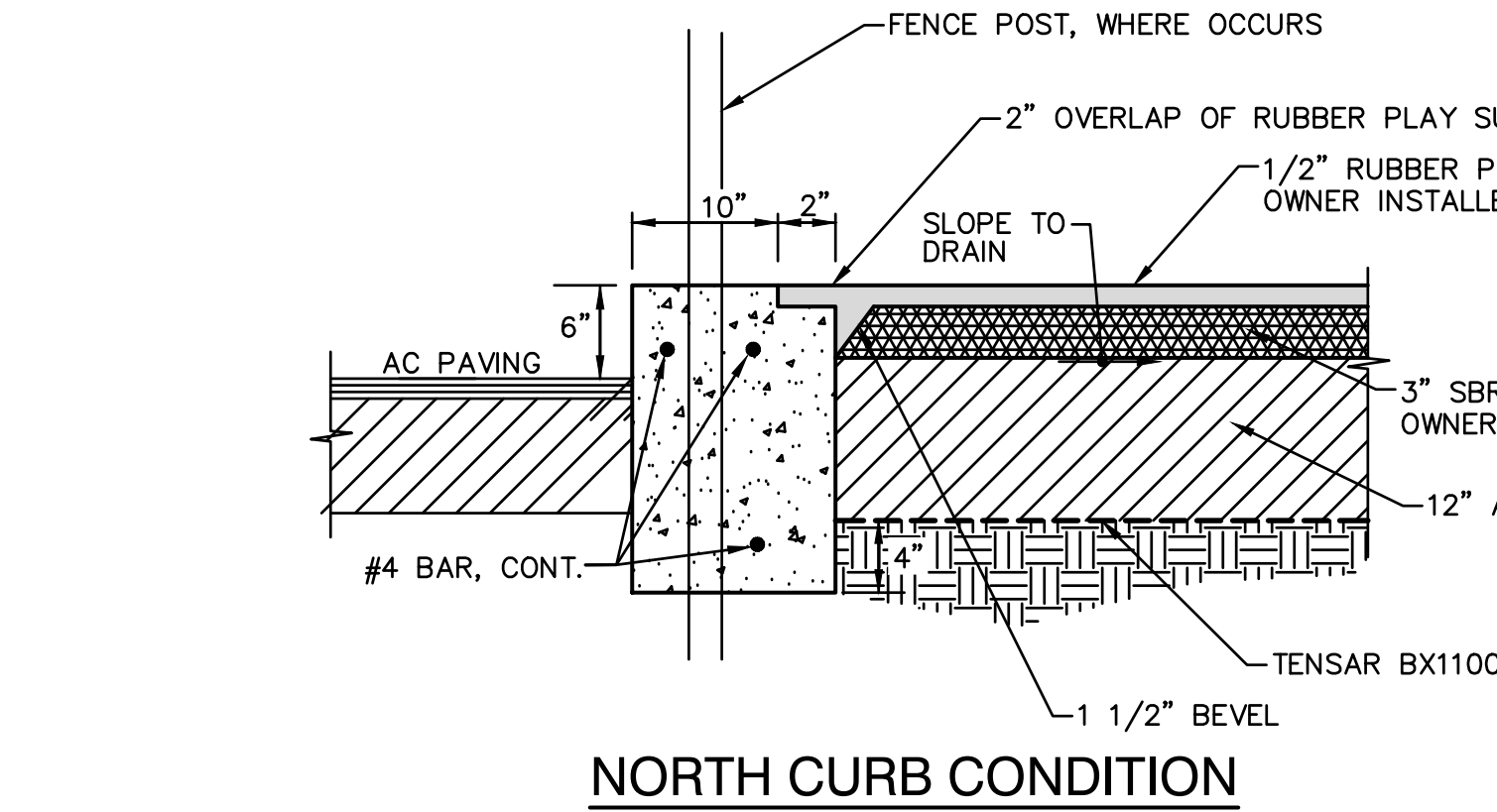
EXPANSION JOINT



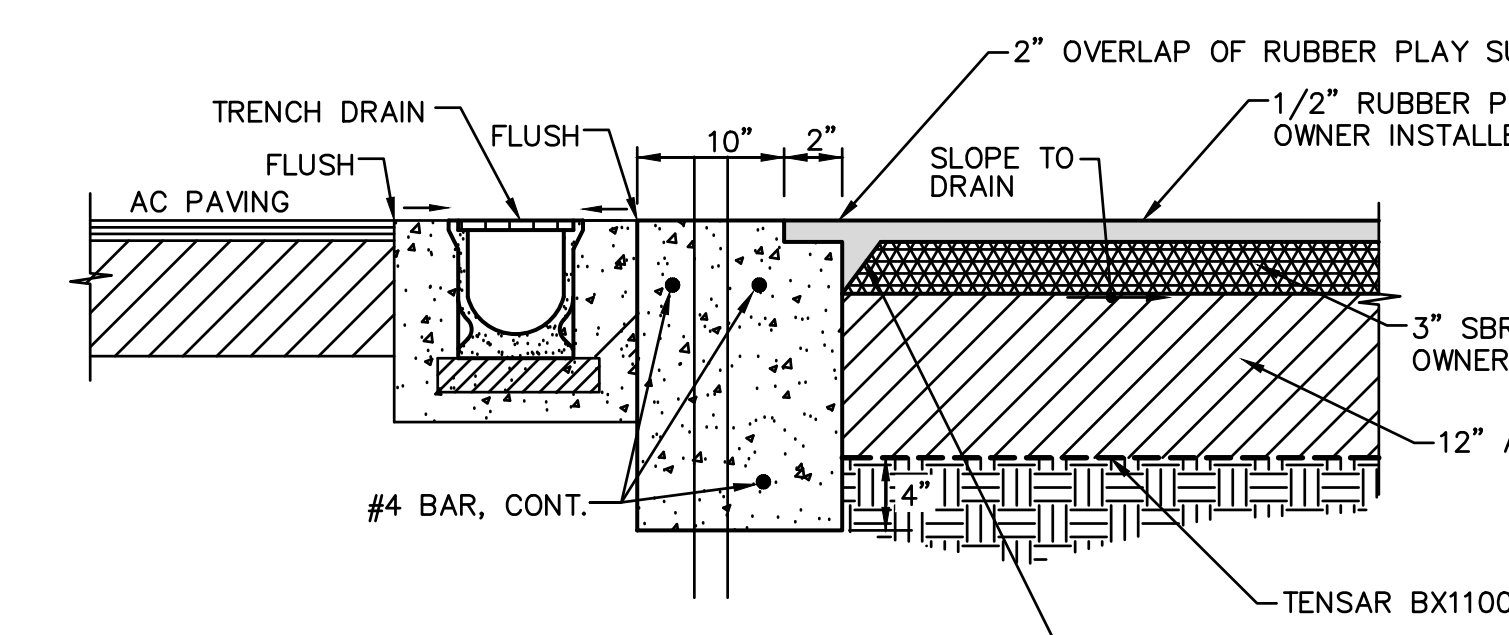
CONTROL JOINT

- NOTES:
1. PROVIDE FELT EXPANSION JOINTS AT 20 FEET O.C. MAX. PROVIDE CONTROL JOINTS AT 8 FEET O.C. MAX.
  2. EXPANSION OR CONTROL JOINTS SHALL NOT EXCEED 1/2" IN SURFACE WIDTH.

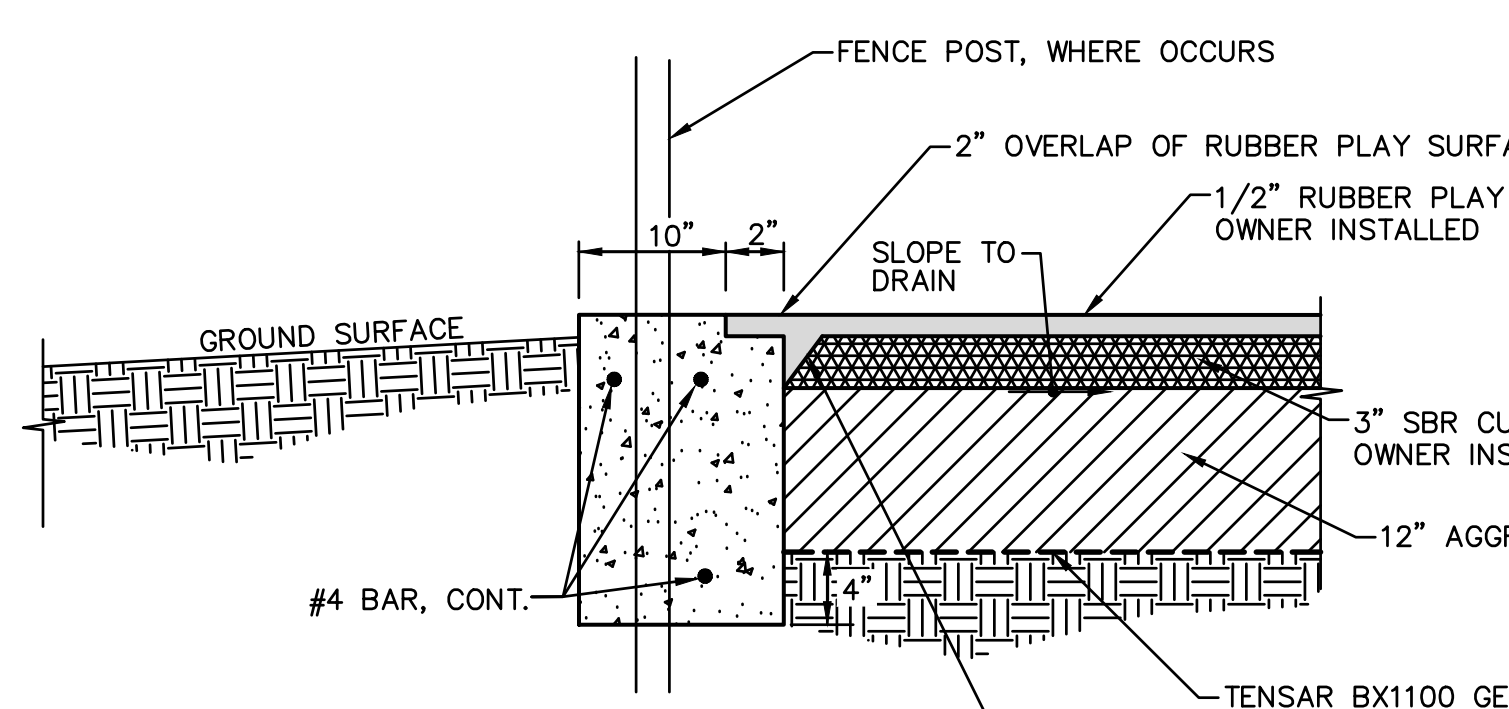
1 CONCRETE SIDEWALK  
C5.1 NO SCALE



NORTH CURB CONDITION

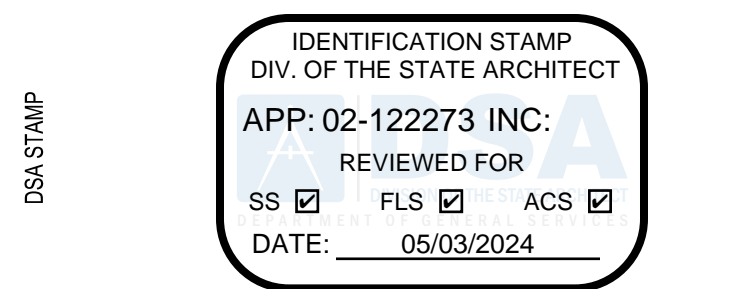


SOUTH CURB CONDITION

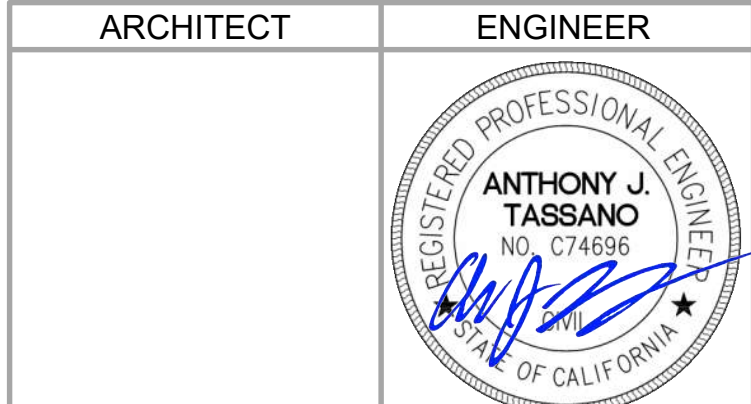


EAST-WEST CURB CONDITION

2 12" APPARATUS CURB  
C5.1 NO SCALE



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NO.	REMARKS	DATE

NO.	REMARKS	DATE

NO.	REMARKS	DATE

WASHINGTON UNIFIED  
SCHOOL DISTRICT  
930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

CONSTRUCTION DOCUMENTS

WUSD RIVERBANK ES  
ESSR III  
1100 CARRIE STREET  
WEST SACRAMENTO, CA 95605

DETAILS AND SECTIONS

Date	Project Number
11/20/2023	22042
Application Number	Drawing Number
Drawn	Checked
AT	AT

C5.1





KEYNOTES

NUMBER	NOTE
02 114	(E) TREES, TO REMAIN TYP.
13 302	NEW FABRIC SHADE STRUCTURE (SEE PC DRAWINGS)
22 451	HI-LO INTERIOR DRINKING FOUNTAIN WITH BOTTLE FILLER (SEE DETAIL 2/A10.10.1)
22 452	HI-LO EXTERIOR DRINKING FOUNTAIN WITH BOTTLE FILLER (SEE DETAIL 1/A10.10.1)
22 454	HI-LO FREE STANDING DRINKING FOUNTAIN (SEE DETAIL 4/A10.10.1)
22 457	BOTTLE FILLER (SEE DETAIL 5/A10.10.1)
32 221	CHAIN LINK FENCE, SEE 5/A10.2.1

PLAY AREA NOTES

- PLAY STRUCTURES TO COMPLY WITH CBC 11B-1008.
- GROUND SURFACES ON ACCESSIBLE ROUTES, CLEAR FLOOR OR GROUND SPACES, AND TURNING SPACES SHALL COMPLY WITH CBC SECTION 11B-1008.2.6.
- GROUND SURFACES SHALL COMPLY WITH ASTM F1951. GROUND SURFACES SHALL BE INSPECTED AND MAINTAINED REGULARLY AND FREQUENTLY TO ENSURE CONTINUED COMPLIANCE WITH ASTM F1951.
- GROUND SURFACES LOCATED WITHIN USE ZONES SHALL COMPLY WITH ASTM F1292.

GENERAL NOTES

- CONTRACTOR IS RESPONSIBLE FOR 6'-0" HIGH TEMPORARY CONSTRUCTION BARRIER WITH VISION SCREEN AT STAGING, STORAGE AND CONSTRUCTION AREA WITH SIGNAGE EVERY 20'-0" TO WARN STUDENTS OF CONSTRUCTION AREA.
- CONTRACTOR SHALL ACCESS THE SITE FROM RIVERBANK ROAD. ANY DAMAGE TO FIRE LANE WILL BE AT THE CONTRACTOR'S EXPENSE.
- CONTRACTOR TO REPAIR BACK TO EXISTING CONDITIONS ALL LAYDOWN AREAS AT THE END OF CONSTRUCTION. THIS INCLUDES LANDSCAPE AREAS AND ANY BROKEN SPRINKLERS, VALVE BOXES, CONCRETE, ASPHALT, ETC.
- CONTRACTOR SHALL REPLACE, RECONSTRUCT AND REPAIR ALL EXISTING WORK THAT IS IMPACTED, DAMAGED, OR DESTROYED AS A RESULT OF ANY CONTRACTOR WORK INCLUDING, BUT NOT LIMITED TO, HARDSCAPING, SIDEWALKS, IRRIGATION SYSTEMS, LANDSCAPING, LAWNS, STRUCTURES AND UTILITIES - ALL TO THE SATISFACTION OF THE DISTRICT.
- WHERE ASPHALT OR CONCRETE IS BEING REPAATCHED, CONTRACTOR SHALL PROVIDE EVEN AND STRAIGHT LINE CUTS WITH 2-FOOT STRAIGHT SLURRY SEAL SURFACE PATCH ON BOTH SIDES OF CUT.
- CONTRACTOR SHALL EXERCISE EXTREME CAUTION IN EXCAVATING AND TRENCHING ON SITE TO AVOID EXISTING DUCTS, PIPING OR CONDUITS, ETC., AND TO PREVENT HAZARDS TO PERSONNEL AND/OR DAMAGE TO EXISTING UNDERGROUND UTILITIES OR STRUCTURES WHETHER OR NOT SHOWN AND INSTALLED BY ANY OTHER CONTRACTS. THE ARCHITECT IS NOT RESPONSIBLE FOR THE LOCATION OF UNDERGROUND UTILITIES OR STRUCTURES WHETHER OR NOT SHOWN OR DETAILED AND INSTALLED BY ANY OTHER CONTRACTS. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT SHOULD SUCH UNIDENTIFIED CONDITIONS BE DISCOVERED. THESE DRAWINGS AND SPECIFICATIONS DO NOT INCLUDE THE NECESSARY ELEMENTS FOR CONSTRUCTION SAFETY.
- GATES IN PATH OF TRAVEL SHALL COMPLY WITH EXIT DOOR REQUIREMENTS WITH PROPER LEVER HARDWARE AND KICK PLATES.
- CONTRACTOR TO TAKE PHOTOS PRIOR TO REMOVAL.
- SALVAGE ALL DRINKING FOUNTAINS AND RETURN TO DISTRICT.
- CONTRACTOR SHALL RE-ROUTE AND REPAIR ANY IRRIGATION LINES AND HEADS IN THE WAY OF NEW WORK TO ENSURE A FULLY FUNCTIONING SYSTEM AT THE END OF CONSTRUCTION.

LEGEND

	PROPERTY LINE
	LIMIT OF WORK
	NOT IN SCOPE
	FIRE HYDRANT
	(N) CONCRETE PAVING, SEE CIVIL DRAWINGS
	(N) ASPHALT PAVING, SEE CIVIL DRAWINGS
	(N) POUR IN PLACE RUBBER SURFACING, SEE CIVIL DRAWINGS

WASHINGTON UNIFIED SCHOOL DISTRICT  
930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

PROJECT STATUS

WUSD RIVERBANK ES  
ESSR III  
1100 CARRIE STREET  
WEST SACRAMENTO, CA 95605

SITE PLAN OVERALL

Date  
03/13/2024

Application Number  
02-122273

Drawn  
Author

Checked  
Checker

Project Number  
22042

Drawing Number  
A1.1

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122273 INC:  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☒  
DATE: 05/03/2024

STUDIO W  
ARCHITECTS

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ARCHITECT  
BRYAN P. WHITFIELD  
No. C 28345  
Ren. 9/26/2025  
DATE SIGNED: 05/03/2024

ENGINEER

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REVISION HISTORY

NO.	REMARKS	DATE

DRAWING STATUS

☐ DSA PLAN CHECK  
☐ DSA BACK CHECK  
☐ BIDDING  
☐ CONSTRUCTION

KEY PLAN

WASHINGTON UNIFIED SCHOOL DISTRICT  
930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

PROJECT STATUS

WUSD RIVERBANK ES  
ESSR III  
1100 CARRIE STREET  
WEST SACRAMENTO, CA 95605

SITE PLAN OVERALL

Date  
03/13/2024

Application Number  
02-122273

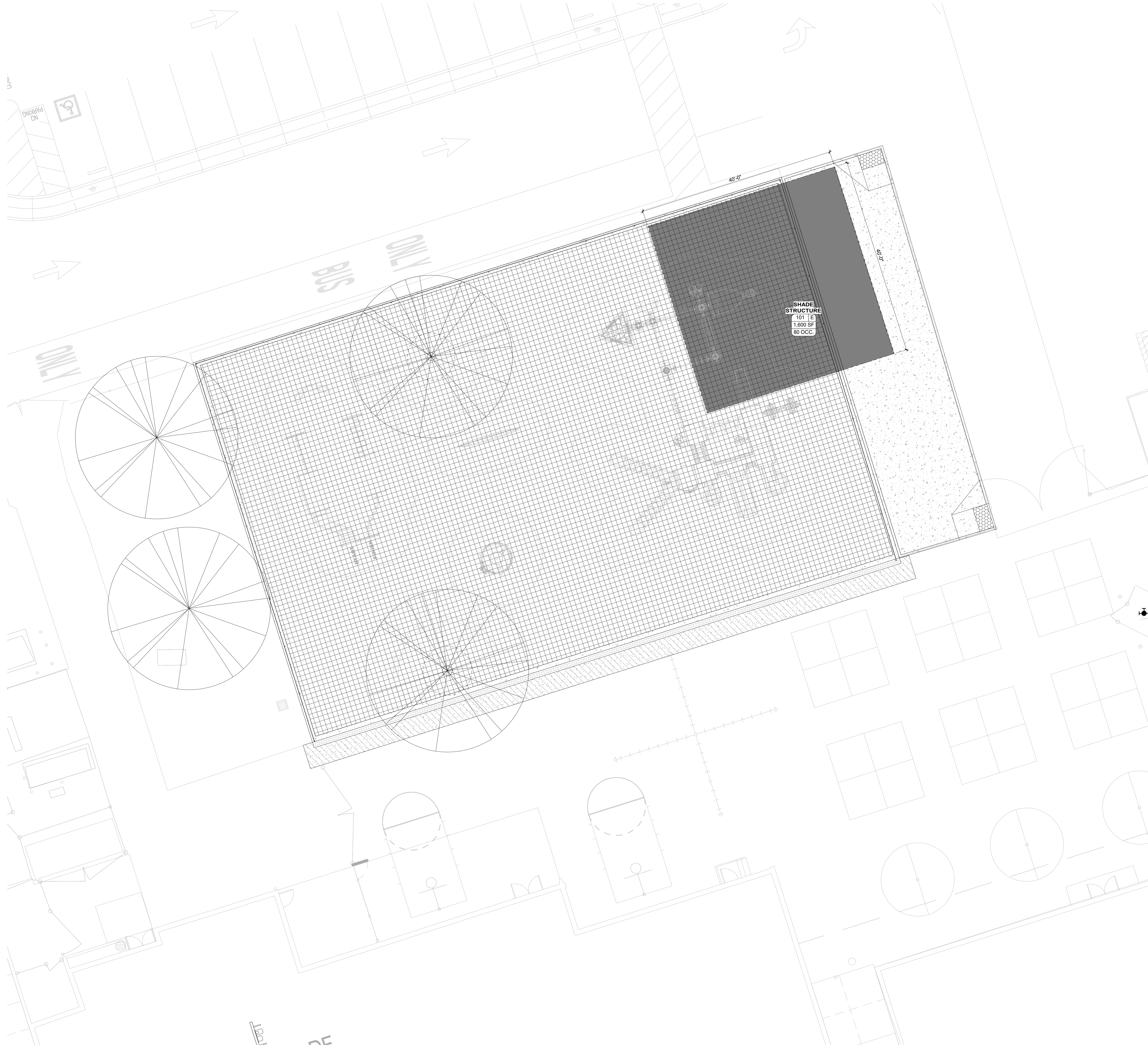
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Author

Checked  
Checker

Project Number  
22042

Drawing Number  
A1.1





ENLARGED SITE PLAN

1/8" = 1'-0"

10

KEYNOTES

NUMBER NOTE

GENERAL NOTES

LEGEND

- PROPERTY LINE
- LIMIT OF WORK
- NOT IN SCOPE
- FIRE HYDRANT
- (N) ASPHALT PAVING, SEE CIVIL DRAWINGS
- (N) CONCRETE PAVING, SEE CIVIL DRAWINGS
- (N) POUR IN PLACE RUBBER SURFACING, SEE CIVIL DRAWINGS

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122273 INC:  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☒  
DATE: 05/03/2024



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ARCHITECT	ENGINEER

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NO.	REMARKS	DATE

DATE

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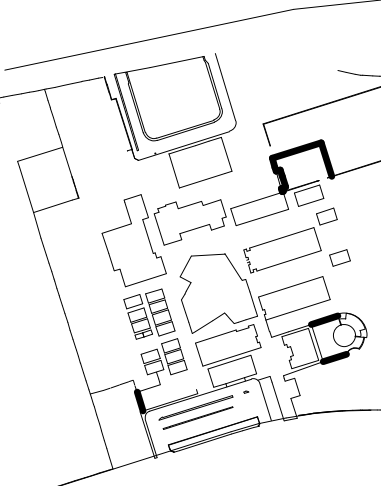
☐ DSA PLAN CHECK

☐ DSA BACK CHECK

☐ BIDDING

☐ CONSTRUCTION

KEY PLAN



WASHINGTON UNIFIED  
SCHOOL DISTRICT  
930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

PROJECT STATUS

WUSD RIVERBANK ES  
ESSR III  
1100 CARRIE STREET  
WEST SACRAMENTO, CA 95605

ENLARGED SITE PLAN

Date

03/13/2024

Application Number

02-122273

Drawn

Author

Project Number

22042

Drawing Number

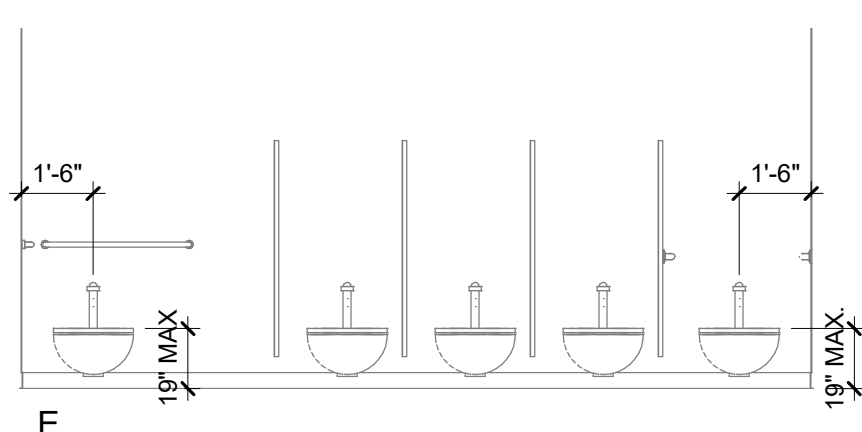
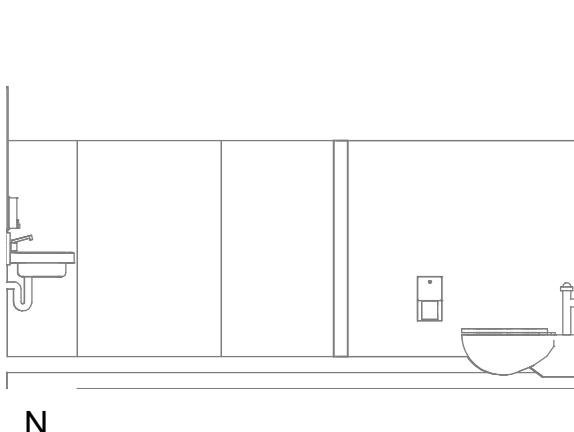
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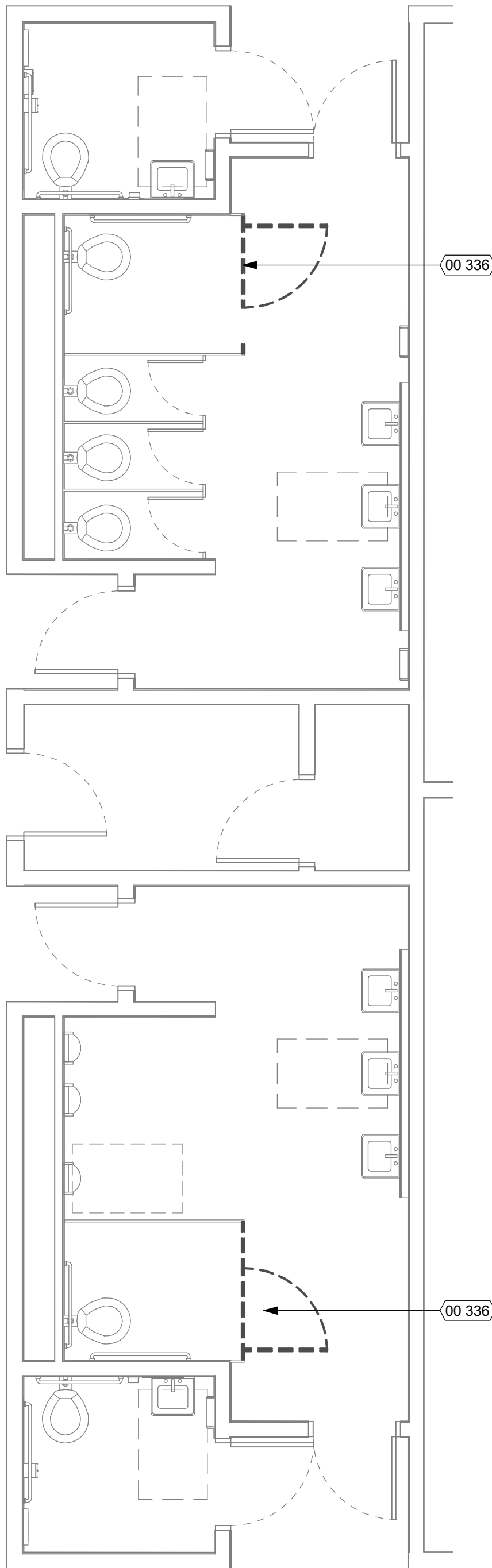
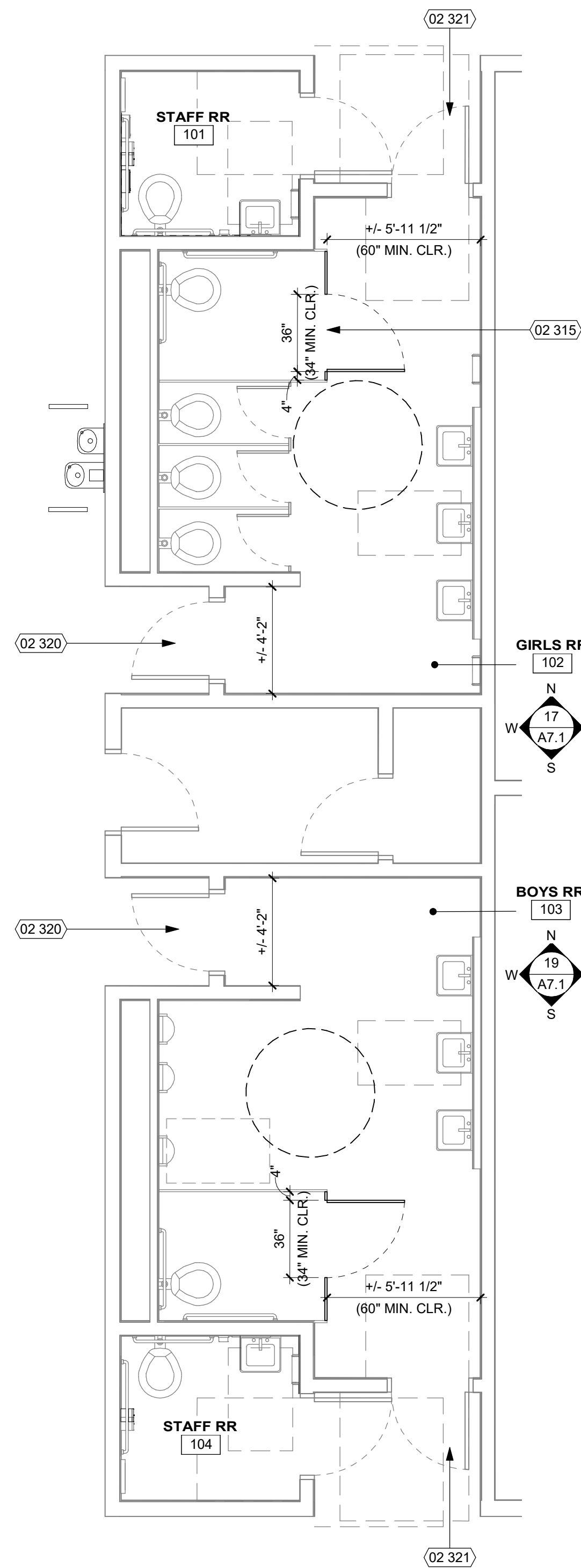
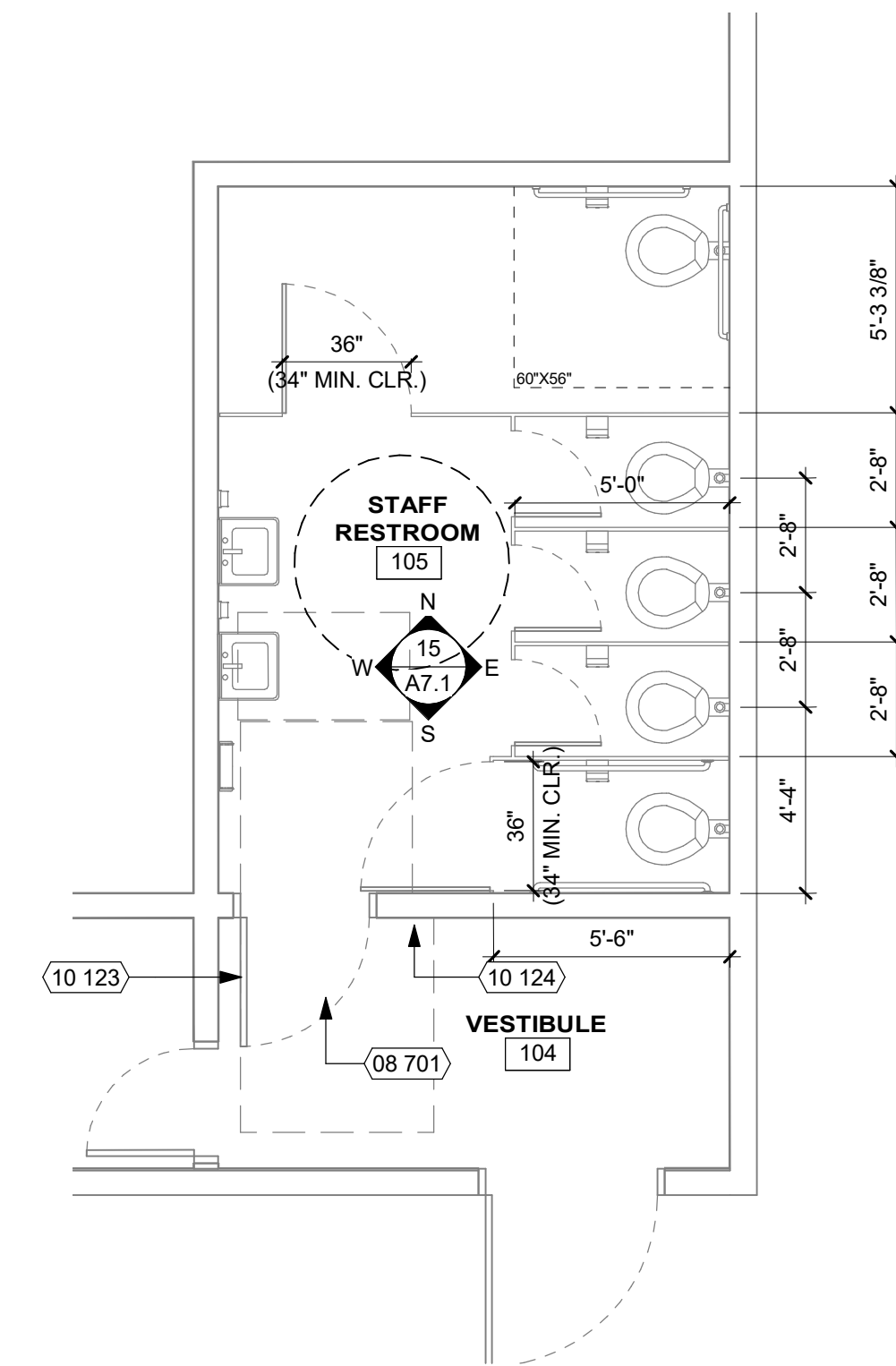
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A1.2





1/4" = 1'-0"

$$1/4" = 1'-0"$$

$$1/4" = 1'-0"$$

$$1/4" = 1'-0"$$

$$1/4" = 1'-0"$$

KEYNOTES	
NUMBER	NOTE
00 338	REMOVE (E) TOILET PARTITION AND DOOR
02 315	REMOVE (E) TOILET PARTITION AND PARTITION DOOR. INSTALL (N) PARTITION AND PARTITION DOOR AS SHOWN. MATCH PARTITION WITH EXISTING.
02 320	(E) NOT ACCESSIBLE DOOR.
02 321	(E) ACCESSIBLE DOOR.
08 071	REPLACE LOCK WITH NEW PRIVACY INDICATOR. SEE HARDWARE SCHEDULE THIS SHEET
10 123	(N) UNISER RESTROOM DOOR SIGNAGE (SEE DETAIL 21A/10.10.1)
10 124	(N) UNISER RESTROOM WALL-MOUNTED SIGNAGE (SEE DETAIL 22A/10.10.1)

## GENERAL NOTES



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1. DIMENSIONS
  - A. ALL MINIMUM CLEARANCE DIMENSIONS FOR PLUMBING FIXTURES ARE FROM FACE OF WALL FINISH TO CENTER OF FIXTURE, U.O.N.
  - B. ALL DIMENSIONS MARKED "MIN. CLR." OR "CLR." ARE TO/ FROM FACE OF WALL FINISH (F.O.F.)
2. ALL RESTROOM FIXTURES AND ACCESSORIES MOUNTING HEIGHTS AND LOCATIONS TO USE ADULT DIMENSIONS

1. REPLACE DOOR LOCK WITH SCHLAGE, PRIVACY W/ INDICATOR (MODEL L9056P6 06A L583-363 L283-722, FINISH 626.

## LEGEND

---

	30" x 48" ACCESSIBLE CLEAR SPACE
	60" DIAMETER ACCESSIBLE CLEAR SPACE

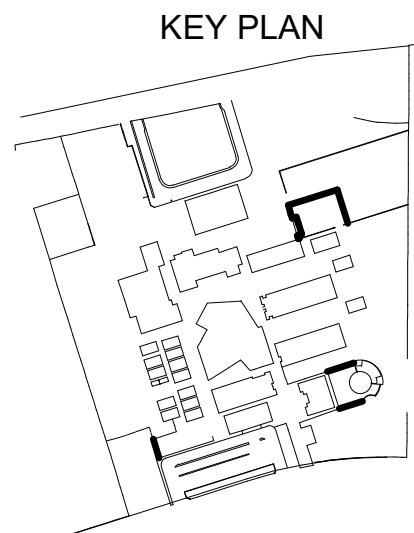
ARCHITECT	ENGINEER
	
DATE SIGNED: 03/13/2024	

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DRAWING STATUS		DATE
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<input type="radio"/>	DSA BACK CHECK	
<input type="radio"/>	BIDDING	
<input type="radio"/>	CONSTRUCTION	



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WEST SACRAMENTO, CA 95691

## PROJECT STATUS

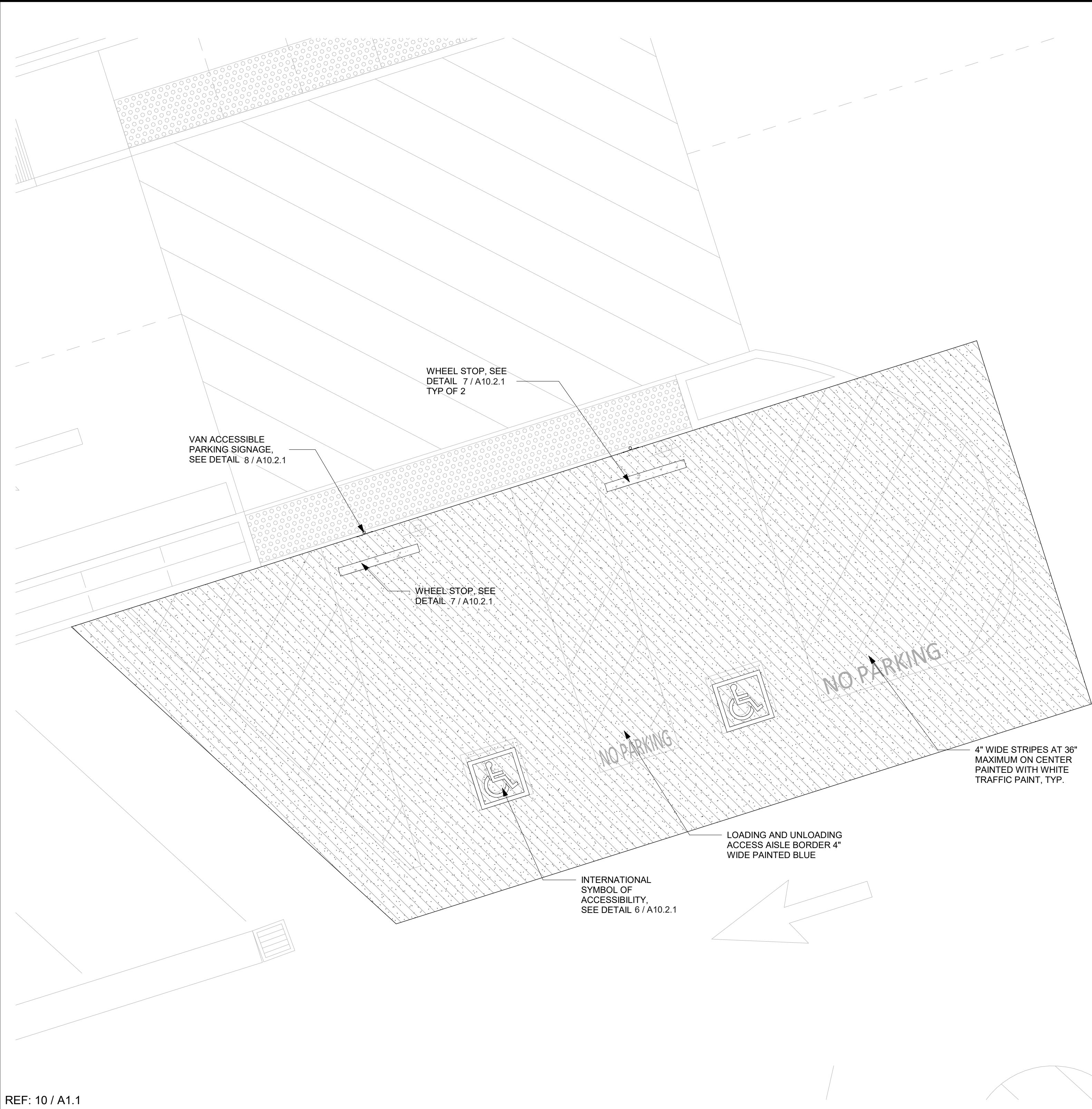
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ESSR III  
1100 CARRIE STREET  
WEST SACRAMENTO, CA 95605

## RESTROOM PLANS AND ELEVATIONS

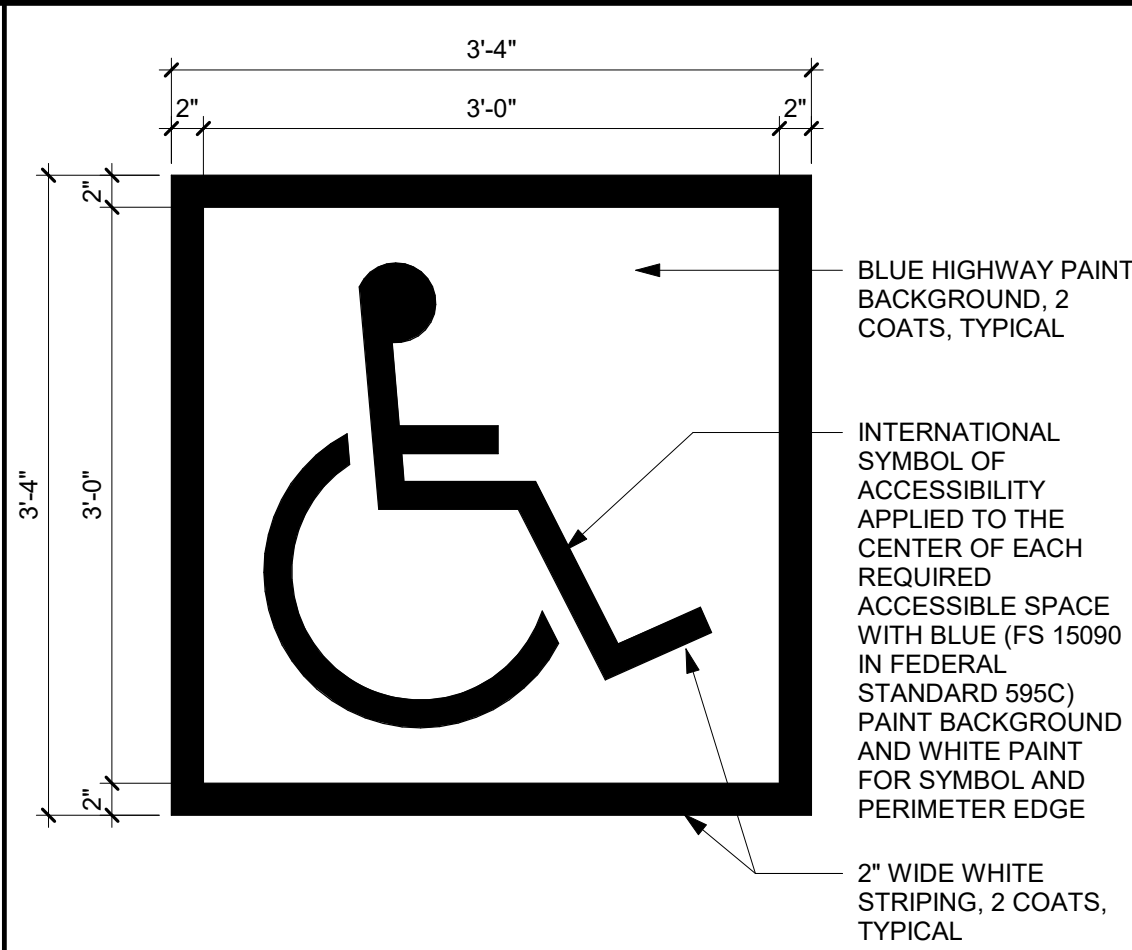
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Application Number		Drawing Number	
02-122273		A7.1	
Drawn	Checked		
Author	Checker		



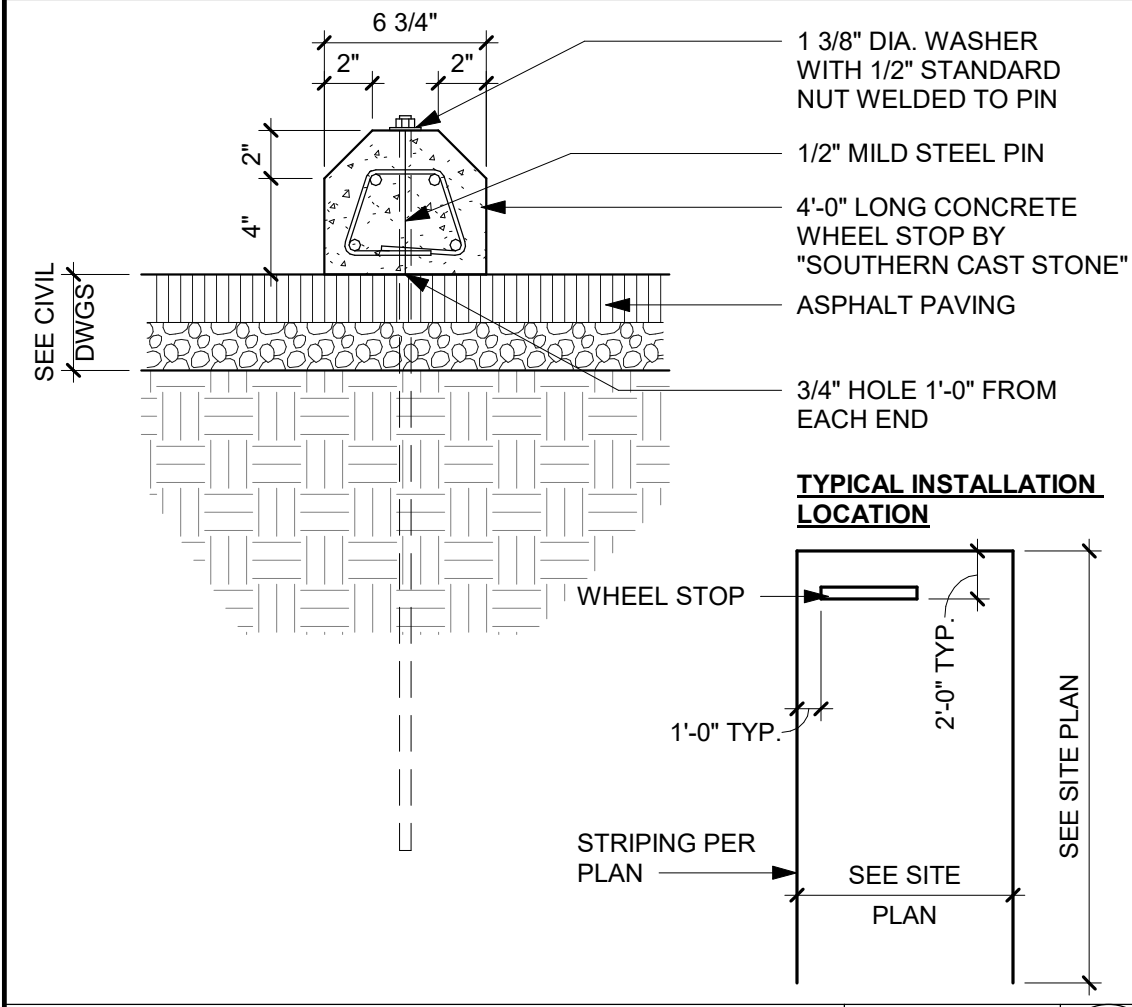
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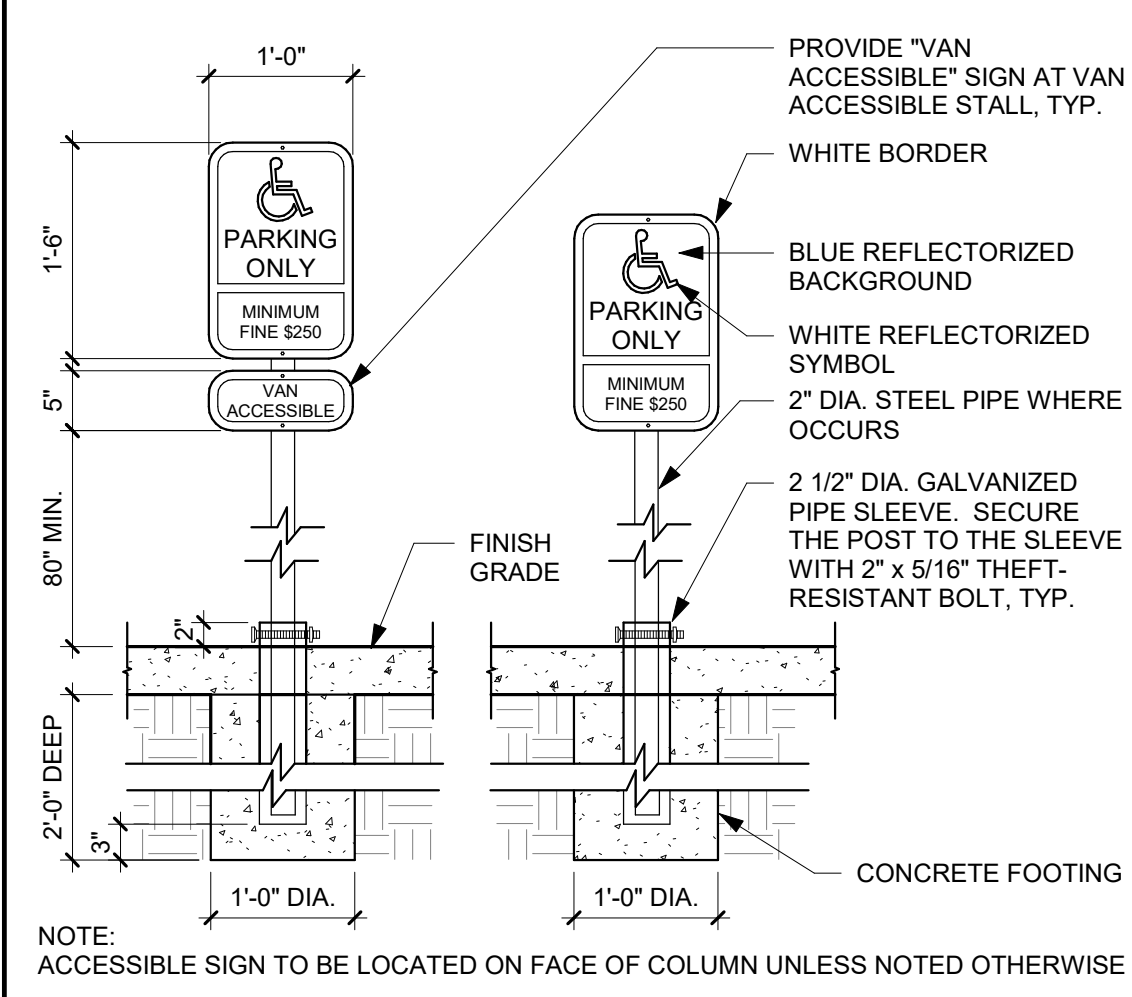
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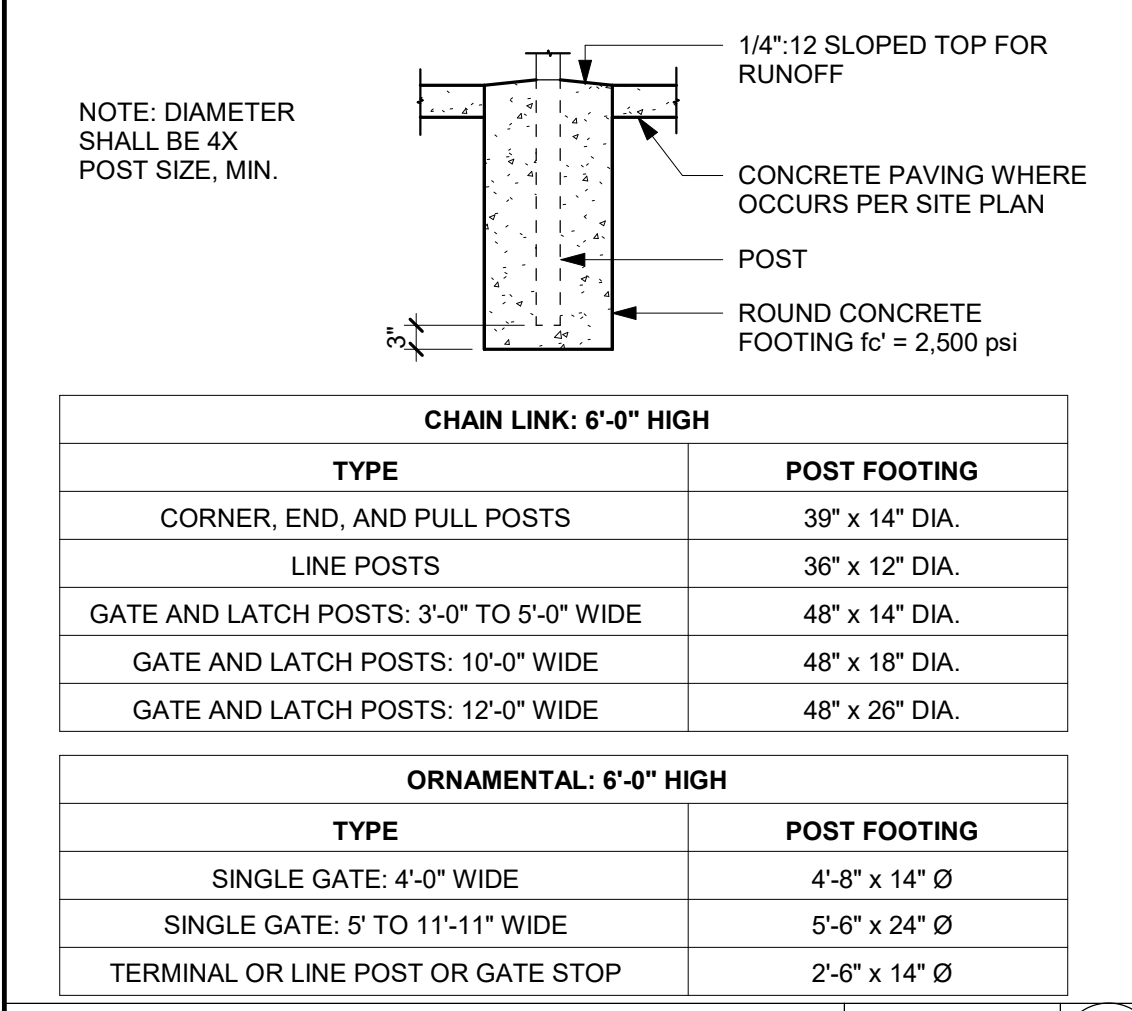
ACCESSIBLE STALL SYMBOL 1" = 1'-0" 6



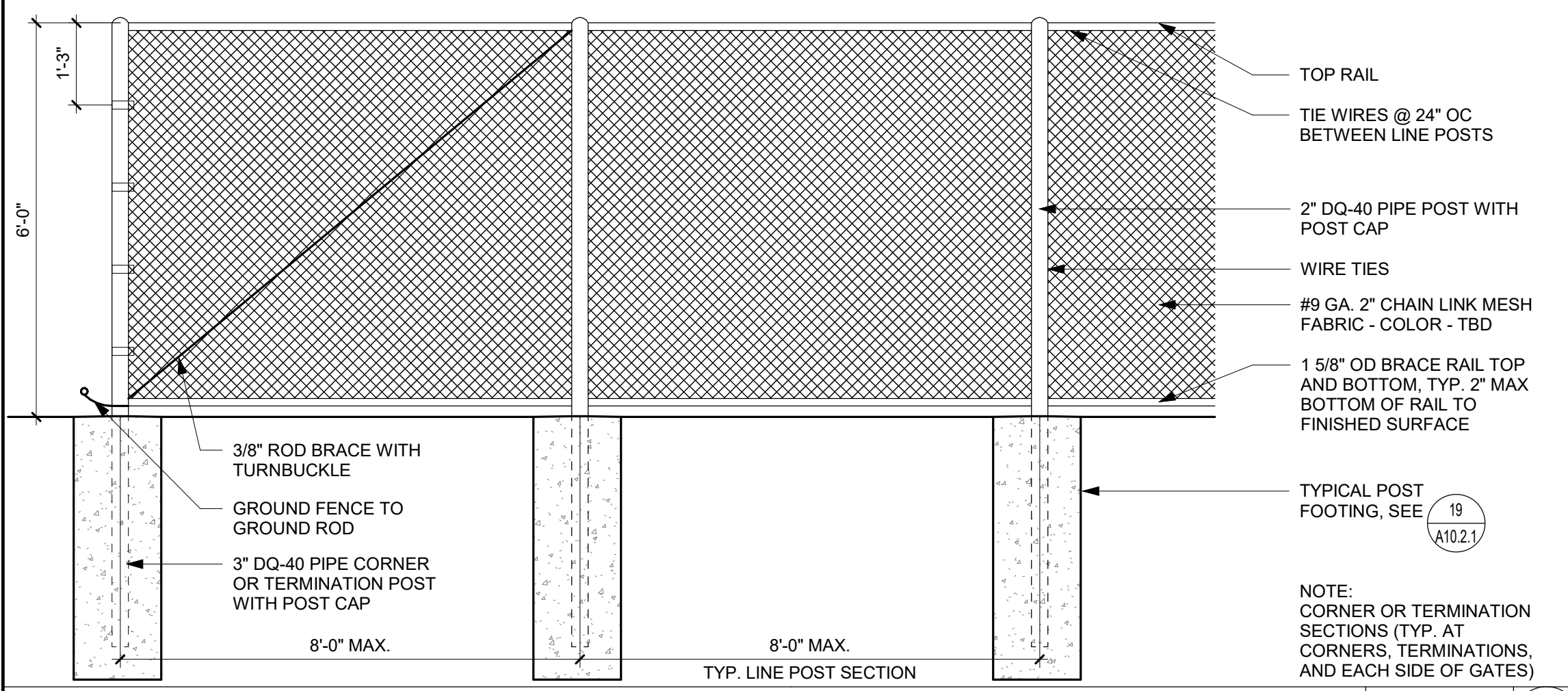
WHEEL STOP 1 1/2" = 1'-0" 7



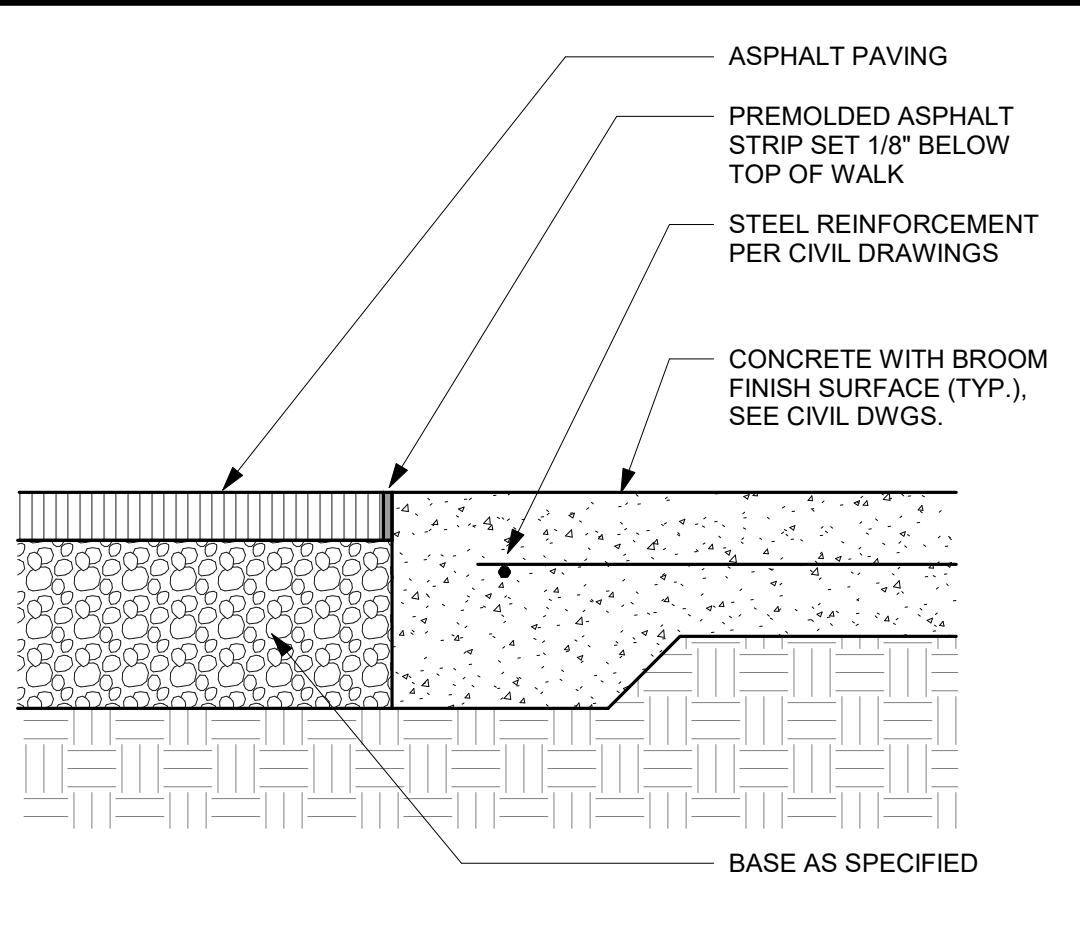
ACCESSIBLE STALL SIGNAGE 3/4" = 1'-0" 8



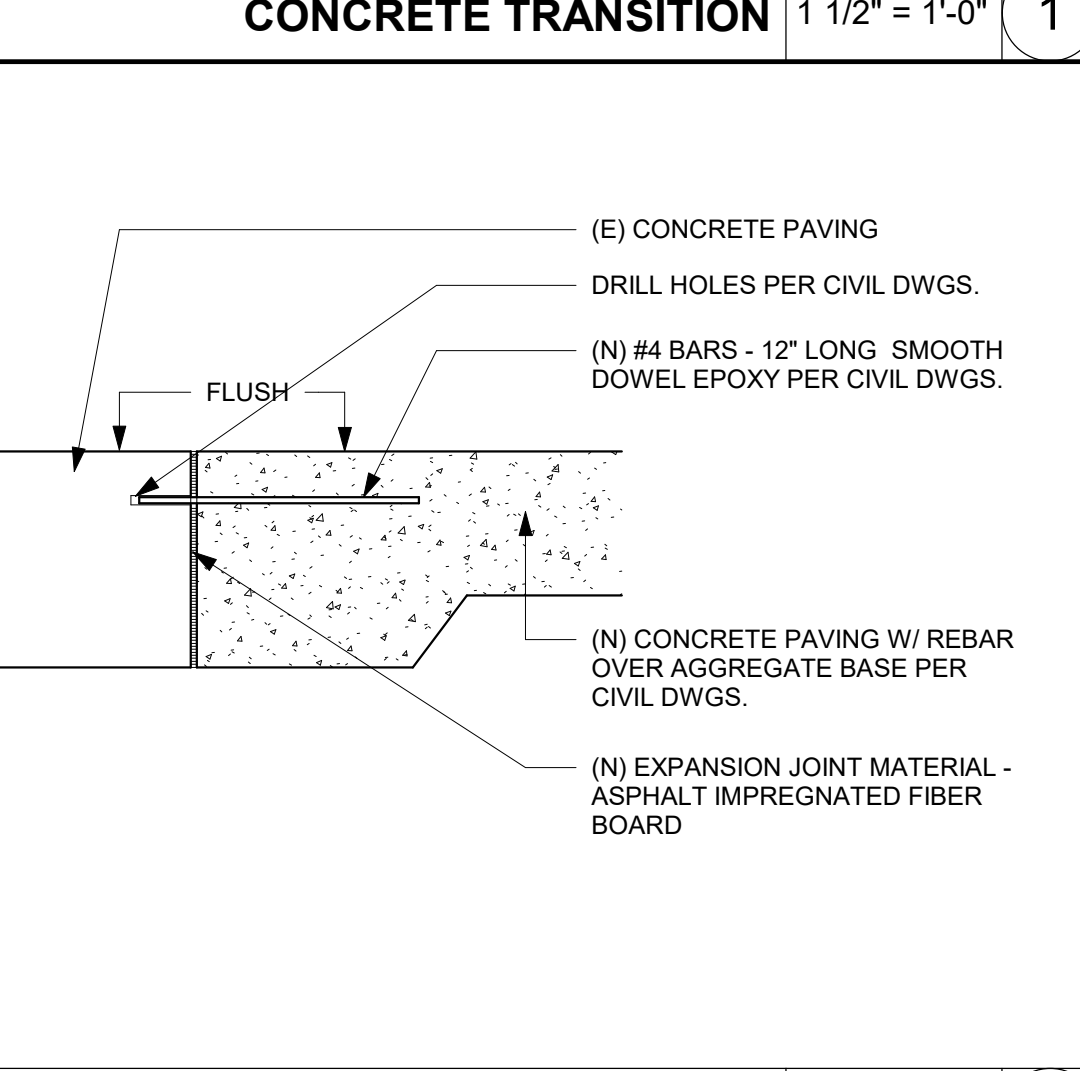
FENCE FOOTING SCHEDULE 1/2" = 1'-0" 19



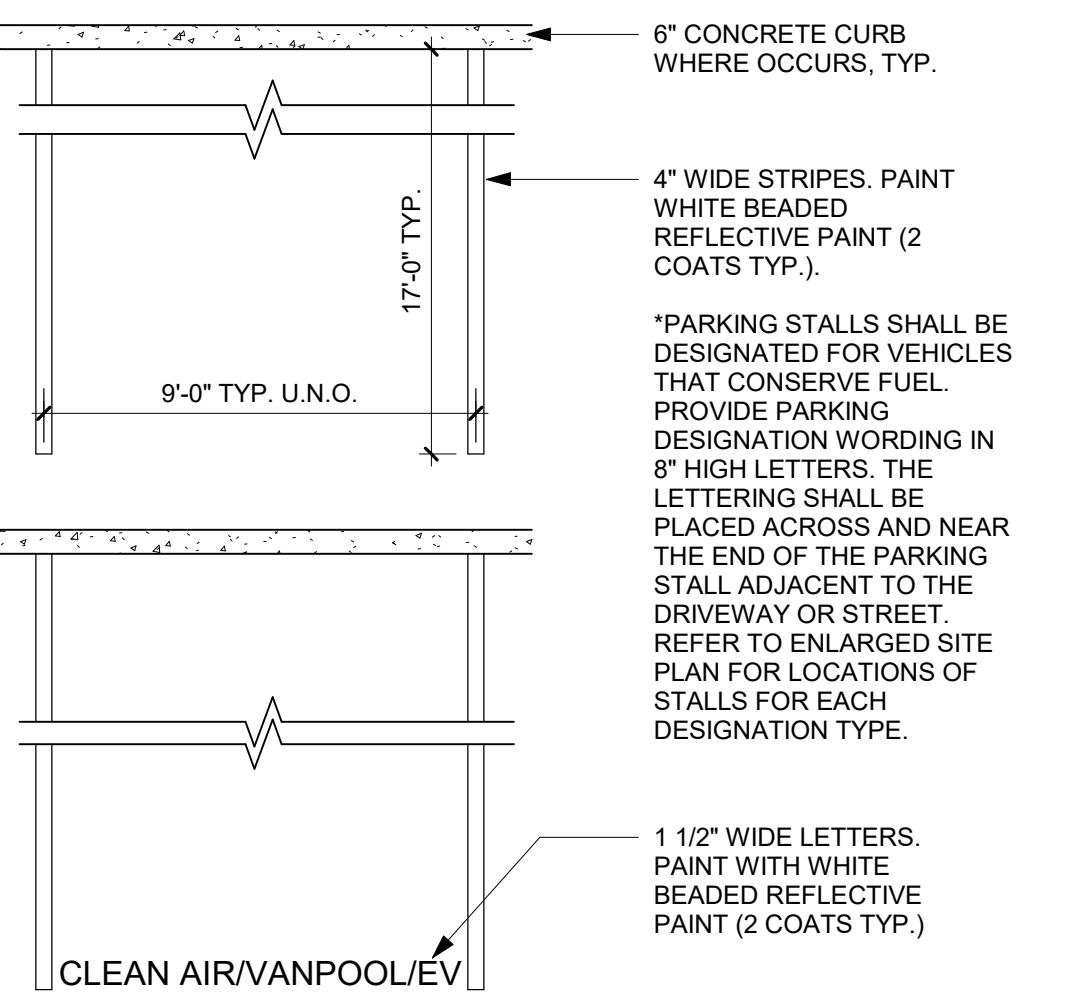
CHAIN LINK FENCE TYPICAL ELEVATION 1/2" = 1'-0" 5



CONCRETE TRANSITION 1 1/2" = 1'-0" 1



(N) PAVING TO (E) PAVING 1 1/2" = 1'-0" 2



TYPICAL STALL MARKING 1/4" = 1'-0" 3

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122273 INC.  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☒  
DATE: 05/03/2024

STUDIO W ARCHITECTS

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Sacramento, California 95811  
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ARCHITECT  
BRYAN P. WHITFIELD  
No. C 28345  
Ren. 9/26/25  
STATE OF CALIFORNIA  
DATE SIGNED: 05/03/2024

ENGINEER

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NO.	REMARKS	DATE

REVISION HISTORY

DRAWING STATUS

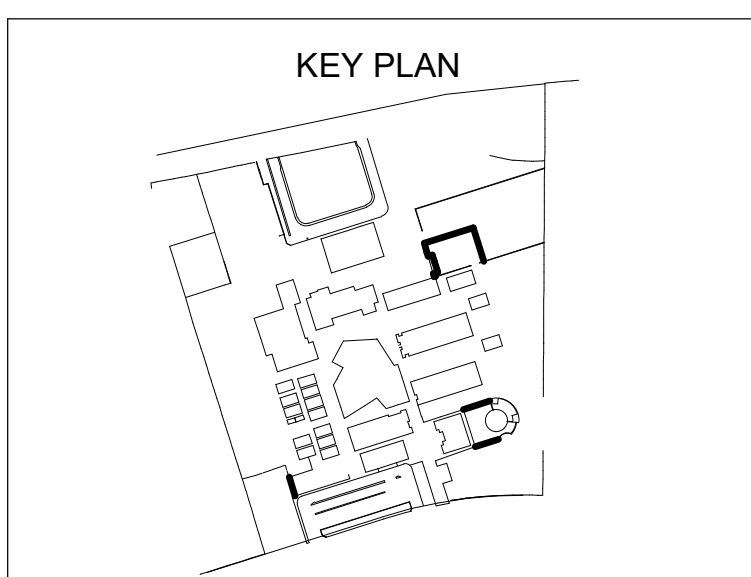
☐ DSA PLAN CHECK

☐ DSA BACK CHECK

☐ BIDDING

☐ CONSTRUCTION

DATE



WASHINGTON UNIFIED SCHOOL DISTRICT  
930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

PROJECT STATUS

WUSD RIVERBANK ES  
ESSR III  
1100 CARRIE STREET  
WEST SACRAMENTO, CA 95605

SITE DETAILS

Date  
03/13/2024

Application Number  
02-122273

Drawn  
Author

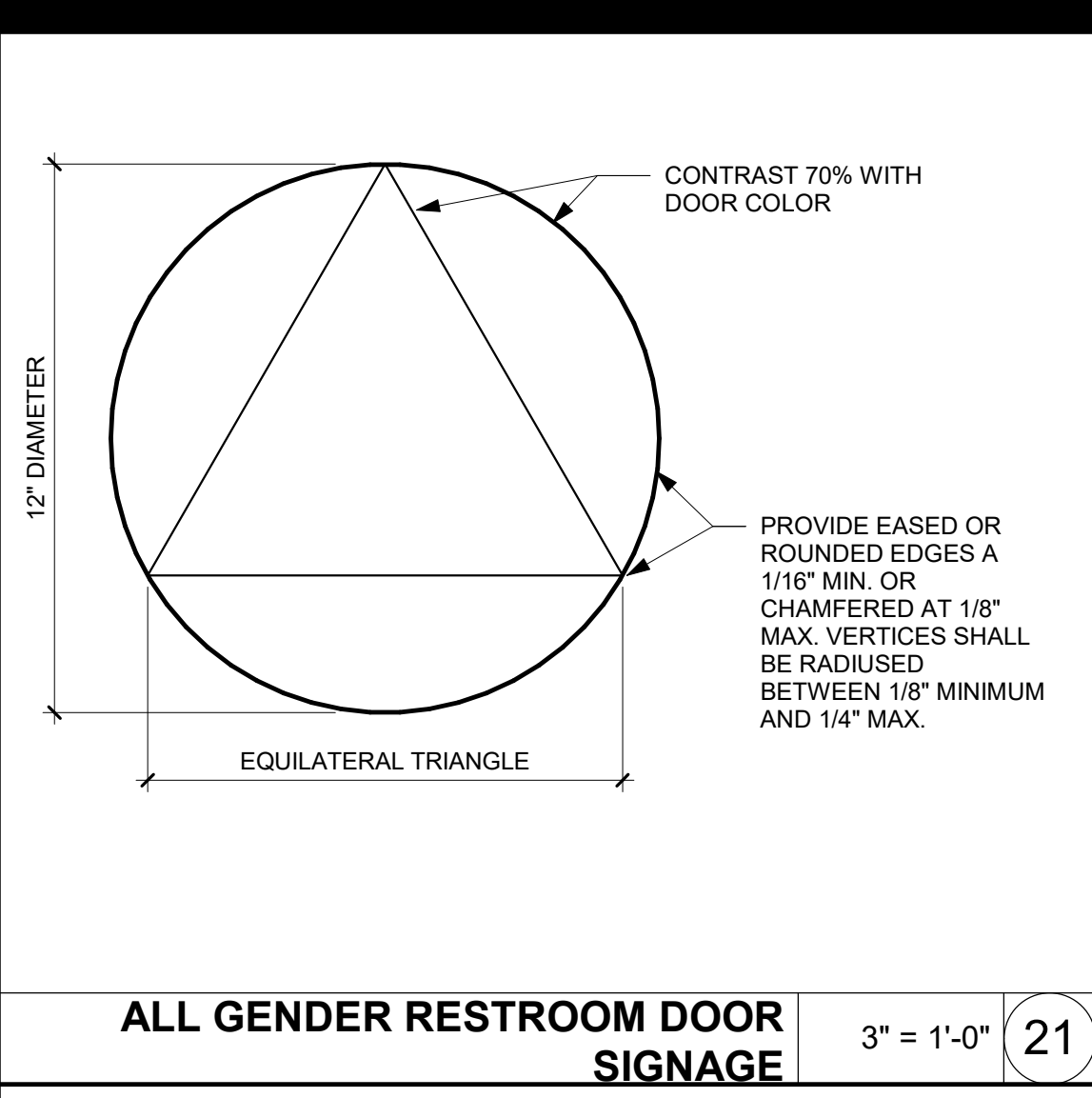
Checked  
Checker

Project Number  
22042

Drawing Number  
A10.2.1

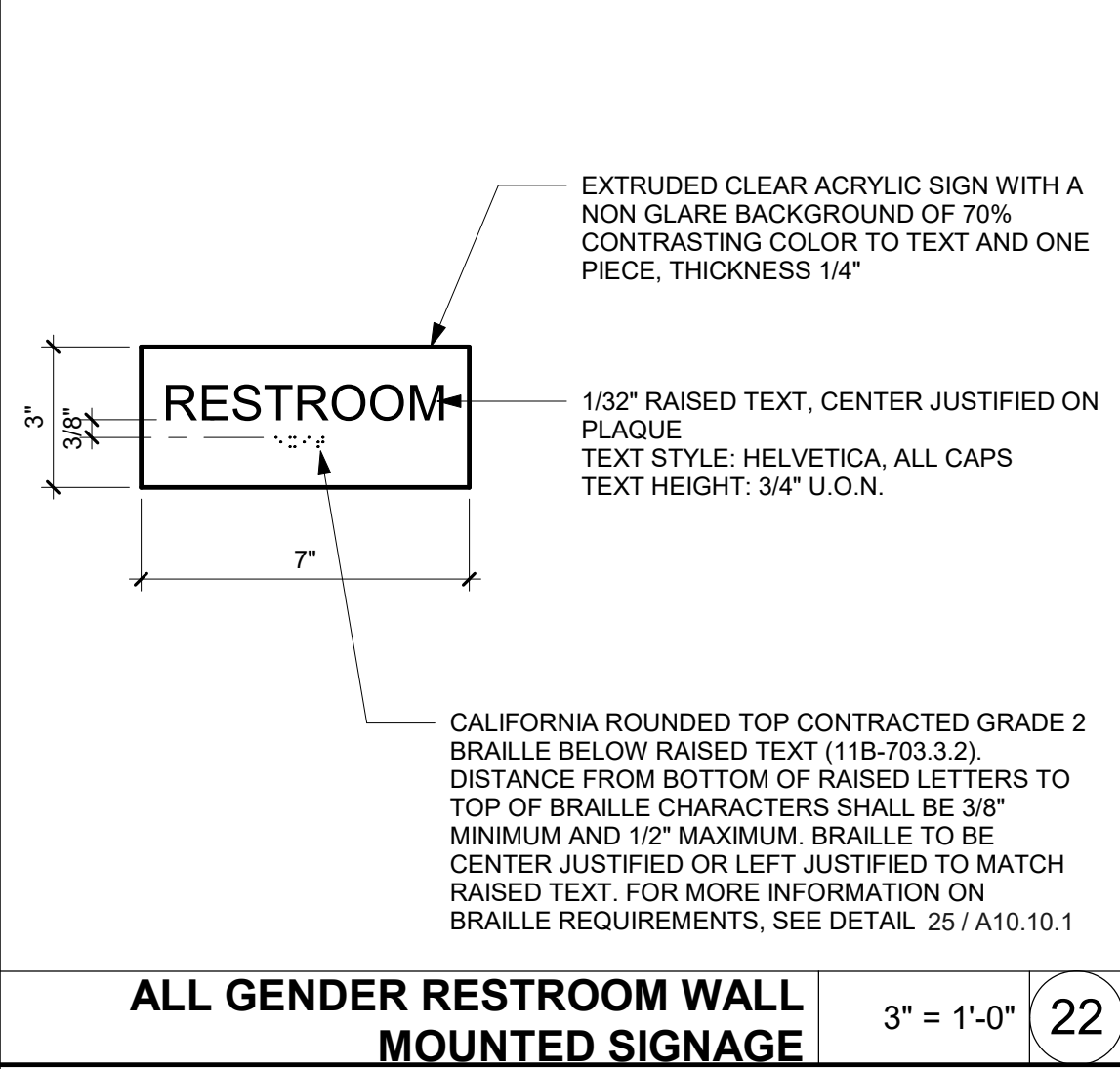


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ALL GENDER RESTROOM DOOR SIGNAGE

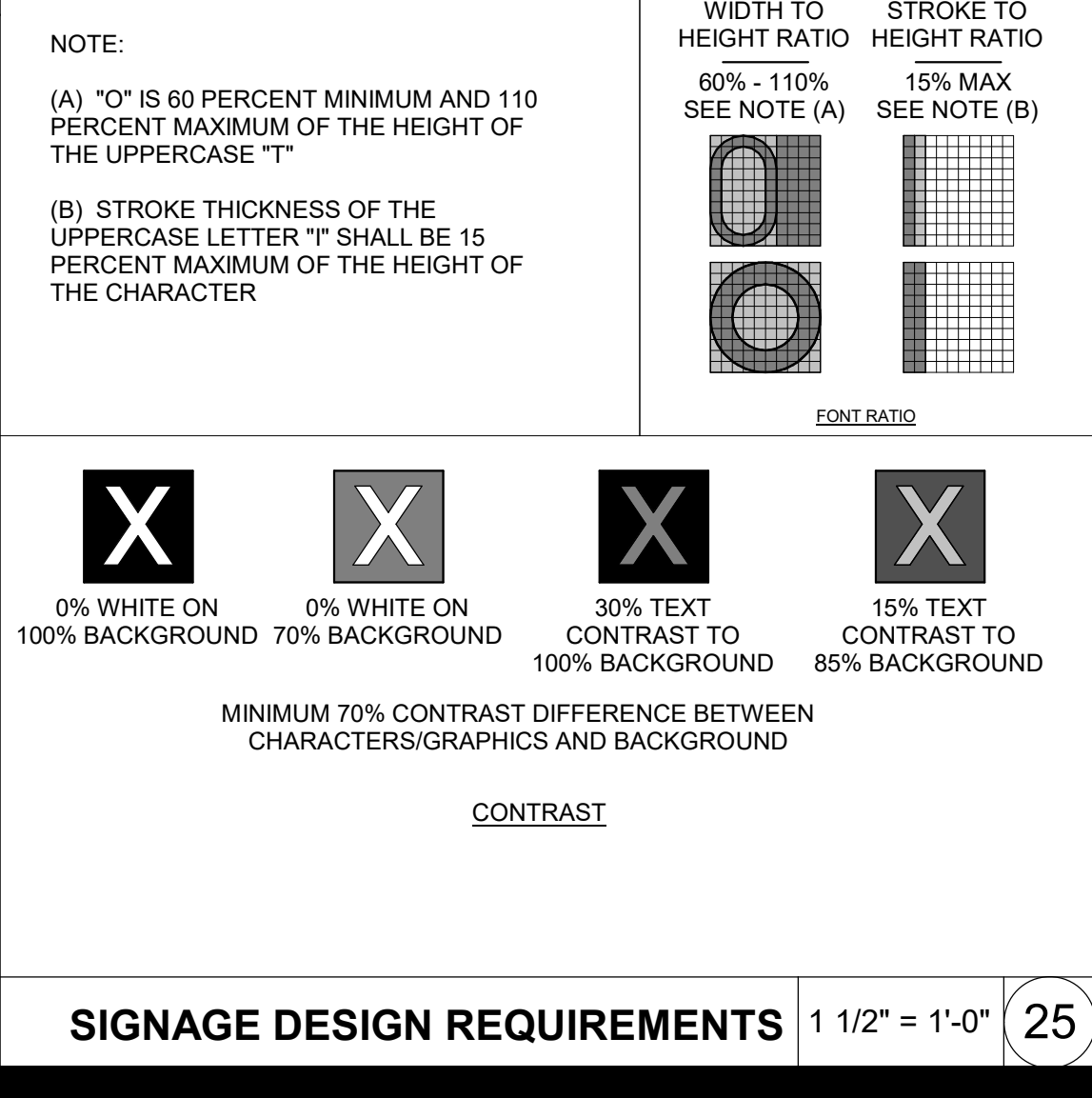
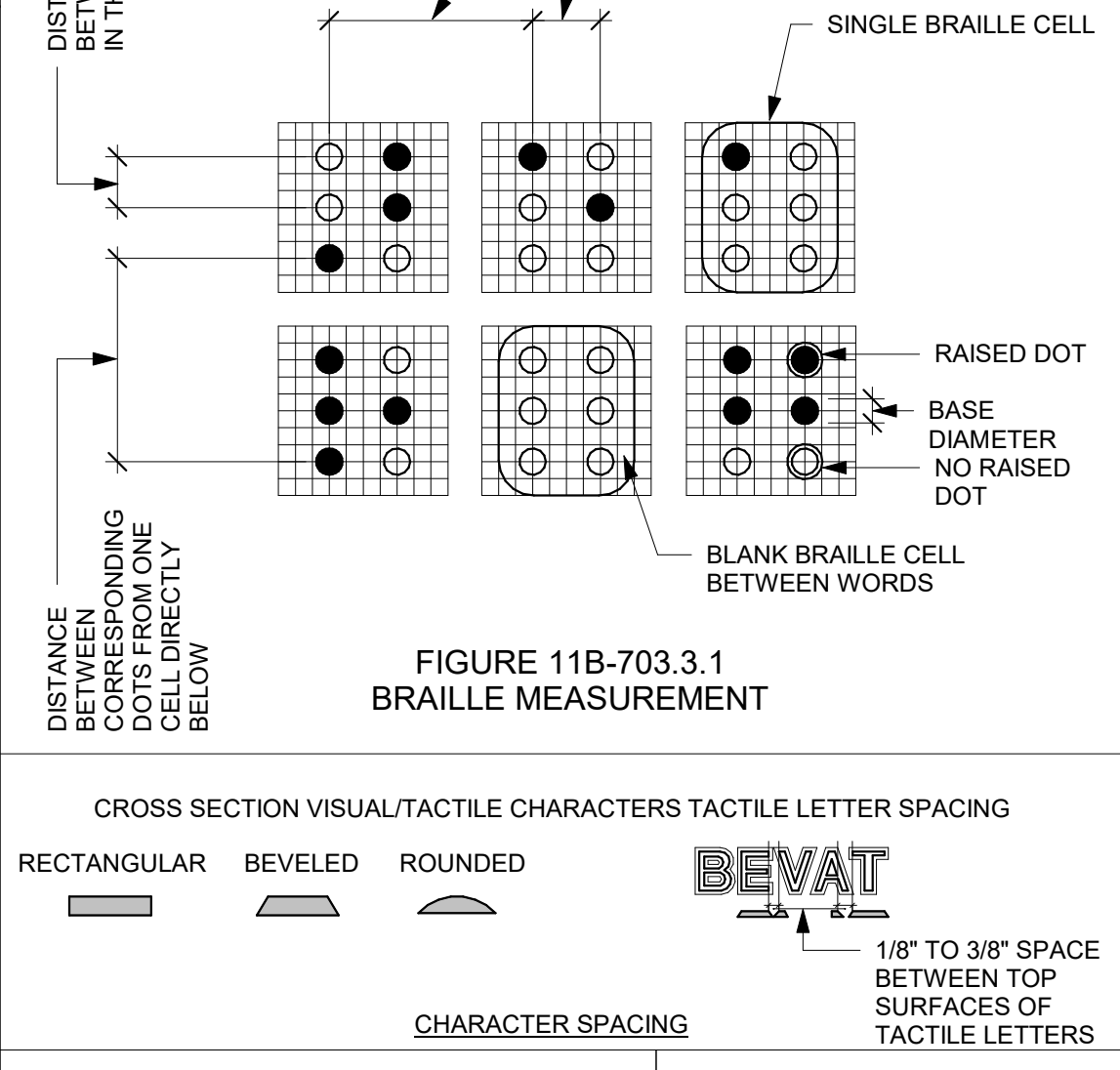
3" = 1'-0" 21



ALL GENDER RESTROOM WALL MOUNTED SIGNAGE

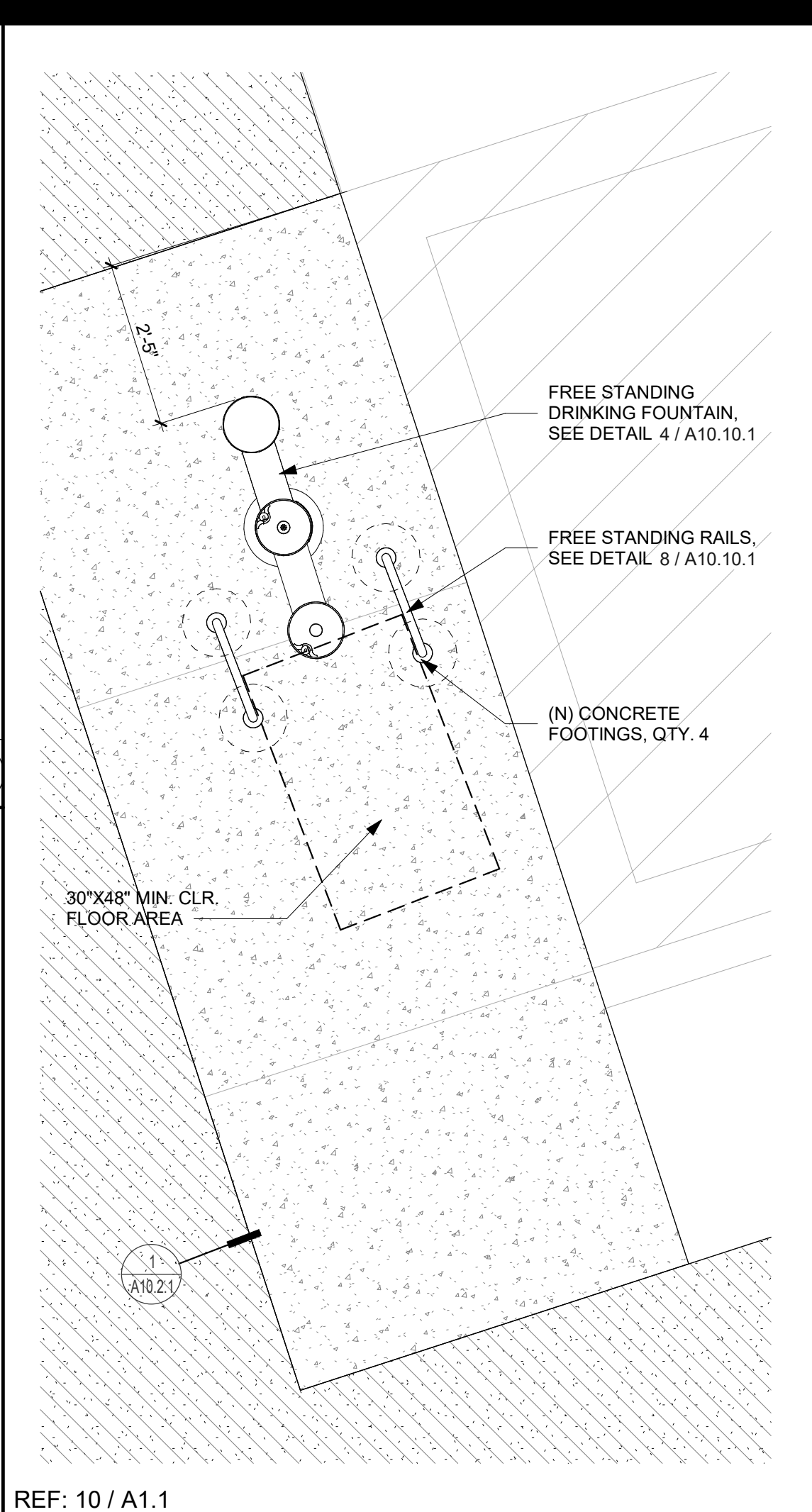
3" = 1'-0" 22

TABLE 11B-703.3.1 BRAILLE DIMENSIONS	
MEASUREMENT RANGE	MINIMUM IN INCHES MAXIMUM IN INCHES
DOT BASE DIAMETER	0.059" (1.5MM) TO 0.063" (1.6MM)
DISTANCE BETWEEN TWO DOTS IN THE SAME CELL *	0.100" (2.5MM)
DISTANCE BETWEEN CORRESPONDING DOTS IN ADJACENT CELLS *	0.300" (7.6MM)
DOT HEIGHT RANGE	0.025" (0.6MM) TO 0.037" (0.9MM)
DISTANCE BETWEEN CORRESPONDING DOTS FROM ONE CELL DIRECTLY BELOW *	0.395" (10MM) TO 0.400" (10.2MM)
* MEASURED CENTER TO CENTER	



SIGNAGE DESIGN REQUIREMENTS

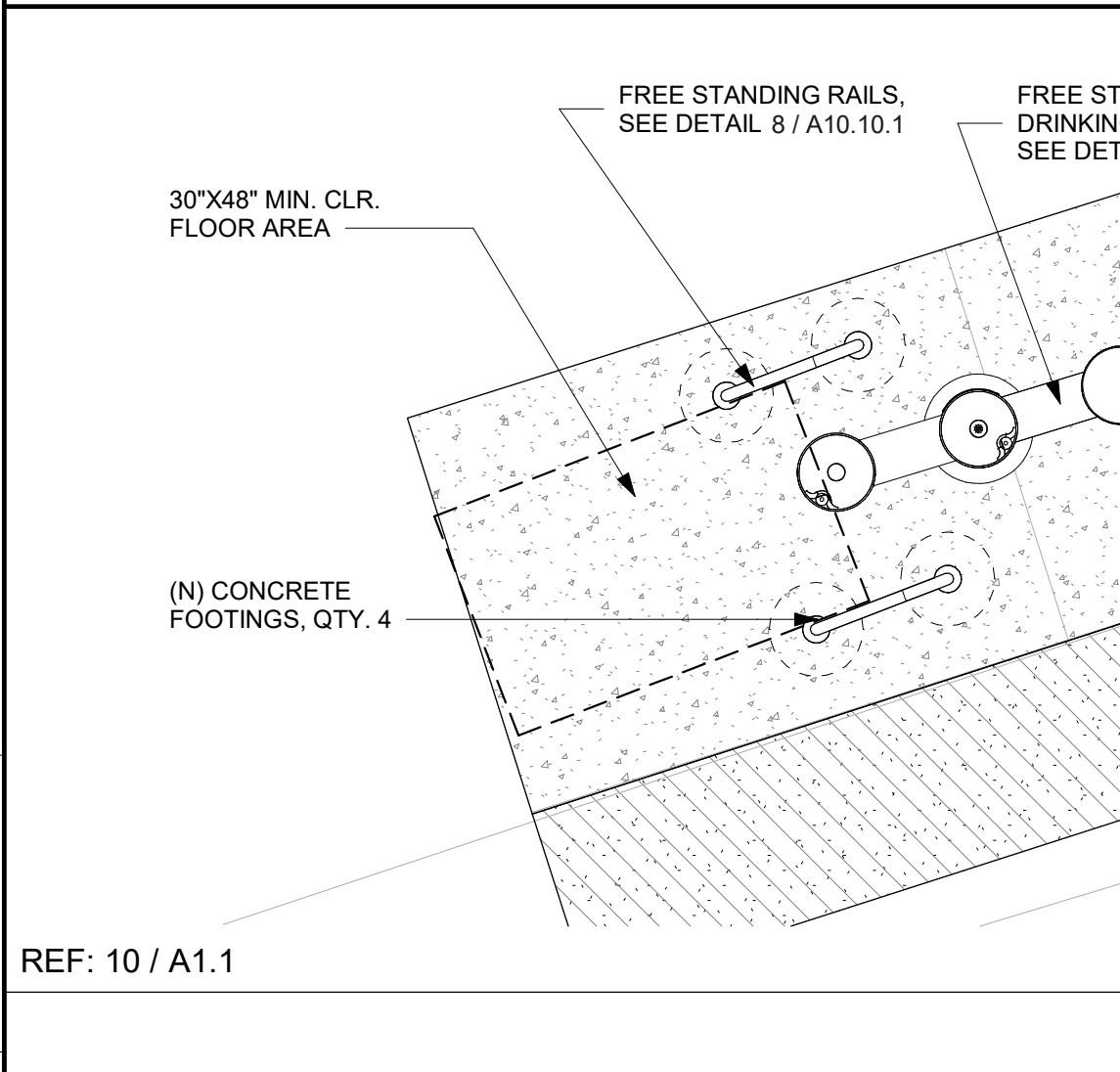
1 1/2" = 1'-0" 25



ENLARGED DF PLAN AT BUILDING P9

1/2" = 1'-0" 17

TABLE 11B-703.3.1 BRAILLE DIMENSIONS	
MEASUREMENT RANGE	MINIMUM IN INCHES MAXIMUM IN INCHES
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* MEASURED CENTER TO CENTER	



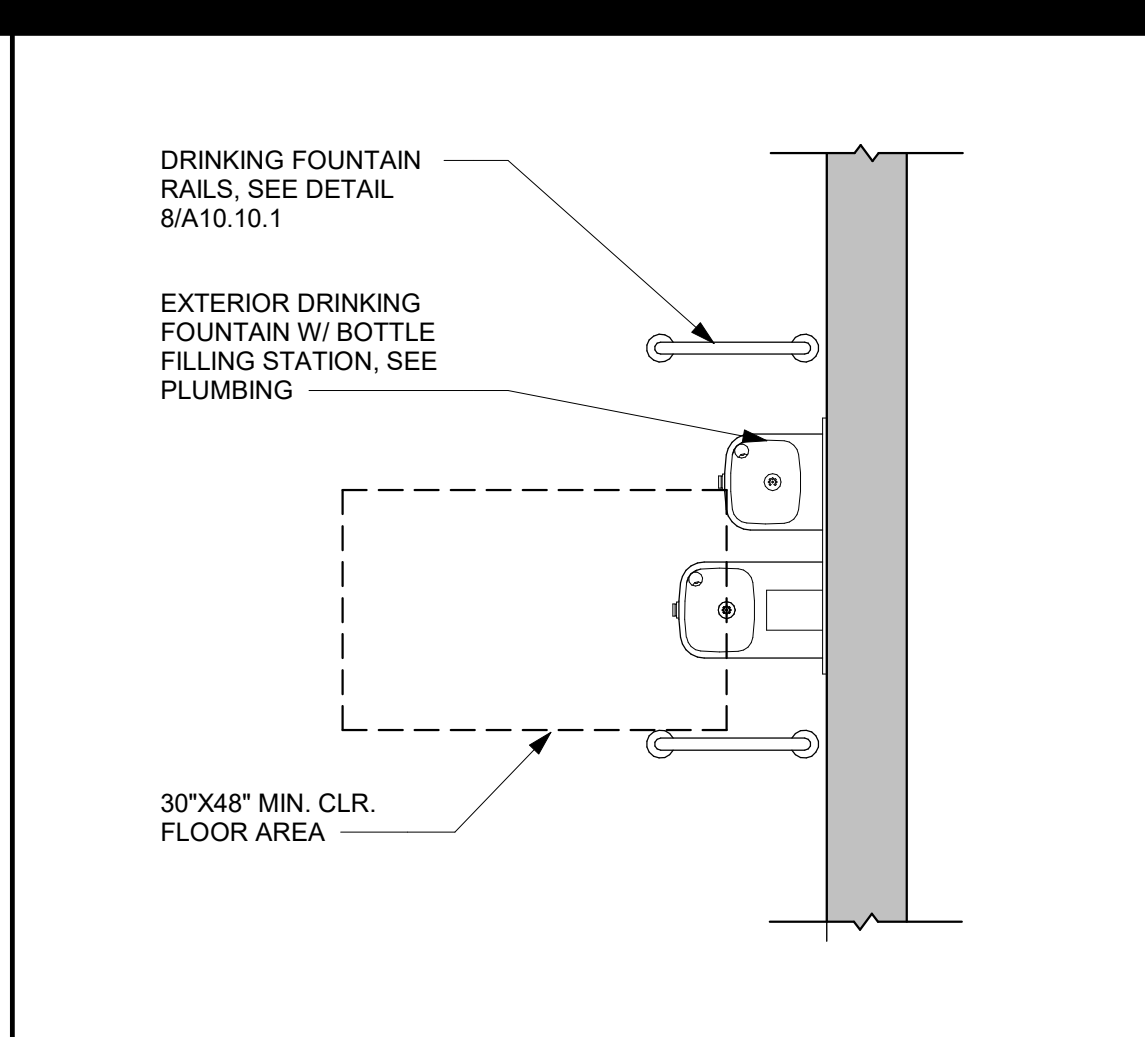
ENLARGED DF PLAN AT BUILDING W

1/2" = 1'-0" 14



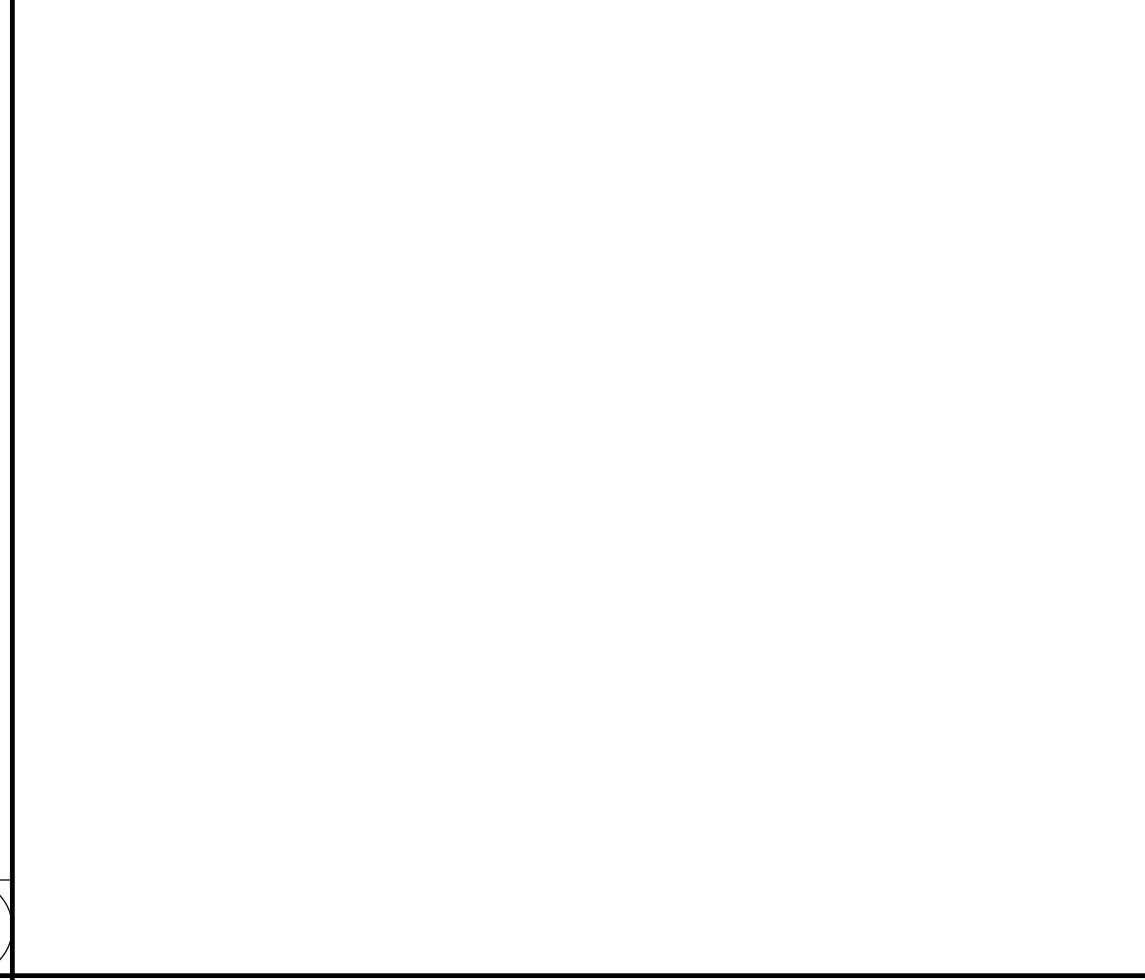
GUARDRAIL ATTACHMENT

3" = 1'-0" 15



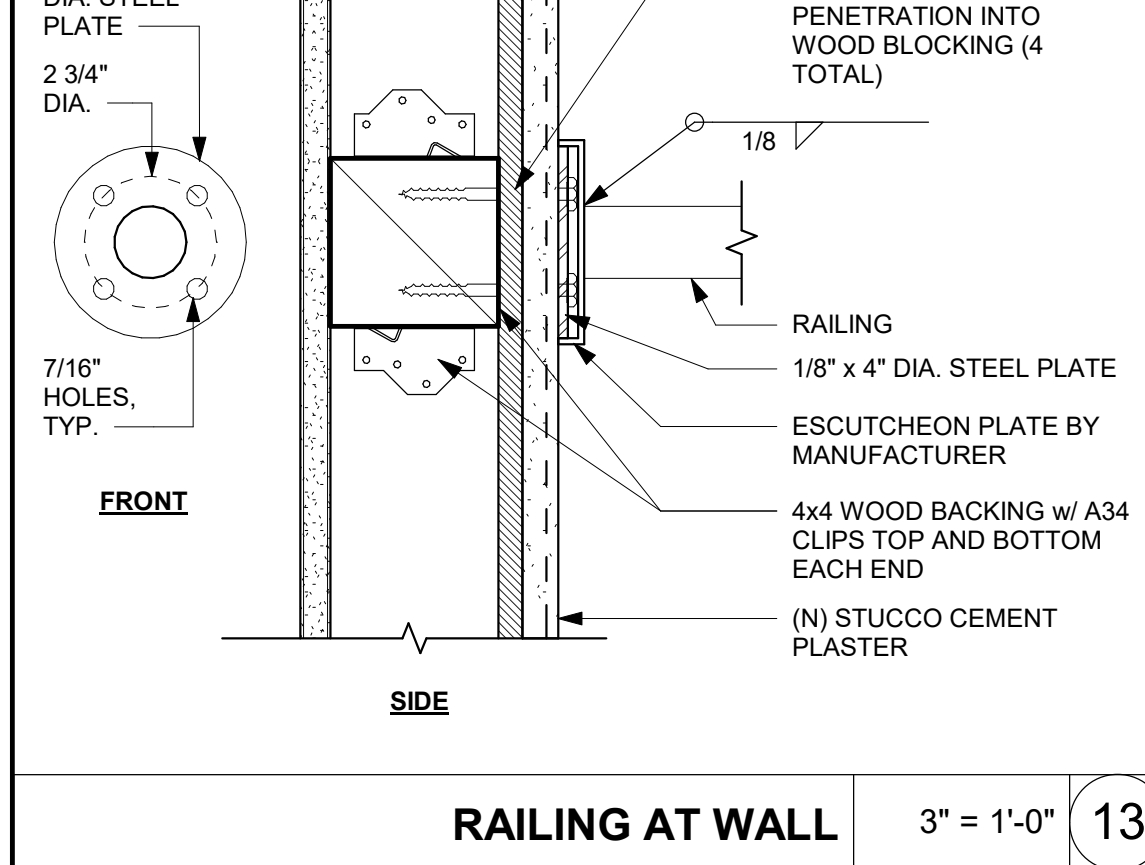
ENLARGED HI-LO EXTERIOR DRINKING FOUNTAIN PLAN

1/2" = 1'-0" 11



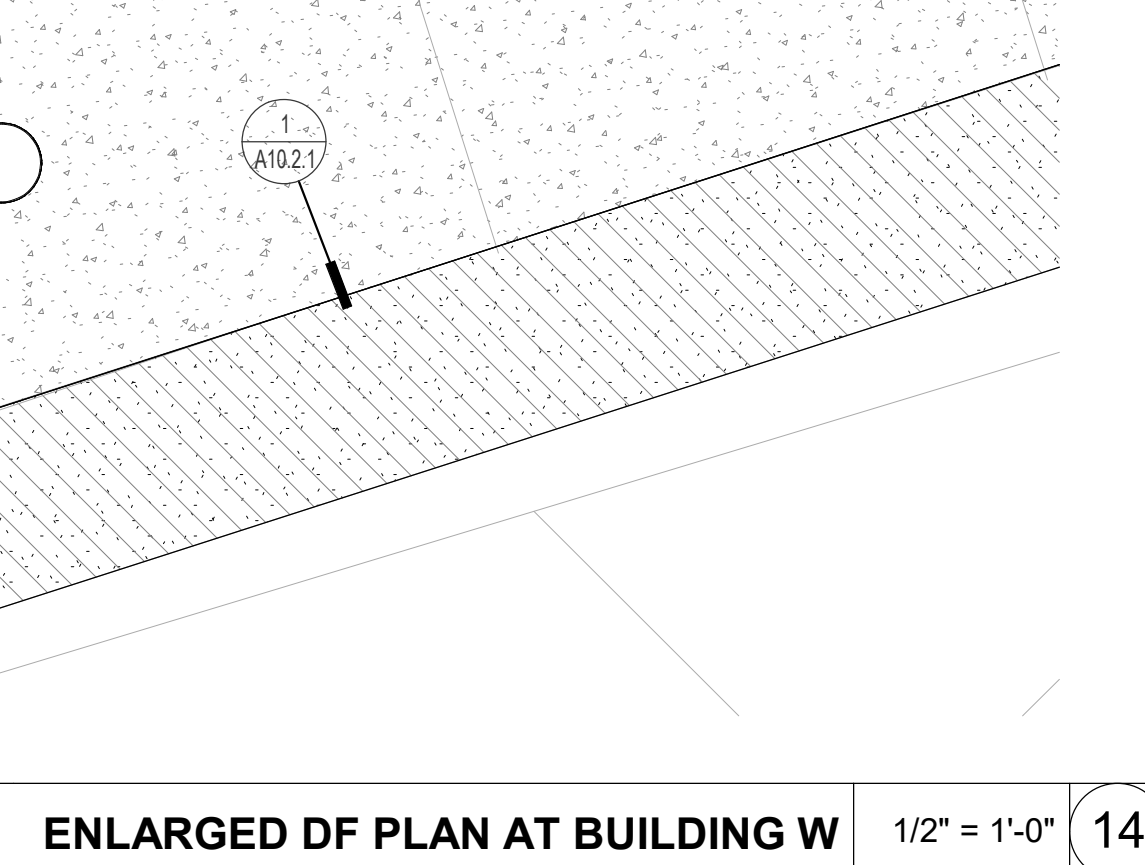
INT. HI-LO DF W/ BOTTLE FILLER SECTION & CLEARANCES

3/4" = 1'-0" 7



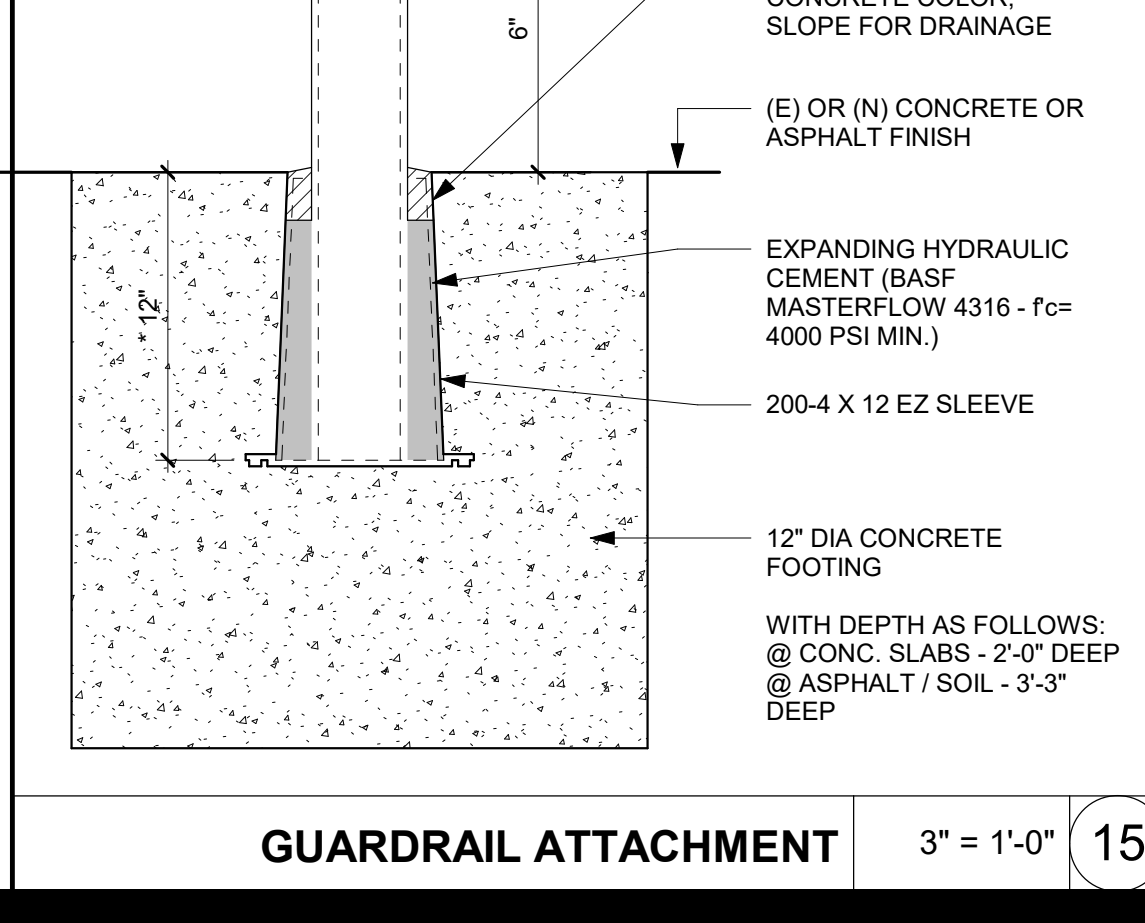
RAILING AT WALL

3" = 1'-0" 13



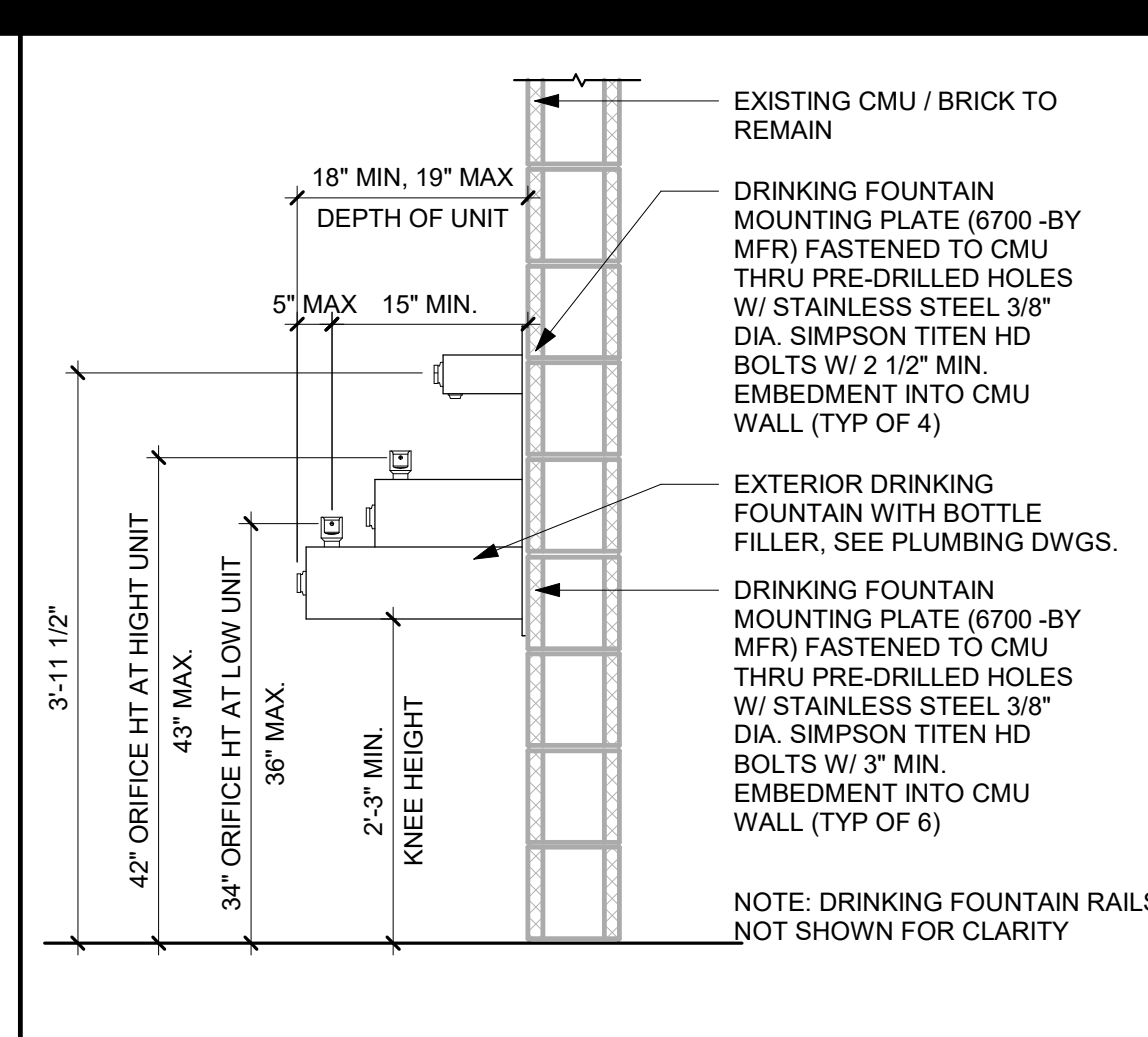
ENLARGED DF PLAN AT BUILDING W

1/2" = 1'-0" 14



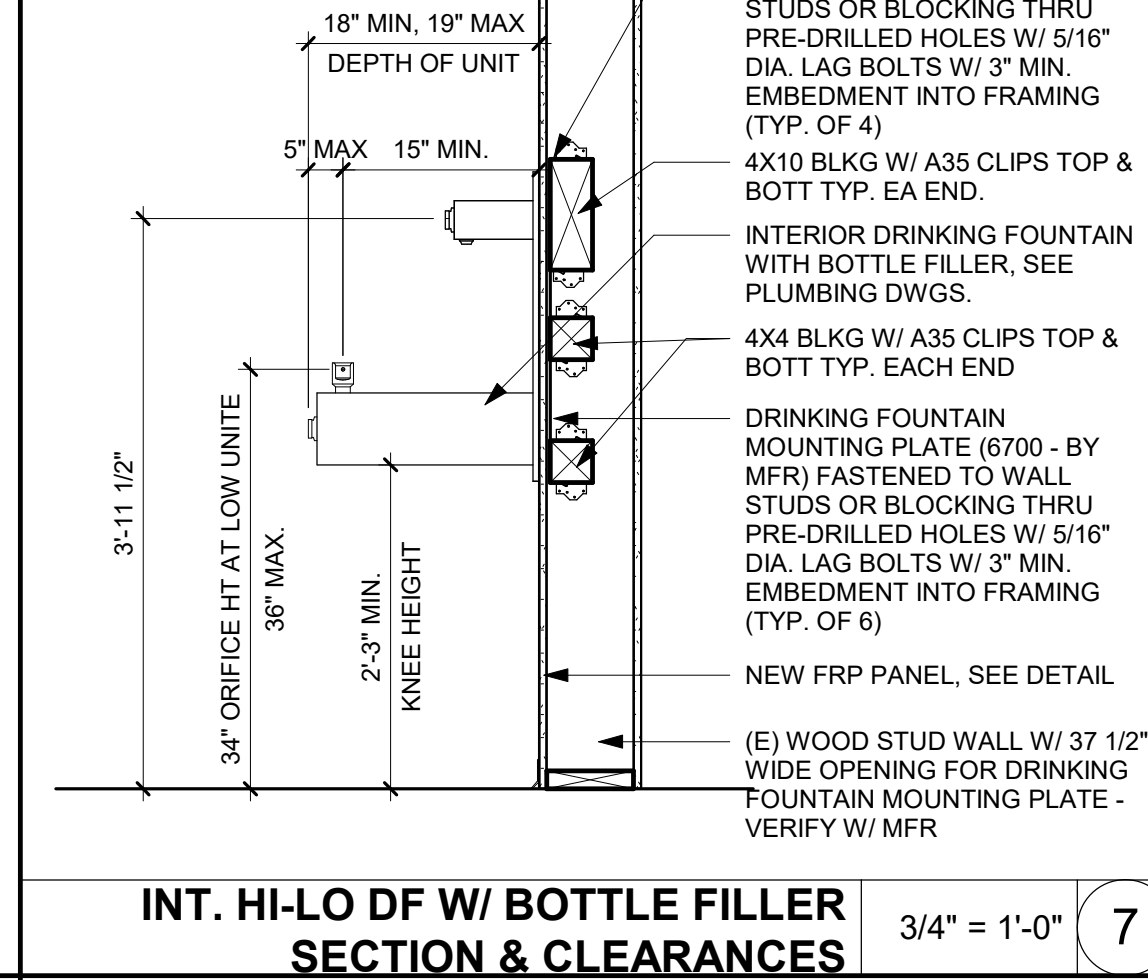
GUARDRAIL ATTACHMENT

3" = 1'-0" 15



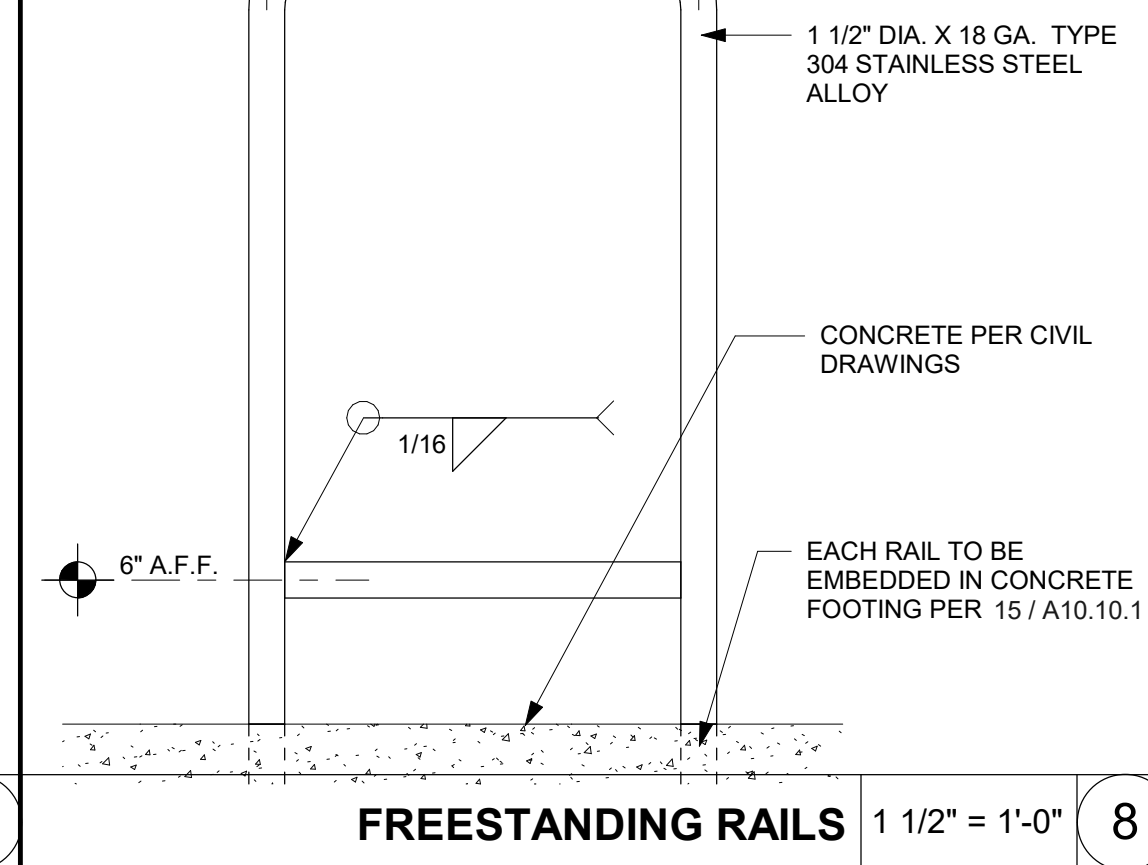
HI-LO DF WITH BOTTLE FILLER SECTION AND CLEARANCES

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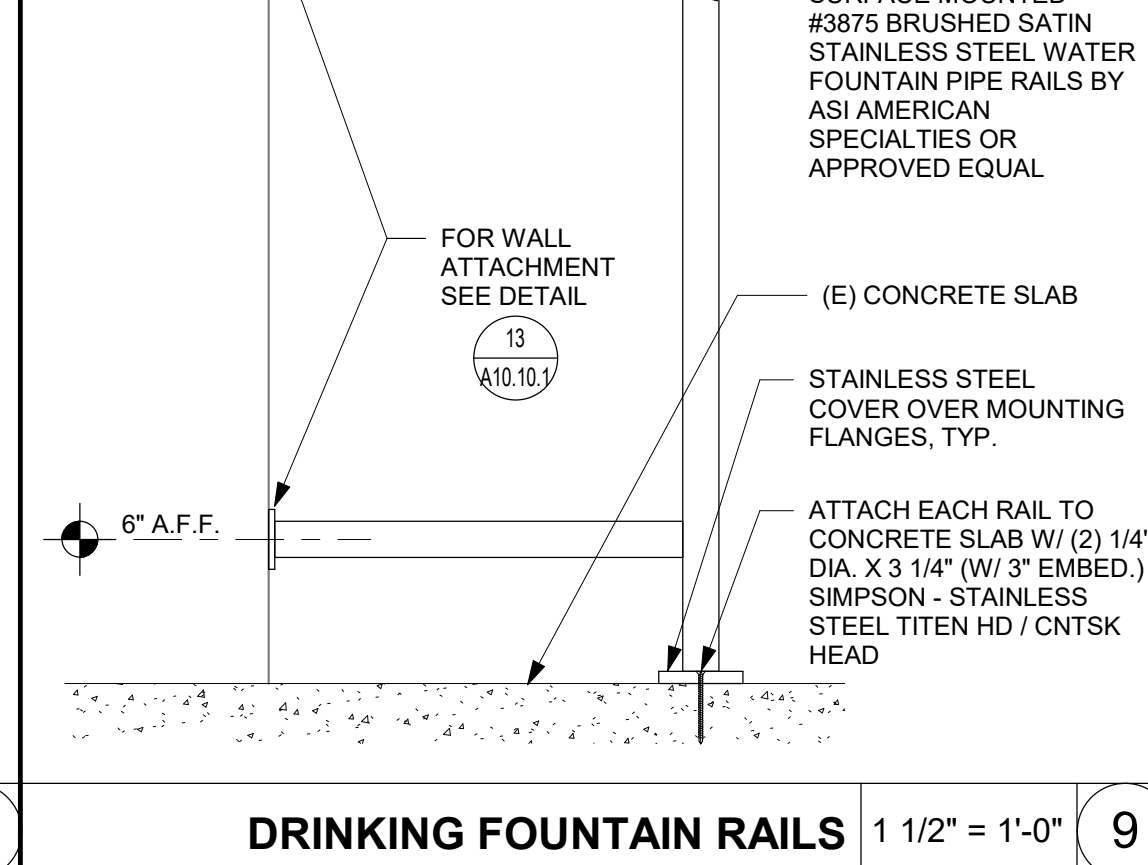
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3/4" = 1'-0" 7



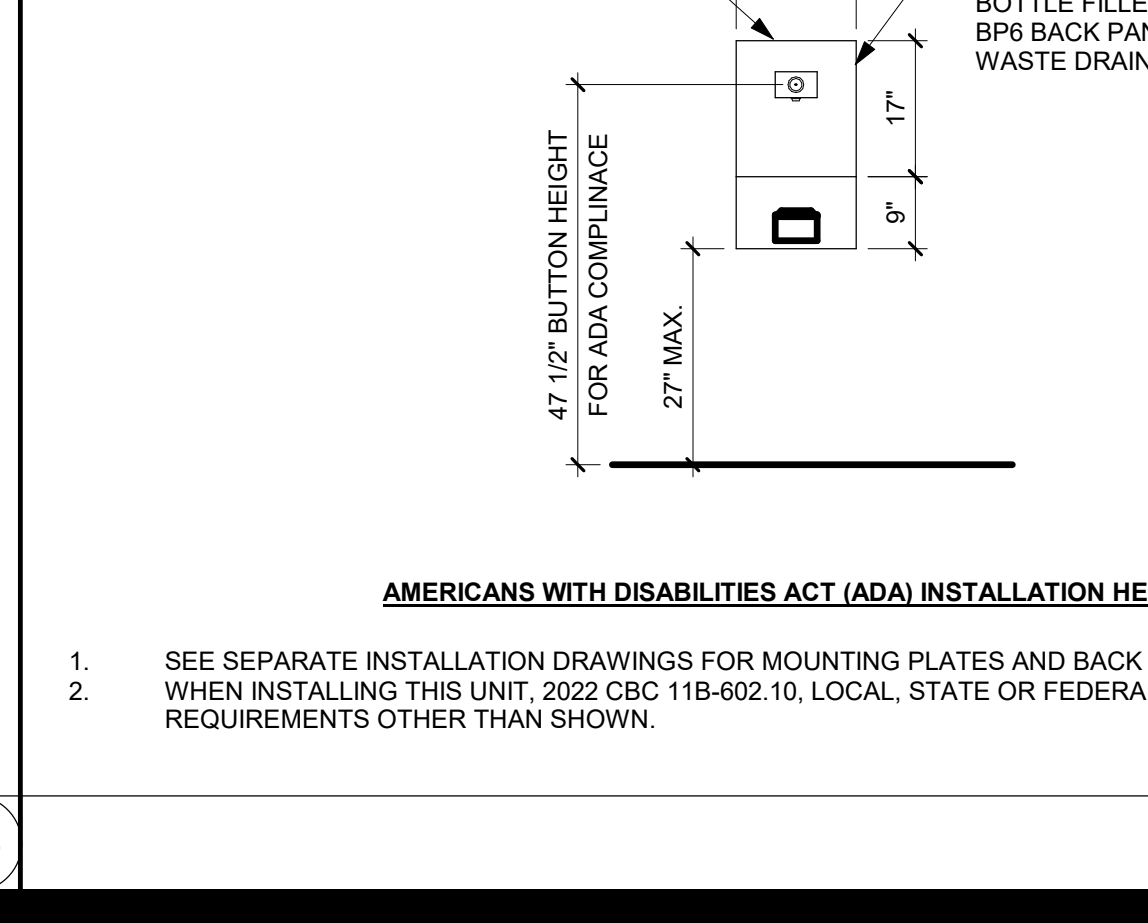
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3" = 1'-0" 13



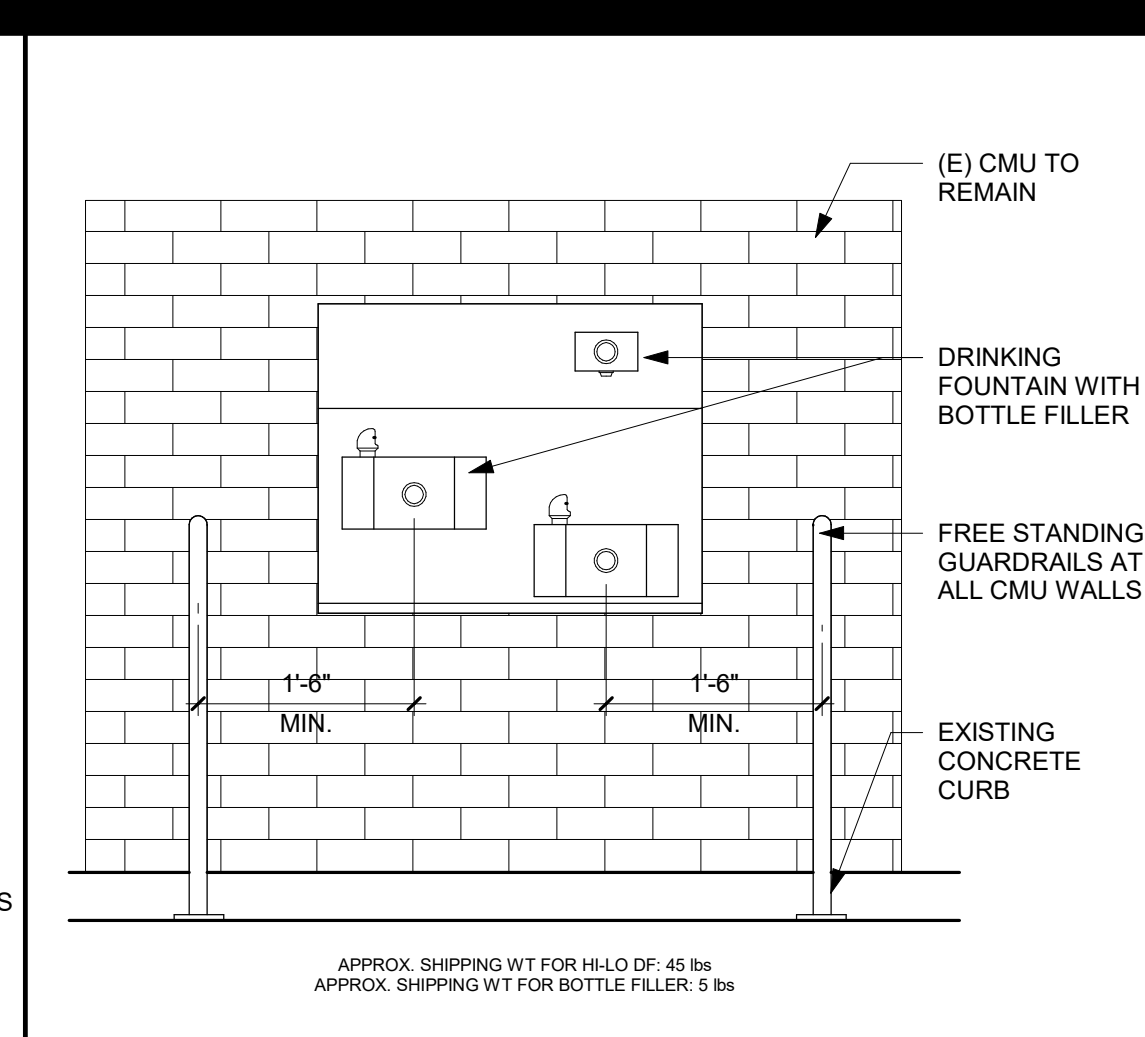
ENLARGED DF PLAN AT BUILDING W

1/2" = 1'-0" 14



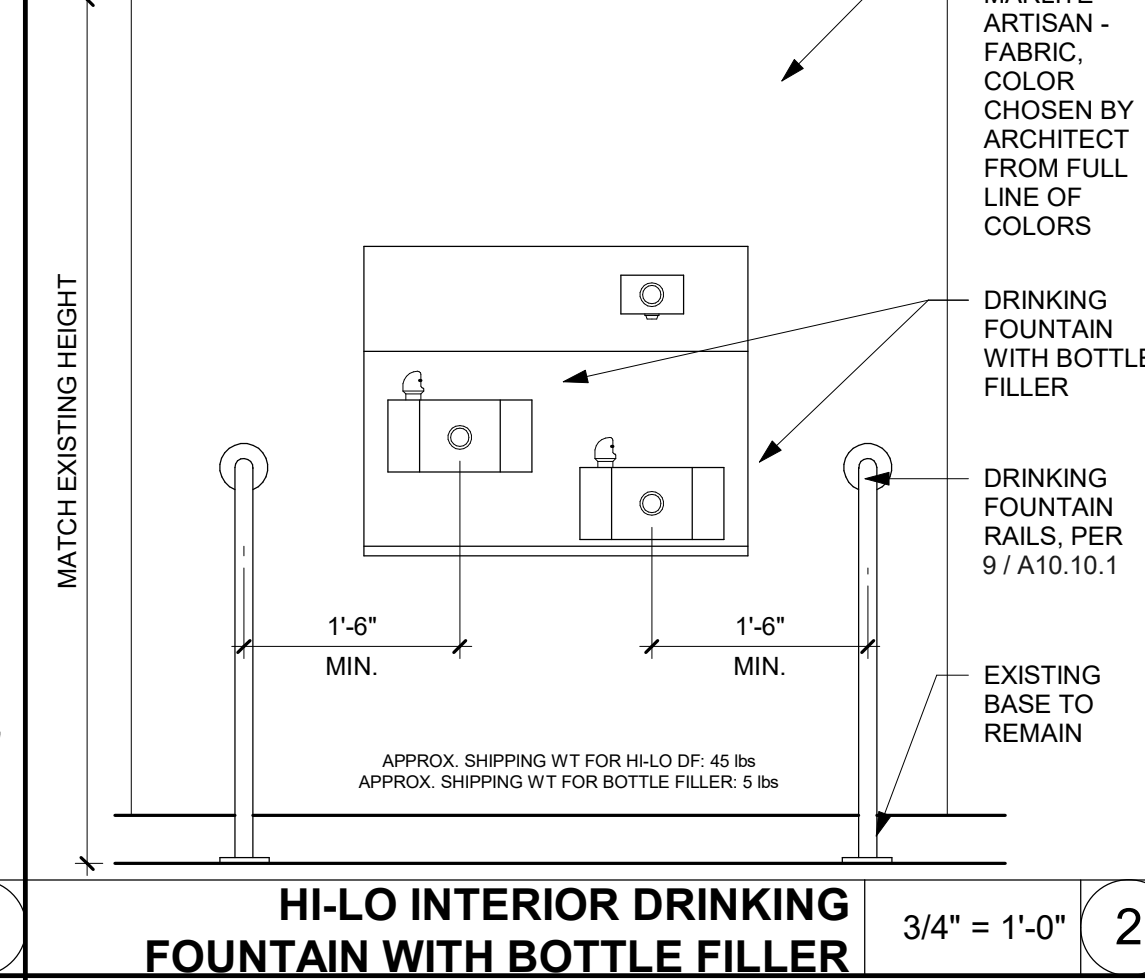
GUARDRAIL ATTACHMENT

3" = 1'-0" 15



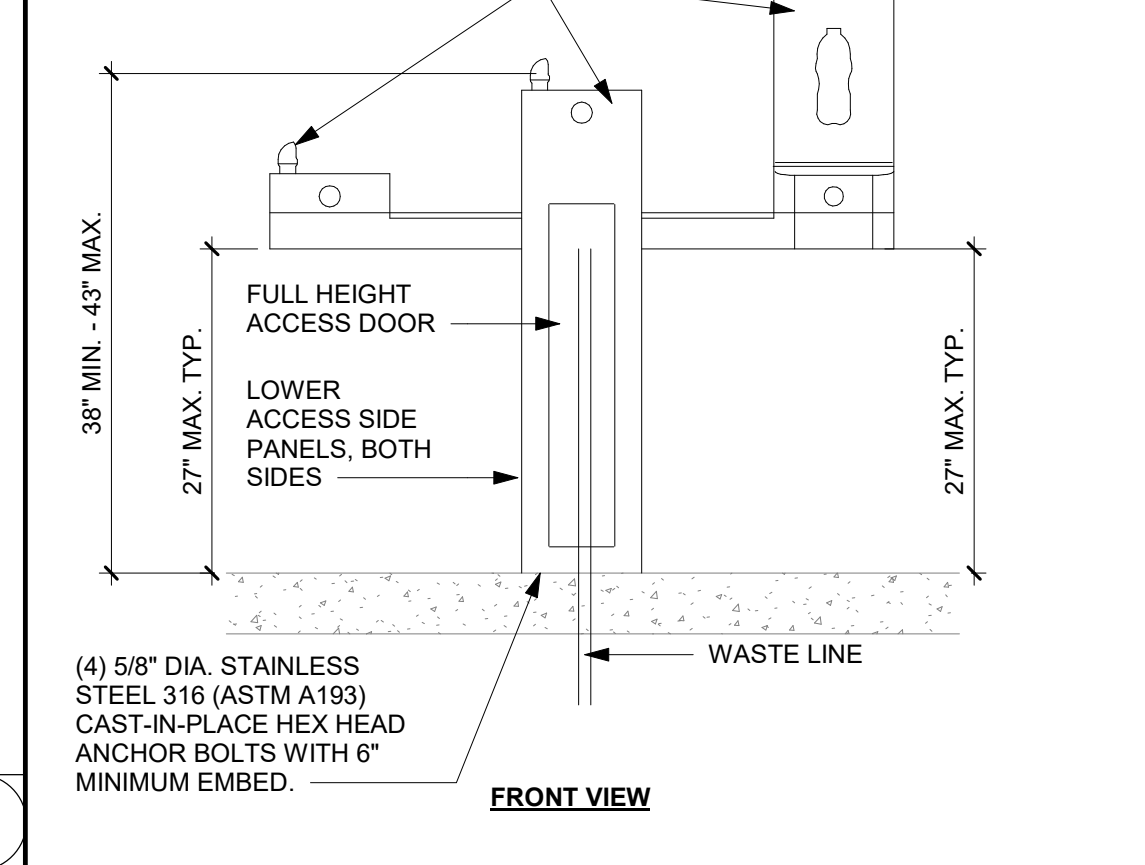
HI-LO EXTERIOR DRINKING FOUNTAIN WITH BOTTLE FILLER

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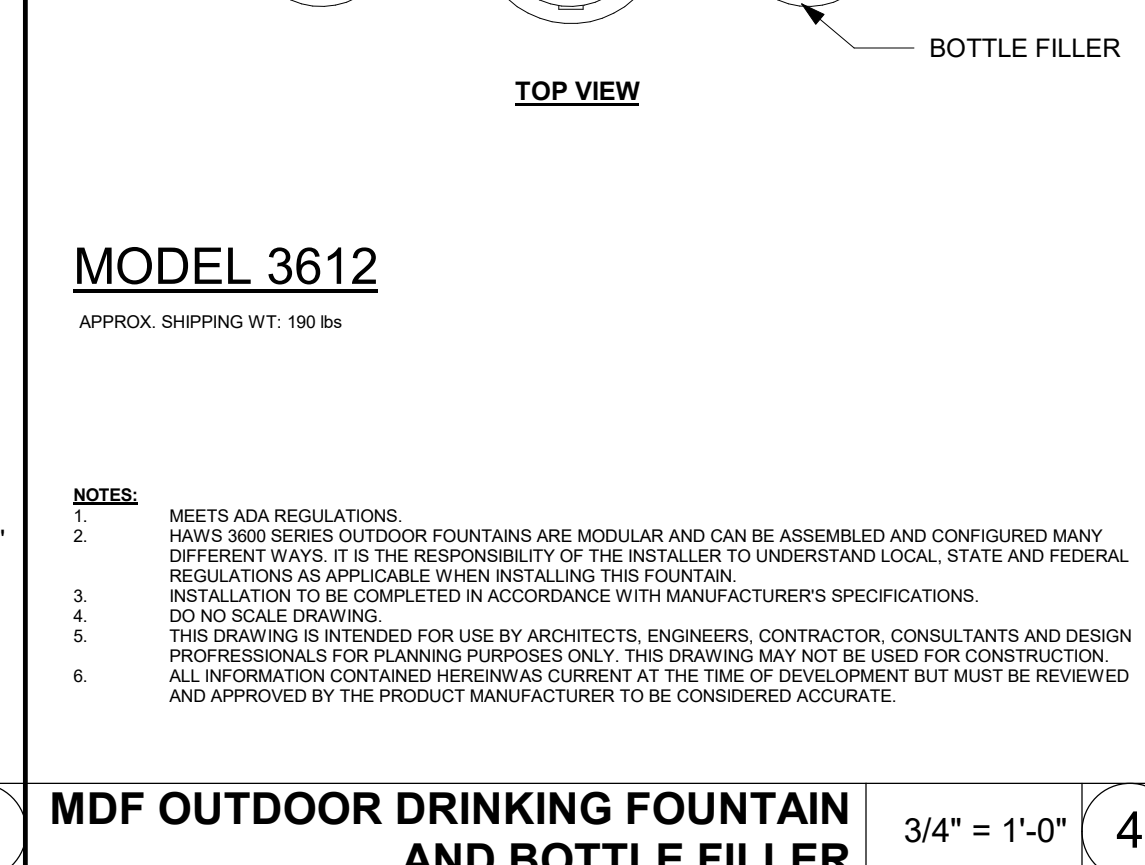
HI-LO INTERIOR DRINKING FOUNTAIN WITH BOTTLE FILLER

3/4" = 1'-0" 2



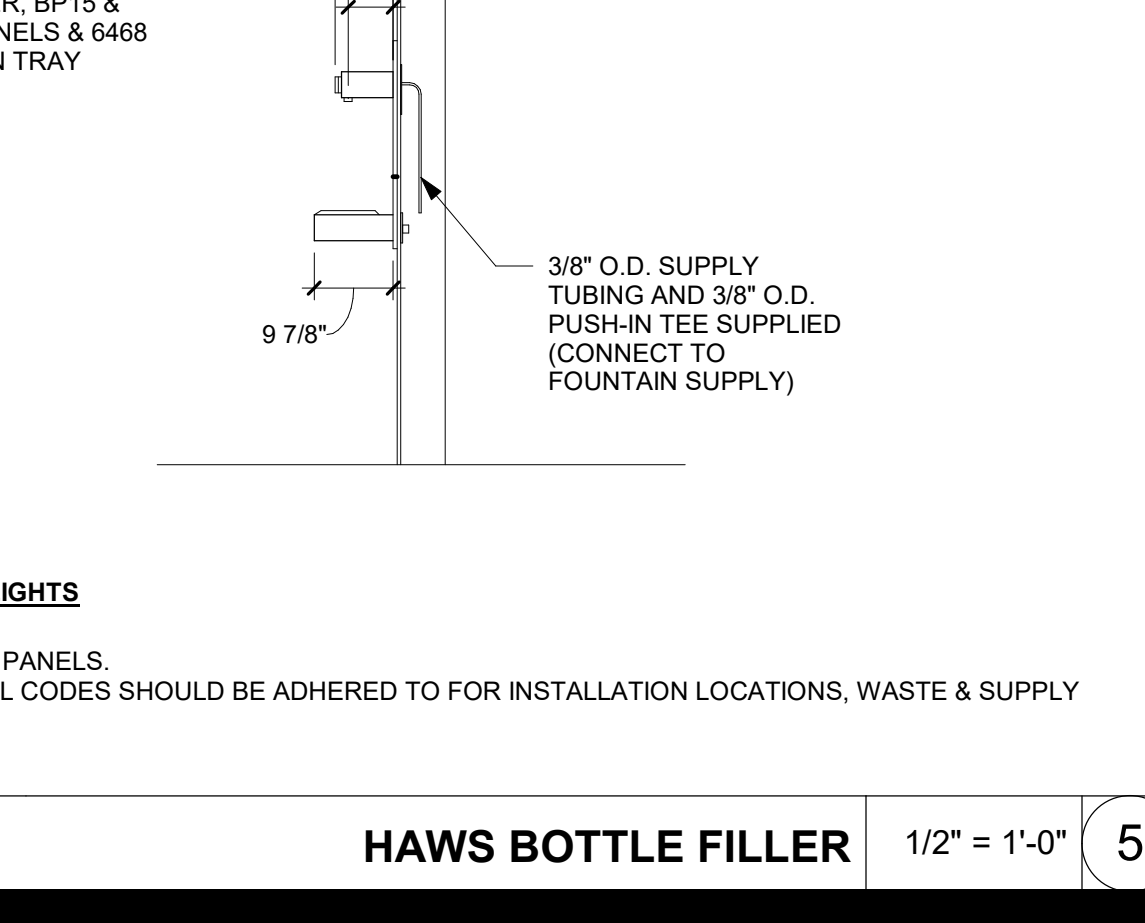
RAILING AT WALL

3" = 1'-0" 13



ENLARGED DF PLAN AT BUILDING W

1/2" = 1'-0" 14



GUARDRAIL ATTACHMENT

3" = 1'-0" 15

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122273 INC:  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☒  
DATE: 05/03/2024

STUDIO W  
ARCHITECTS

Studio W Architects  
1930 H Street  
Sacramento, California 95811  
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ARCHITECT  
ENGINEER

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NO. REMARKS DATE

DATE

DSA PLAN CHECK  
DSA BACK CHECK  
BIDDING  
CONSTRUCTION

KEY PLAN

WASHINGTON UNIFIED  
SCHOOL DISTRICT  
930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

PROJECT STATUS

WUSD RIVERBANK ES  
ESSR III  
1100 CARRIE STREET  
WEST SACRAMENTO, CA 95605

SPECIALTIES

Date: 03/13/2024  
Application Number: 02-122273  
Drawing Number: A10.10.1  
Author: [ ]  
Checked: [ ]  
Project Number: 22042  
Drawing Number: A10.10.1



PLUMBING LEGEND					
SYMBOL	ABBREVIATION	DESCRIPTION	ABBREVIATION	DESCRIPTION	DESCRIPTION
	S	SEWER PIPE	ABV	ABOVE	
	OW	OILY WASTE PIPE	A/C	ABOVE CEILING	
	GW	GREASE WASTE PIPE	AGA	AMERICAN GAS ASSOCIATION	
	PW	PUMPED (FORCED) WASTE PIPE	ANSI	AMERICAN NATIONAL STANDARD INSTITUTE	
	IW	INDIRECT WASTE PIPE	ASME	AMERICAN SOCIETY FOR MECHANICAL ENGINEERS	
	V	VENT PIPE	ASSE	AMERICAN SOCIETY FOR SANITARY ENGINEERS	
	CW	COLD WATER PIPE	ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	
	ICW	INDUSTRIAL COLD WATER PIPE	ADA	AMERICANS WITH DISABILITIES ACT	
	SCW	SOFT COLD WATER PIPE	AFF	ABOVE FINISHED FLOOR	
	HW	HOT WATER PIPE	AFS	ABOVE FINISHED GRADE	
	IHW	INDUSTRIAL HOT WATER PIPE	A/G	ABOVE GRADE	
	HWR	HOT WATER RETURN PIPE	AP	ACCESS PANEL	
	140	140°F HOT WATER PIPE	ARCH	ARCHITECT OR ARCHITECTURAL	
	R	RECLAIMED WATER PIPE	BT	BATH TUB	
	G	LOW PRESSURE NATURAL GAS PIPE	BEL	BELOW	
	MPG	MEDIUM PRESSURE NATURAL GAS PIPE	B/F	BELOW FLOOR	
	HPG	HIGH PRESSURE NATURAL GAS PIPE	B/G	BELOW GRADE	
	LPG	LIQUEFIED PETROLEUM GAS PIPE	BOP	BOTTOM OF PIPE	
	CD	CONDENSATE DRAIN PIPE	B/S	BELOW SLAB	
	SCD	SECONDARY CONDENSATE DRAIN PIPE	BTU	BRITISH THERMAL UNIT	
	PCD	PUMPED CONDENSATE DRAIN PIPE	CBC	CALIFORNIA BUILDING CODE	
	RD	ROOF DRAIN PIPE	CEC	CALIFORNIA ELECTRICAL CODE	
	ORD	OVERFLOW ROOF DRAIN PIPE	CFC	CALIFORNIA FIRE CODE	
	CA	COMPRESSED AIR PIPE	CMC	CALIFORNIA MECHANICAL CODE	
	FCO	FLOOR CLEAN OUT	CPC	CALIFORNIA PLUMBING CODE	
	GCO	GRADE CLEAN OUT	CI	CAST IRON	
	WCO	WALL CLEAN OUT	CI	CAST IRON	
	FC	FLEXIBLE CONNECTION	CI	CAST IRON	
	SOV	SHUT OFF VALVE	CL	CLEAR	
	GC	GAS COCK	CL	CLEAR	
	CV	CHECK VALVE	CL	CLEAR	
	BV	BALL VALVE	CL	CLEAR	
	PRV	PRESSURE REDUCING VALVE	CL	CLEAR	
	BLV	BALANCING VALVE	CL	CLEAR	
	PTR	PRESSURE AND TEMPERATURE RELIEF VALVE	CL	CLEAR	
	U	UNION	CL	CLEAR	
		CAPPED PIPE	CL	CLEAR	
	CONT	CONTINUED OR CONTINUATION	CL	CLEAR	
	TP	TRAP PRIMER LINE	CL	CLEAR	
	WHA	WATER HAMMER ARRESTOR	CL	CLEAR	
	RPBP	REDUCED PRESSURE BACKFLOW PREVENTER	CL	CLEAR	
	HB	HOSE BIBB	CL	CLEAR	
		PIPE DOWN OR DROP	CL	CLEAR	
		PIPE UP OR RISE	CL	CLEAR	
		VALVE ON DROP	CL	CLEAR	
		VALVE ON RISE	CL	CLEAR	
	T	THERMOMETER	CL	CLEAR	
	AS	AQUASTAT	CL	CLEAR	
	P.O.D.	POINT OF DISCONNECT	CL	CLEAR	
	POC	POINT OF CONNECTION	CL	CLEAR	
	AD, FD	AREA DRAIN OR FLOOR DRAIN	CL	CLEAR	
	FS, RR	FLOOR SINK OR ROOF RECEPTOR	CL	CLEAR	
	VTR	VENT THROUGH ROOF	CL	CLEAR	
	DEMO	DEMOLITION OR DEMOLISH	CL	CLEAR	
	RELO	RELOCATE	CL	CLEAR	
	CIRC PUMP	CIRCULATING PUMP	CL	CLEAR	
	DIA, DIAM	DIAMETER	CL	CLEAR	

PLUMBING GENERAL NOTES:	
1. THESE DOCUMENTS MAY NOT BE USED FOR ANY REPRODUCTION, BIDDING, OR CONSTRUCTION UNLESS AUTHORIZED IN WRITING BY SALAS O'BRIEN AND THE ENGINEER OF RECORD RESPONSIBLE FOR THEIR PREPARATION.	
2. CONTRACTOR SHALL FIELD VERIFY THE LOCATIONS OF ALL EXISTING UTILITY PIPES PRIOR TO START OF WORK. NECESSARY ADJUSTMENTS TO THE PLUMBING LAYOUT SHALL BE DONE AT NO EXTRA COST.	
3. CONTRACTOR SHALL NOTIFY ALL LOCAL UTILITY COMPANIES INCLUDING BUT NOT LIMITED TO THE GAS COMPANY, ELECTRIC COMPANY, TELEPHONE COMPANY, AND THE WATER DEPARTMENT, ABOUT THE EXTENT OF PLUMBING WORK. ALL EXCAVATION WORK SHALL BE APPROVED BY ALL UTILITY COMPANIES TO ASSURE PREVENTION OF INTERRUPTION OF EXISTING SERVICES PRIOR TO START OF WORK.	
4. ALL PLUMBING WORK SHALL MEET OR EXCEED THE REQUIREMENTS OF THE CALIFORNIA PLUMBING CODE, CALIFORNIA BUILDING CODE, CALIFORNIA MECHANICAL CODE, CALIFORNIA ADMINISTRATIVE CODE, TITLE 24, AMERICANS WITH DISABILITIES ACT (ADA), NATIONAL FIRE PROTECTION ASSOCIATION (NFPA), THE LOCAL CITY AND COUNTY CODES, AND ALL OTHER CODES HAVING JURISDICTION. IN CASE OF CONFLICT, THE MORE STRICT REGULATIONS SHALL GOVERN.	
5. ALL PLUMBING WORK SHALL BE COORDINATED WITH THE WORKS OF OTHER TRADES PRIOR TO START OF WORK. NECESSARY ADJUSTMENTS SHALL BE MADE AT NO EXTRA COST.	
6. FOR MINIMUM PIPE SIZE CONNECTIONS TO EACH PLUMBING FIXTURE SEE PLUMBING FIXTURE SCHEDULE. THESE VALUES ARE MINIMUM; LARGER CONNECTIONS MAY RESULT BASED ON THE DIFFERENT MANUFACTURER'S RECOMMENDATIONS.	
7. MANUFACTURER'S NAMES AND MODEL NUMBERS SHOWN FOR PLUMBING FIXTURES AND EQUIPMENT ARE FOR REFERENCE ONLY. OTHER MANUFACTURERS WHICH CAN MEET THE DESIGN REQUIREMENTS OF THE PLUMBING SYSTEM MAY BE SUBSTITUTED UPON APPROVAL FROM THE ARCHITECT AND THE OWNER.	
8. PROVIDE DIELECTRIC FITTINGS FOR DISSIMILAR METALS IN CONTACT.	
9. PROVIDE HANGERS AND SUPPORTS FOR PIPING IN ACCORDANCE WITH THE RECOMMENDATIONS OF MSS SP-69-2003.	
10. PROVIDE VALVES AT THE FOLLOWING LOCATIONS: A. WATER MAIN SHUT-OFF VALVE IN VALVE BOX. B. VALVE WITH HOSE CONNECTION ON DOWNSTREAM SIDE OF THE MAIN SHUT-OFF VALVE. C. SHUT-OFF VALVE ON EACH SUPPLY TO EACH FIXTURE AND EQUIPMENT ITEM NOT PROVIDED WITH CONTROL STOP OR OTHER AUXILIARY SHUT-OFF VALVE. INSTALL SHUT-OFF VALVES SO THAT STEMS EITHER ARE VERTICAL WITH HANDWHEELS OR OPERATORS ON TOP OR ARE HORIZONTAL AND SO THAT VALVES ARE EASILY ACCESSIBLE FOR OPERATION, SERVICE, REMOVAL AND REPLACEMENT.	
11. PROVIDE SLEEVES FOR ALL PIPE AND TUBING PASSING THROUGH FLOORS, ROOFS, AND WALLS. PACK CAULK INTO THE SPACE AROUND THE PIPE OR TUBING. PROVIDE FLASHING FOR ALL PIPES EXTENDING THROUGH THE ROOF.	
12. ALL VENT TERMINATIONS AT ROOF SHALL BE AT LEAST 10 FEET AWAY FROM OUTSIDE AIR INTAKES, OPERABLE WINDOWS, AND BUILDING OPENINGS.	
13. FILL CRACKS BETWEEN FIXTURES AND WALL/FLOORS WITH SILICONE RUBBER SEALANT.	
14. LOCATE, SIZE, AND INSTALL WATER HAMMER ARRESTERS IN ACCORDANCE WITH PLUMBING AND DRAINAGE INSTITUTE STANDARD NO. WH-201.	
15. INSTALL FIXTURES IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND ALL APPLICABLE CODES. SECURE FLOOR OUTLET OF FLOOR-MOUNTED FIXTURES TO DRAINAGE CONNECTIONS AND FLOOR IN A RIGID MANNER. RIGIDLY SUPPORT WALL-HUNG FIXTURES BY MEANS OF METAL SUPPORTING MEMBERS. USE CHROMIUM-PLATED BRASS BOLTS, NUTS, AND WASHERS WHERE EXPOSED. ALL CONNECTIONS SHALL BE MADE GAS-TIGHT AND WATER-TIGHT. USE OF PUTTY AND PLASTICS FOR GASKETS WILL NOT BE PERMITTED.	
16. PROVIDE ALL FIXTURE COMPONENTS AS INDICATED ON DRAWINGS. PROVIDE ADDITIONAL COMPONENTS AS PER MANUFACTURER'S RECOMMENDATIONS FOR PROPER OPERATION OF THE FIXTURES.	
17. PROVIDE EACH PLUMBING FIXTURE (INCLUDING HOSE BIBBS) WITH AN INDIVIDUAL STOP OR COMPRESSION VALVE OF POLISHED CHROME-PLATED LOOSE KEY TYPE.	
18. WHERE DEPTHS OR INVERTS ELEVATIONS ARE NOT INDICATED, PROVIDE MINIMUM COVERAGE (ABOVE TOP OF PIPES) AS FOLLOWS: A. ANY PIPING UNDER SLAB (TOP OF PIPE TO UNDERSIDE OF SLAB): 18 INCHES. B. CAST IRON AND COPPER PIPES IN OTHER LOCATIONS: 18 INCHES. C. EXCAVATE TO UNDISTURBED EARTH: CUT LEVEL AND FORM TRUE. REMOVE DEBRIS, RUBBISH AND SOFT MATERIAL (SUCH AS MUD). WHERE ROCK IS ENCOUNTERED, UNDERCUT TRENCHES 6-INCHES AND FILL WITH WELL TAMPED NEUTRAL SAND AND PEA GRAVEL TO PROPER PIPE ELEVATION. DURING EXCAVATION FREE OF STANDING WATER. UNDERCUT TRENCH 6-INCHES AND INSTALL PIPING IN A 6-INCH NEUTRAL SAND ENVELOPE.	
19. BACKFILL TO A POINT 12-INCHES ABOVE TOP OF PIPING WITH EARTH (EXCAVATED MATERIAL MAY BE USED) FREE OF CLAY, DEBRIS, RUBBISH, ROCKS, OR CLOUDS OVER 4-INCHES IN THE GREATEST DIMENSION. BACKFILL ABOVE 12-INCHES FROM TOP OF PIPING MAY BE WITH EXCAVATED MATERIAL. APPLY BACKFILL BY HAND IN 6-INCH DEEP LAYERS THE FULL WIDTH OF THE TRENCH. MOISTEN EACH LAYER (DO NOT FLOOD OR PUDDLE), AND HAND TAMP TO A MINIMUM 90 PERCENT COMPACTION BEFORE PROCEEDING WITH THE NEXT LAYER OF BACKFILL.	
20. DO NOT EXCAVATE UNDER FOUNDATIONS OR FOOTINGS EXCEPT IN MANNER PERMITTED BY THE ARCHITECT. DO NOT BACKFILL UNTIL INSTALLED PIPING HAS BEEN SUCCESSFULLY TESTED.	
21. VERIFICATION OF WATER AGENCY APPROVAL SHALL BE SUBMITTED TO THE BUILDING AND SAFETY DIVISION PRIOR TO ISSUANCE OF A PLUMBING PERMIT FOR THIS PROJECT.	
22. ALL PENETRATIONS THRU FIRE RATED ASSEMBLIES SHALL BE PACKED WITH APPROVED FIRE PROOFING. FOR LOCATIONS OF FIRE RATED ASSEMBLIES, SEE ARCHITECTURAL PLANS.	
23. ROUTE ALL PIPES AS HIGH AS POSSIBLE IN EXPOSED LOCATIONS. COORDINATE ROUTING WITH ALL OTHER TRADES PRIOR TO START OF WORK.	
24. NO SPRAY FOAM INSULATION SHALL BE APPLIED TO AREAS CONTAINING PEX PIPING.	

APPLICABLE CODE: 2022 CBC

PIPING, DUCTWORK, AND ELECTRICAL DISTRIBUTION SYSTEM BRACING NOTE  
PIPING, DUCTWORK, AND ELECTRICAL DISTRIBUTION SYSTEM SHALL BE BRACED TO COMPLY WITH THE FORCES AND DISPLACEMENTS PRESCRIBED IN ASCE 7-16 SECTION 13.3 AS DEFINED IN ASCE 7-16 SECTIONS 13.6.5, 13.6.6, 13.6.7, 13.6.8, AND 2022 CBC, SECTIONS 1617A.1.24, 1617A.25 AND 1617A.1.26.




THE METHOD OF SHOWING BRACING AND ATTACHMENTS TO THE STRUCTURE FOR THE IDENTIFIED DISTRIBUTION SYSTEM ARE AS NOTED BELOW. WHEN BRACING AND ATTACHMENTS ARE BASED ON A PREAPPROVED INSTALLATION GUIDE (e.g., HCAI OPM FOR 2013 CBC OR LATER), COPIES OF THE BRACING SYSTEM INSTALLATION GUIDE OR MANUAL SHALL BE AVAILABLE ON THE JOBSITE PRIOR TO THE START OF AND DURING THE HANGING AND BRACING OF THE DISTRIBUTION SYSTEMS. THE STRUCTURAL ENGINEER OF RECORD SHALL VERIFY THE ADEQUACY OF THE STRUCTURE TO SUPPORT THE HANGER AND BRACE LOAD.

MECHANICAL PIPING (MP), MECHANICAL DUCTS (MD), PLUMBING PIPING (PP), ELECTRICAL DISTRIBUTION SYSTEMS (E):

MP □ MD □ PP □ E □ OPTION 1: DETAILED ON THE APPROVED DRAWINGS WITH PROJECT SPECIFIC NOTES AND DETAILS.

MP □ MD □ PP □ E □ OPTION 2: SHALL COMPLY WITH HCAI (OSHPD) PREAPPROVAL (OPM#) AS INCLUDED IN THESE DRAWINGS WITH PROJECT-SPECIFIC NOTES AND DETAILS.

PLUMBING PIPE MATERIAL SCHEDULE				
SERVICE	LOCATION	PIPE MATERIAL	SLOPE	
WATER	ABOVE GRADE	ASTM B88 TYPE "L" HARD DRAWN COPPER WITH WROUGHT COPPER FITTINGS.	1/32" PER 1'	
	BELOW GRADE	ASTM B88 TYPE "K" HARD DRAWN COPPER, FACTORY INSULATED, WITH WROUGHT COPPER FITTINGS.	1/32" PER 1'	
SEWER AND VENT	ABOVE GRADE	ASTM A888 SERVICE WEIGHT CAST IRON PIPE AND DWV FITTINGS SHALL CONFORM TO CPC AND BEAR THE COLLECTIVE TRADEMARK OF CISPI AND NSF.	1/4" PER 1'	
	BELOW GRADE	ABS SCHEDULE 40 PIPE AND DWV FITTINGS SHALL CONFORM TO ASTM D2321-2000 AND CPC.	1/4" PER 1'	

PLUMBING FIXTURE SCHEDULE						
SYMBOL	FIXTURE	MIN. PIPE SIZE				REMARKS
		CW	HW	V	S	
	DRINKING FOUNTAIN W/ BOTTLE FILLER	3/4"	--	--	2"	FREE STANDING GROUND MOUNTED OUTDOOR DRINKING FOUNTAIN HAWS MODEL 3612, VANDAL RESISTANT, ADA COMPLIANT PEDESTAL MOUNTED BOTTLE FILLER WITH 1 GPM FLOW AND HIGH-LOW DRINKING FOUNTAIN WITH HEAVY DUTY STAINLESS STEEL PEDESTAL WITH PUSH BUTTON OPERATED STAINLESS STEEL VALVE AND FLOW CONTROL. INSTALL WITH HOSE BIBB MODEL 3660, LOCKABLE HOSE BIBB.
	DRINKING FOUNTAIN W/ BOTTLE FILLER	3/4"	--	1-1/2"	2"	WALL MOUNTED EXTERIOR/INTERIOR DRINKING FOUNTAIN HAWS MODEL 1119-1920, VANDAL RESISTANT, ADA COMPLIANT WITH BOTTLE FILLER OF 1 GPM FLOW AND HIGH-LOW DRINKING FOUNTAINS WITH PUSH BUTTON OPERATED. INSTALL WITH WALL MOUNTING PLATE.
	BOTTLE FILLER	3/4"	--	1-1/2"	2"	WALL MOUNTED BOTTLE FILLER INDOOR/OUTDOOR HAWS MODEL 1922, ADA COMPLIANT, 1 GPM FLOW, PUSH BUTTON OPERATED, WITH DRIP TRAY DRAIN, MODEL 6468. INSTALL WITH WALL MOUNTING PLATE.

NOTES	
1. ALL PLUMBING SYSTEM COMPONENTS SHALL MEET OR EXCEED THE REQUIREMENTS OF CURRENT CBC, CMC, CPC, CEC, NFPA, ASTM, ANSI, AND ALL LOCAL AND STATE CODE REQUIREMENTS. (SEE BELOW)	
2. ALL PLUMBING EQUIPMENT LISTED IN OF THE 2022 CALIFORNIA CODE OF REGULATIONS (CCR), TITLE-24, PART 6, SECTION 110.3 ENERGY EFFICIENCY STANDARDS MUST BE CERTIFIED BY THE MANUFACTURER TO MEET OR EXCEED SPECIFICATIONS OR EFFICIENCIES ADOPTED BY THE CEC.	
3. ALL INSULATING MATERIALS INSTALLED MUST BE CERTIFIED BY CALIFORNIA ENERGY COMMISSION TO MEET 2022 CALIFORNIA CODE OF REGULATIONS, TITLE-24, PART 6, ENERGY EFFICIENCY STANDARDS, SECTION 120.3 AND TABLE 4-15.	
4. ALL INSULATION INSTALLED SHALL MEET THE FLAME SPREAD AND SMOKE DENSITY REQUIREMENTS OF 2022 CBC, PART 1, SECTION 720 AND 2022 CMC, SECTION 602.2.	
5. ALL PIPING EXPOSED TO WEATHER SHALL BE METALLIC.	
6. ALL FERROUS PIPING EXPOSED TO WEATHER SHALL BE GALVANIZED AND PAINTED.	
7. ALL PIPES, FITTINGS AND FIXTURES USED TO CONVEY POTABLE WATER SHALL BE LEAD FREE IN COMPLIANCE WITH CPC SECTION 604.2.	
8. ALL FIXTURES REQUIRED TO BE ACCESSIBLE SHALL BE INSTALLED AS PER THE LATEST REQUIREMENTS OF TITLE 24 AND ADA (AMERICANS WITH DISABILITIES ACT).	
9. CROSS CONNECTION PROTECTION SHALL BE PROVIDED AT ALL POTABLE WATER SUPPLIED APPLIANCES AND EQUIPMENT (OTHER THAN THOSE LISTED IN INFORMATION BULLETIN 103).	
10. ALL INSTALLATION OF PEX PIPE INSTALLED IN NEW CONSTRUCTION SHALL BE FLUSHED TWICE OVER A PERIOD OF AT LEAST ONE WEEK PER CPC SECTION 604.1.2. PEX.	
1) AT THE TIME OF FILL, EACH NEW PLUMBING FIXTURE SHALL HAVE A REMOVABLE TAG APPLIED STATING: o. THIS NEW PLUMBING SYSTEM SHALL BE FIRST FILLED AND FLUSHED ON (DATE) BY (NAME). THE STATE OF CALIFORNIA REQUIRES THAT THE SYSTEM BE FLUSHED AFTER STANDING AT LEAST ONE WEEK AFTER THE FILL DATE SPECIFIED ABOVE. IF THIS SYSTEM IS USED EARLIER THAN ONE WEEK AFTER THE FILL DATE ABOVE, THIS SYSTEM IS USED EARLIER THAN ONE WEEK AFTER THE FILL DATE, THE WATER MUST BE ALLOWED TO RUN FOR AT LEAST TWO MINUTES PRIOR TO USE FOR HUMAN CONSUMPTION. THE TAG MAY NOT BE REMOVED PRIOR TO THE COMPLETION OF THE REQUIRED SECOND FLUSHING, EXCEPT BY BUILDING OWNER OR OCCUPANT.	
2) PRIOR TO ISSUING A BUILDING PERMIT TO INSTALL PEX PIPE, THE BUILDING OFFICIAL SHALL REQUIRE AS PART OF THE PERMITTING PROCESS THAT THE CONTRACTOR, OR THE APPROPRIATE PLUMBING SUBCONTRACTORS, PROVIDE WRITTEN CERTIFICATION THAT HE OR SHE WILL COMPLY WITH THE FLUSHING PROCEDURES SET FORTH BY CODE.	
3) THE BUILDING OFFICIAL SHALL NOT GIVE FINAL PERMIT APPROVAL FOR ANY PEX PLUMBING INSTALLATION UNLESS HE OR SHE FINDS THAT THE MATERIAL HAS BEEN INSTALLED IN COMPLIANCE WITH THE REQUIREMENTS OF THE CODE, INCLUDING THE REQUIREMENTS TO FLUSH AND TAG THE SYSTEMS.	
4) ANY CONTRACTOR OR SUBCONTRACTOR FOUND TO HAVE FAILED TO COMPLY WITH THE PEX FLUSHING REQUIREMENTS SHALL BE SUBJECT TO THE PENALTIES IN HEALTH AND SAFETY CODE, DIVISION 13, PART 1.5, CHAPTER 6 (SECTION 17995, et seq.).	

APPLICABLE CODES	
• 2022 CALIFORNIA ADMINISTRATIVE CODE (CAC), CCR PART 1, TITLE 24	
• 2022 CALIFORNIA BUILDING CODE (CBC), CCR TITLE 24, PARTS 1 & 2 (BASED ON THE 2021 EDITION INTERNATIONAL BUILDING CODE, VOLS. 1 & 2)	
• 2022 CALIFORNIA ELECTRICAL CODE (CEC), CCR TITLE 24, PART 3 (BASED ON THE 2020 EDITION NATIONAL ELECTRICAL CODE WITH CALIFORNIA AMENDMENTS)	
• 2022 CALIFORNIA MECHANICAL CODE (CMC), CCR TITLE 24, PART 4, TITLE 24 CCR (BASED ON THE 2021 EDITION UNIFORM MECHANICAL CODE WITH CALIFORNIA AMENDMENTS)	
• 2022 CALIFORNIA PLUMBING CODE (CPC), CCR TITLE 24, PART 5, (BASED ON THE 2021 EDITION UNIFORM PLUMBING CODE WITH CALIFORNIA AMENDMENTS)	
• 2022 CALIFORNIA ENERGY CODE (CEC), CCR TITLE 24, PART 6, AND ASSOCIATED ADMINISTRATIVE REGULATION IN PART 1.	
• 2022 CALIFORNIA FIRE CODE (CFC), CCR TITLE 24, PART 9 (BASED ON THE 2021 EDITION INTERNATIONAL FIRE CODE WITH CALIFORNIA AMENDMENTS)	
• 2022 CALIFORNIA EXISTING BUILDING CODE (CEBC), CCR TITLE 24, PART 10, (BASED ON THE 2021 EDITION INTERNATIONAL EXISTING BUILDING CODE WITH CALIFORNIA AMENDMENTS)	
• 2022 CALIFORNIA GREEN BUILDING STANDARDS CODE (CALGreen), CCR TITLE 24, PART 11	
• 2022 CALIFORNIA REFERENCED STANDARDS CODE, CCR TITLE 24, PART 12	
• TITLE 19 CCR, PUBLIC SAFETY, STATE FIRE MARSHAL REGULATIONS	

APPLICABLE CODE: 2022 CBC

MEP COMPONENT ANCHORAGE NOTE:  
ALL MECHANICAL, PLUMBING, AND ELECTRICAL COMPONENTS SHALL BE ANCHORED AND INSTALLED PER THE DETAIL ON THE DSA-APPROVED CONSTRUCTION DOCUMENTS. THE FOLLOWING COMPONENTS SHALL BE ANCHORED OR BRACED TO MEET THE FORCE AND DISPLACEMENT REQUIREMENTS PRESCRIBED IN THE 2022 CBC SECTIONS 1617A.1.18 THROUGH 1617A.1.26 AND ASCE 7-16 CHAPTERS 13.26, AND 30:

1. ALL PERMANENT EQUIPMENT AND COMPONENTS.
2. TEMPORARY, MOVABLE OR MOBILE EQUIPMENT THAT IS PERMANENTLY ATTACHED (e.g., HARD WIRED) TO THE BUILDING UTILITY SERVICE SUCH AS ELECTRICITY, GAS OR WATER. "PERMANENTLY ATTACHED" SHALL INCLUDE ALL ELECTRICAL CONNECTIONS EXCEPT PLUGS FOR 110/220 VOLT RECEPTACLES HAVING A FLEXIBLE CABLE.
3. TEMPORARY MOVABLE OR MOBILE EQUIPMENT WHICH IS HEAVIER THAN 400 POUNDS OR HAS CENTER OF MASS LOCATED 4 FEET OR MORE ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORT THE COMPONENT IS REQUIRED TO BE RESTRAINED IN A MANNER APPROVED BY DSA.

THE FOLLOWING MECHANICAL AND ELECTRICAL COMPONENTS SHALL BE POSITIVELY ATTACHED TO THE STRUCTURE BUT NEED NOT DEMONSTRATE COMPLIANCE WITH THE REFERENCES NOTED ABOVE. THESE COMPONENTS SHALL HAVE FLEXIBLE CONNECTIONS PROVIDED BETWEEN THE COMPONENT AND ASSOCIATED DUCTWORK, PIPING, AND CONDUIT. FLEXIBLE CONNECTIONS MUST ALLOW MOVEMENT IN BOTH TRANSVERSE AND LONGITUDINAL DIRECTIONS:

- A. COMPONENTS WEIGHING LESS THAN 400 POUNDS AND HAVING A CENTER OF MASS LOCATED 4 FEET OR LESS ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORT THE COMPONENT.
- B. COMPONENTS WEIGHING LESS THAN 20 POUNDS, OR IN THE CASE OF DISTRIBUTED SYSTEMS, LESS THAN 5 POUNDS PER FOOT, WHICH ARE SUSPENDED FROM A ROOF OR FLOOR OR HUNG FROM WALL.

THE ANCHORAGE OF ALL MECHANICAL, ELECTRICAL AND PLUMBING COMPONENTS SHALL BE SUBJECT TO THE APPROVAL OF THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE OR STRUCTURAL ENGINEER DELEGATED RESPONSIBILITY AND ACCEPTANCE BY DSA. THE PROJECT INSPECTOR WILL VERIFY THAT ALL COMPONENTS AND EQUIPMENT HAVE BEEN ANCHORED IN ACCORDANCE WITH THE ABOVE REQUIREMENTS.

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122273 INC.  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☒  
DATE: 05/03/2024

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ARCHITECT	ENGINEER

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NO.	REMARKS	DATE

GENERAL NOTES

REVISION HISTORY

DRAWING STATUS

KEY PLAN

PROJECT STATUS

WASHINGTON UNIFIED SCHOOL DISTRICT  
930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

WUSD RIVERBANK ES  
ESSR III  
1100 CARRIE STREET  
WEST SACRAMENTO, CA 95605

PLUMBING LEGEND AND GENERAL NOTES

Date  
MM/DD/YYYY

Application Number  
XX-XXXXXX

Drawn  
PP

Checked  
SO

Project Number  
22042

Drawing Number  
XX-XXXXXX

P0.1





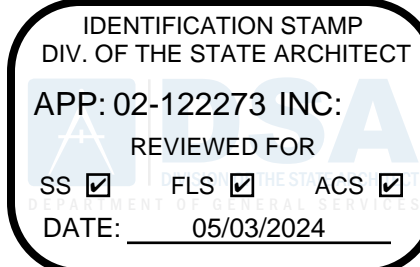
## GENERAL NOTES

- A. CONTRACTOR SHALL FIELD VERIFY EXACT LOCATION OF ALL PIPING AND UTILITIES PRIOR TO START OF WORK. IN THE EVENT OF ANY DISCREPANCIES OR POTENTIAL CONFLICTS, NOTIFY THE ARCHITECT AND ENGINEER IN WRITING PRIOR TO START OF WORK.
- B. ALL PIPING LOCATIONS ARE DIAGRAMMATIC. CONTRACTOR SHALL COORDINATE WITH ALL TRADES AND OWNER'S REPRESENTATIVE AND VERIFY EXACT ROUTING PRIOR TO START OF WORK.
- C. VERIFY EXACT SIZE AND LOCATION OF ALL PLUMBING CONNECTIONS TO MECHANICAL EQUIPMENT PRIOR TO START OF WORK. IN NO CASE SHALL THE CONNECTION SIZE BE LARGER THAN THE BRANCH PIPING SIZE.

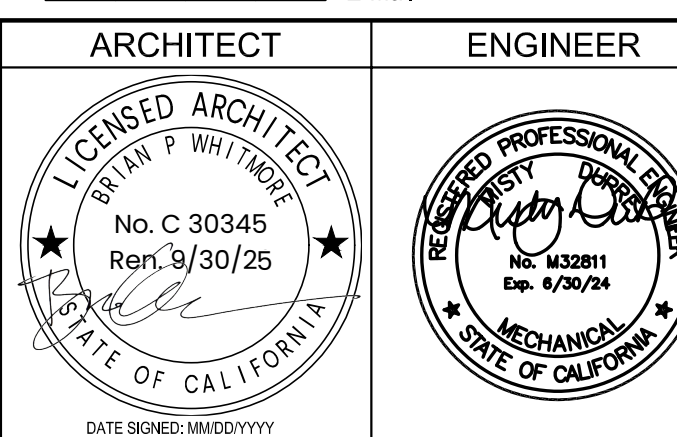
## KEY NOTES

1. INSTALL NEW DRINKING FOUNTAIN WITH BOTTLE FILLER. CONNECT TO PLUMBING SERVICE OF THE REMOVED FIXTURE.
2. DISCHARGE 2" WASTE FROM DRINKING FOUNTAIN TO DRYWELL. REFER TO CIVIL PLANS FOR DRYWELL DETAILS.
3. DRYWELL PER CIVIL PLAN.
4. CONNECT 3/4" CW TO 1" CW BELOW GRADE PER CIVIL PLANS. REFER TO CIVIL PLANS FOR CONTINUATION.
5. CONNECT 3/4" CW TO NEAREST (E) 1" CW.
6. CONNECT 2" WASTE TO NEAREST (E) 3" SEWER.
7. 1-1/2" VENT UP IN WALL. CONNECT TO NEAREST (E) 2" VENT ABOVE CEILING.

DSA STAMP



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NO.	REMARKS	DATE

DRAWING STATUS	DATE
<input type="radio"/> DSA PLAN CHECK	
<input type="radio"/> DSA BACK CHECK	
<input type="radio"/> BIDDING	
<input type="radio"/> CONSTRUCTION	

## KEY PLAN

WASHINGTON UNIFIED  
SCHOOL DISTRICT  
930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

## PROJECT STATUS

WUSD RIVERBANK ES  
ESSR III  
1100 CARRIE STREET  
WEST SACRAMENTO, CA 95605

## PLUMBING SITE PLAN

Date MM/DD/YYYY	Project Number 22042
Application Number XX-XXXXXX	Drawing Number P1.1
Drawn PP	Checked SO

PLUMBING SITE PLAN

1/30" = 1'-0"

01



DESIGN CRITERIA	
DESCRIPTION	DESIGN VALUES
BASE LOCATION LOCATED AT BOTTOM OF BASE PLATE TOP OF FOOTING	
DEAD AND LIVE LOADS	
ROOF LIVE LOAD	20 PSF
ROOF DEAD LOAD (SUPERIMPOSED ON FRAME)	5 PSF MAX
ROOF PANEL DEAD LOAD	M=1.1 PSF, G = 1.2 PSF, S = 1.3 PSF
COLLATERAL DEAD LOAD	M = 3.9 PSF, G = 3.8 PSF, S = 3.7 PSF
ROOF LIVE LOAD, $L_r$	
	20 PSF
ROOF SNOW LOAD	
GROUND SNOW LOAD, $P_g$	20 PSF
RISK CATEGORY	II
ROOF SNOW LOAD: SLOPED, $P_s$	20 PSF
FOR SNOW LOAD CONDITIONS ONLY - SITE APPLICATION REVIEWER SHALL VERIFY THE STRUCTURE SHALL BE LOCATED AT LEAST 20 FEET FROM ANY ADJACENT STRUCTURE FOR SNOW DRIFT.	
SNOW LOAD SLOPE FACTOR, $C_e$	1.0
SNOW LOAD EXPOSURE FACTOR, $C_e$	1.0
SNOW LOAD IMPORTANCE FACTOR, $I_s$	1.0
THERMAL FACTOR, $C_t$	1.2
LOWEST ANTICIPATED SERVICE TEMPERATURE	30°
WIND DESIGN	
BASIC WIND SPEED (3 SECOND GUST), $V_{ult}$ , $V_{avg}$	100 MPH, 76 MPH
RISK CATEGORY	II
EXPOSURE CATEGORY	C
FACTORS: $K_d$ , $K_z$ , $K_e$	0.85, 1.0, 0.85
$q_h = 0.00256 K_d K_z K_e V^2$	18.50 PSF
$C_{dw}$ PER ASCE FIGURE 27.3-5 ROOF ANGLE 18.43° - CLEAR / OBSTRUCTED	CASE A (1.1 / -1.2) CASE B (0.01 / -0.69)
$C_{de}$ PER ASCE FIGURE 27.3-5 ROOF ANGLE 18.43° - CLEAR / OBSTRUCTED	CASE A (-0.17 / -1.09) CASE B (-0.96 / -1.65)
$C_{di}$ PER ASCE FIGURE 27.3-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED (< h)	CASE A (-0.8 / -1.2) CASE B (0.8 / 0.5)
$C_{di}$ PER ASCE FIGURE 27.3-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED (> h, < 2h)	CASE A (-0.6 / -0.9) CASE B (0.5 / 0.5)
$C_{di}$ PER ASCE FIGURE 27.3-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED (> 2h)	CASE A (-0.3 / -0.6) CASE B (0.3 / 0.3)
COMPONENTS & CLADDING - $C_{cl}$ (PRESSURE/SUCTION) CLEAR / OBSTRUCTED	ZONE 3 - (2.29 / -2.11) / (1.0 / -3.0) ZONE 2 - (1.77 / -1.63) / (0.8 / -2.3) ZONE 1 - (1.15 / -1.05) / (0.5 / -1.5)
SEISMIC DESIGN	
LATERAL FORCE RESISTING SYSTEM	STEEL - ORDINARY CANTILEVER COLUMN
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE
SEISMIC IMPORTANCE FACTOR, $I_e$	1.0
SEISMIC SITE CLASS	D
$W_{CE}$ , SPECTRAL RESPONSE ACCELERATION @ 0.2 s, $S_s$	2.60
$W_{CE}$ , SPECTRAL RESPONSE ACCELERATION @ 0.2 s, $S_1$	0.90
SHORT PERIOD SITE COEFFICIENT, $F_a$	1.20
LONG PERIOD COEFFICIENT, $F_v$	1.70
FUNDAMENTAL PERIOD OF THE STRUCTURE, T (WORST CASE FOR ALL STRUCTURES)	0.152 s
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$	2.08 <input type="checkbox"/>
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED TO DETERMINE $C_s$ (WITH CAP PER ASCE 7 12.8.1.3) SOIL PROPERTIES MAY NOT BE CLASSIFIED AS SITE CLASS E	2.08 * 0.70 = 1.456 <input type="checkbox"/>
DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$	1.02
SEISMIC DESIGN CATEGORY	E
SITE SPECIFIC RESPONSE ANALYSIS NOT REQUIRED PER ASCE 7 11.4.8 EXCEPTION 2	$T_n = 0.49$ s <input type="checkbox"/> $T < 1.5 * T_n$
RESPONSE MODIFICATION FACTOR, R	1.25
OVERSTRENGTH FACTOR, $\Omega$	1.25
REDUNDANCY FACTOR, $\rho$	1.3
HORIZONTAL OR VERTICAL IRREGULARITIES	NONE
SEISMIC RESPONSE COEFFICIENT, $C_u$ (20° WIDE, 30° WIDE, 40° WIDE)	1.16 <input type="checkbox"/> 1.00 <input type="checkbox"/> 1.00 <input type="checkbox"/>
DESIGN BASE SHEAR, V (20° WIDE, 30° WIDE, 40° WIDE)	12.73 PSF <input type="checkbox"/> 13.41 PSF <input type="checkbox"/> 14.65 PSF <input type="checkbox"/>
ALLOWABLE SOIL BEARING FOR FOUNDATIONS	VARIABLES - SEE FOUNDATION CHARTS
FLOOD DESIGN - DESIGN IS ASSUMED TO NOT BE IN FLOOD HAZARD AREA	
IF PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED & SIGNED FROM A SOILS ENGINEER IS REQUIRED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED.	

STRUCTURAL SEPARATION		DEFLECTIONS ARE FOR (I) STRUCTURE		
ALL DEFLECTIONS SHOWN ALSO INCLUDE THE P-DELTA ROTATION PER IRC-7		SOIL CLASSES PER CBC TABLE 1806A.2		
MAXIMUM DRIFT $\delta_{h\max}$	SIDE COLUMNS	<u>Soil Class 5</u>	<u>Soil Class 4</u>	<u>Soil Class 3</u>
20° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT) (INCHES)		[ ] 2.40	[ ] 2.55	[ ] 2.65
30° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT) (INCHES)		[ ] 2.15	[ ] 2.30	[ ] 2.40
40° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT) (INCHES)		[ ] 2.20	[ ] 2.20	[ ] 2.30
MINIMUM SEPARATION ( $\delta_m = C_d \delta_{h\max}$ ) $C_d = 1.25$				
20° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT) (INCHES)		[ ] 3.00	[ ] 3.19	[ ] 3.31
30° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT) (INCHES)		[ ] 2.69	[ ] 2.88	[ ] 3.00
40° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT) (INCHES)		[ ] 2.75	[ ] 2.75	[ ] 2.88
MAXIMUM DRIFT $\delta_{h\max}$	END COLUMNS	<u>Soil Class 5</u>	<u>Soil Class 4</u>	<u>Soil Class 3</u>
20° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT) (INCHES)		[ ] 2.40	[ ] 2.55	[ ] 2.65
30° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT) (INCHES)		[ ] 2.15	[ ] 2.30	[ ] 2.40
40° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT) (INCHES)		[ ] 2.20	[ ] 2.20	[ ] 2.30
MINIMUM SEPARATION ( $\delta_m = C_d \delta_{h\max}$ ) $C_d = 1.25$				
20° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT) (INCHES)		[ ] 3.00	[ ] 3.19	[ ] 3.31
30° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT) (INCHES)		[ ] 2.69	[ ] 2.88	[ ] 3.00
40° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT) (INCHES)		[ ] 2.75	[ ] 2.75	[ ] 2.88

INSTRUCTIONS FOR ARCHITECTS SUBMITTING THESE PRE-CHECKED DRAWINGS TO DSA:

BEFORE SUBMITTING THESE PRE-CHECKED DRAWINGS FOR YOUR PROJECT, FOLLOW THE STEPS BELOW TO PROPERLY DEFINE THE APPROVED OPTIONS:

STEP 1: SELECT FRAME DIMENSIONS FOR YOUR PROJECT  
-HP STRUCTURES UP TO 20' WIDE USE THE "RH 20" BASE FRAME  
-HP STRUCTURES UP TO 30' WIDE USE THE "RH 30" BASE FRAME  
-HP STRUCTURES UP TO 40' WIDE USE THE "RH 40" BASE FRAME  
-MAXIMUM WIDTH IS 40' (SEE "ARCHITECTURAL VIEWS" SHEET FOR REFERENCE)  
-THE 24", 44", 64", 84" AND 104" LENGTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST COMMON (20" BAYS ARE THE MOST ECONOMIC)  
-FRAME LENGTHS ASSUME 2" OVERHANDS (UNO BY ARCHITECT - 2" MAX DIMENSION)

STEP 1	FRAME DIMENSIONS					
	SUGGESTED					OTHER
	FRAME WIDTH	[ ] 20'	[X] 30'	[ ] 40'		[ ] (40' MAX)
	FRAME LENGTH	[ ] 44'	[X] 64'	[ ] 84'	[ ] 104'	[ ] (NO MAX)

STEP 2: SELECT ROOF DECK FOR YOUR PROJECT  
- "M" REPRESENTS MCLEOD METAL "MULTI-RIB" ROOF PANEL  
- "G" REPRESENTS MCLEOD METAL "MEGA-RIB" ROOF PANEL  
- "S" REPRESENTS MCLEOD METAL "MEDALLION-LOCK" 16" STANDING SEAM ROOF PANEL

STEP 2	ROOF PANEL			
	ROOF PANEL TYPE	[ ] M	[ ] G	[X] S

STEP 3: IDENTIFY THE  $S_s$  ACCELERATION (g) FOR YOUR PROJECT  
-  $S_s$  VALUE DETERMINES THE REQUIRED SEISMIC DESIGN FORCES  
-  $S_s$  VALUE DEPENDS ON THE PROJECT'S GEOGRAPHICAL LOCATION (VALUES RANGE FROM 0.00 TO 3.73)  
- FIND  $S_s$  VALUES FOR YOUR PROJECT ON THE USGS WEBSITE (SEARCH INTERNET FOR "USGS SEISMIC DESIGN MAPS")

STEP 3	PROJECT SITE - $S_s$ ACCELERATION (g)
	0.59

STEP 4: IDENTIFY THE  $S_s$  REGION FOR YOUR PROJECT  
- THE REGIONS ARE DEPENDANT ON THE  $S_s$  VALUE DETERMINED IN STEP 3  
- THE  $S_s$  REGION DICTATES THE MAXIMUM DEAD LOAD PERMITTED ON THE FRAME

STEP 4	$S_s$ REGION		
	$S_s$ REGIONS		MAX DEAD LOAD
	DESCRIPTION		
	$0 < S_s \leq 2.14$		5 PSF
	$2.14 < S_s \leq 2.50$		9 PSF
	$2.50 < S_s \leq 2.60$		9 PSF

STEP 5: IDENTIFY THE ROOF DEAD LOAD FOR YOUR PROJECT  
- THE ROOF DEAD LOAD WILL ALWAYS BE INCLUDED  
- THE COLLATERAL LOAD REPRESENTS ADDITIONAL LOAD THAT CAN BE SUPPORTED BY THE FRAME  
- BE SURE THE TOTAL ROOF DEAD LOAD FOR YOUR PROJECT IS LESS THAN OR EQUAL TO THE MAX DEAD LOAD SHOWN IN STEP 4 FOR YOUR  $S_s$  VALUE  
-  $S_{ds}$  VALUE USED IN CALCULATION IS THE CAPPED  $S_{ds}$  (SEE DESIGN CRITERIA)

STEP 5	TOTAL ROOF DEAD LOAD		
	DEAD LOAD	EXAMPLES	
	ROOF DECK	1.3 PSF	M=1.1PSF, G=1.2PSF, S=1.3PSF (SEE STEP 2)
	COLLATERAL	0	LIGHTNING/FIRE SUPPRESSION/SOLAR PANELS, ETC.
	TOTAL	1.3 PSF	ADD ROOF DECK AND COLLATERAL LOADS (MAX 9 PSF)

STEP 6: IDENTIFY THE FOUNDATION REQUIREMENTS FOR YOUR PROJECT  
- IDENTIFY SOIL CLASS FOR PROJECT SITE PER SITE SPECIFIC SOIL CONDITIONS  
- USE THIS TO SELECT CORRECT FOUNDATION SIZE ON FOUNDATION SHEET

STEP 6	FOUNDATION REQUIREMENTS		
	[ ] GEOTECHNICAL REPORT NOT REQUIRED		[ ] GEOTECHNICAL REPORT REQUIRED
	SOIL CLASS 5 (BEARING) 1500 PSF [X]		
	SOIL CLASS 4 (BEARING) 2000 PSF [ ]		SOIL CLASS 3 (BEARING) 3000 PSF [ ]
	SOIL CLASS 5 (LATERAL BEARING) 200 PSF/FT		SOIL CLASS 5 (LATERAL BEARING) 300 PSF/FT
	COHESION 130 PSF		FRICTION COEFFICIENT 0.25
			FRICTION COEFFICIENT 0.30
	- SELECT & VERIFY MINIMUM SEPARATION DISTANCE BETWEEN STRUCTURES		

STEP 7: SELECT MISCELLANEOUS OPTIONS FOR YOUR PROJECT  
- MAXIMUM CLEAR HEIGHT IS 12'-0" (SEE "ARCHITECTURAL VIEWS" SHEET FOR REFERENCE)  
- MARK UP PC DRAWINGS WITH SIZE AND LOCATION OF CUTOUTS BEFORE SUBMITTING TO DSA

STEP 7	MISCELLANEOUS		
	DESIGN OPTIONS		
	CLEAR HEIGHT	[ ] 8' [X] 10' [ ] 12' MAX	
	ELECTRICAL CUTOUTS	[ ] YES	[X] NO
	GUTTERS	[X] YES	[ ] NO

STEP 8: SELECT APPLICABLE SHEET INDEX FOR YOUR PROJECT  
- REFERENCE THE BASE FRAME (STEP 1) AND THE ROOF PANEL TYPE (STEP 2)  
- IDENTIFY THE APPLICABLE SHEET INDEX

		SHEET INDEX											
		RH 20			RH 30			RH 40					
BASE FRAME		M	G	S	M	G	S	M	G	S			
ROOF PANEL TYPE		[ ]			[ ]			[ ]					
SELECT ONE		US1	US10	US11	US10	US10	US11	US10	US10	US11			
GENERAL NOTES		LS1.0	LS2.0	LS2.0	LS3.0	LS3.0	LS3.0	LS4.0	LS4.0	LS4.0			
FOUNDATION PLAN		LS2.1	LS2.1	LS2.1	LS3.1	LS3.1	LS3.1	LS4.1	LS4.1	LS4.1			
FRAMING PLAN		LS2.1	LS2.1	LS2.1	LS3.1	LS3.1	LS3.1	LS4.1	LS4.1	LS4.1			
FRAME CONNECTION DETAILS		LS2.1	LS2.1	LS2.1	LS3.1	LS3.1	LS3.1	LS4.1	LS4.1	LS4.1			
ROOFING LAYOUT & DETAILS		LS2.2	LS2.2	LS2.2	LS3.2	LS3.2	LS3.2	LS4.2	LS4.2	LS4.2			
DSA 103 EXAMPLE		US1	US12	US12	US12	US12	US12	US12	US12	US12			
MISC DESIGN OPTIONS		LS5.0	LS5.0	LS5.0	LS5.0	LS5.0	LS5.0	LS5.0	LS5.0	LS5.0			
STEP 8													

STEP 9: INCLUDE APPLICABLE SHEETS WITH YOUR DSA SUBMITTAL  
- INCLUDE "MISC DESIGN OPTIONS" SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUTS OR GUTTERS

STEP 10: IDENTIFY PROJECT NAME AND LOCATION	PROJECT NAME:	SCHOOL DISTRICT:
	RIVERBANK ELEMENTARY SCHOOL	WASHINGTON UNIFIED SCHOOL DISTRICT

STEP 11: CROSS OUT EXAMPLE 103 FORMS & INCORPORATE REQUIRED SPECIAL INSPECTIONS 103 FORMS THAT ARE PROJECT SPECIFIC

SITE SPECIFIC PARAMETERS  
INSTRUCTIONS: DESIGN PROFESSIONAL SHALL CHECK THE APPROPRIATE SELECTION BOXES BELOW AND ENTER THE DESIGN PARAMETERS APPLICABLE TO THE SPECIFIC PROJECT SITE

SNOW  
 $S_s = 0$  psf  
 $R = 0$  psf  
 $C_e = 0$  psf

WIND  
 $V = 95$  mph <  $XI$  mph  
 $I_{st} = 1.0$  ☐  
EXPOSURE: ☒ ☐ ☐

SEISMIC  
☒ DESIGN BASED ON SITE CLASS D  
NO GEOTECHNICAL INVESTIGATION REQUIRED  
 $S_s = 0.59$   $F_a = 1.2$

☐ DESIGN BASED ON SITE CLASS DETERMINED PER CHAPTER 20 OF ASCE 7-16  
GEOTECHNICAL INVESTIGATION PROVIDED

SITE CLASS ☐ ☐ ☐ ☐  
 $S_s =$   $F_a =$  PER ASCE 7-16 SUPPL 3, TABLE 11.4-1

☐ DESIGN BASED ON SITE SPECIFIC GROUND MOTION HAZARD ANALYSIS  
PER CHAPTER 21 OF ASCE 7-16  
SHORT-PERIOD DESIGN SPECTRAL RESPONSE PARAMETER,  $S_{ds}$ , SHALL BE AS SPECIFIED IN GEOTECHNICAL INVESTIGATION  
CGS APPROVAL REQUIRED  
NOT ELEGIBLE FOR OTC REVIEW

SITE CLASS ☐ ☐ ☐ ☐

$S_{ds} = F_a S_s = 0.59$  ( $S_{ds} = 2.08$  USED IN DESIGN, CONSERVATIVE)

☐ SITE CLASS D  $S_{ds} = 0.7 + S_s = 0.7 + 0.59 = 1.29$   $C_{sm} =$   $C_{sm}$

$C_{sm} = 1.00$  USED IN DESIGN

SEISMIC DESIGN CATEGORY ☒ ☐ ☐

\*SITE SPECIFIC  $S_{ds}$  VALUE BEFORE APPLYING REDUCTION  
ALLOWED BY ASCE 7 SECTION 12.8.1.3

ABBREVIATIONS:			
ACI	AMERICAN CONCRETE INSTITUTE	MPH	MILES PER HOUR
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	M	MULTI-RIB ROOF PANEL (MCLEROY)
ASM	ASSEMBLY (INTERNAL REFERENCE)	NTS	NOT TO SCALE
ASTM	AMERICAN SOCIETY FOR TESTING AND MAT'L S	NO	NUMBER
AWS	AMERICAN WELDING SOCIETY	OC	ON CENTER
CBC	CALIFORNIA BUILDING CODE	OSHA	OCCUPATIONAL HEALTH AND SAFETY ADMIN
CJP	COMPLETE JOINT PENETRATION	PCF	POUNDS PER CUBIC FOOT
CLR	CLEAR	PJ	PRETENSIONED JOINT
DEG	DEGREE	PLCS	PLACES
DIA	DIAMETER	PLT	PLATE
DM	DIMENSION	PSF	POUNDS PER SQUARE FOOT
DSA	DIVISION OF THE STATE ARCHITECT	PSI	POUNDS PER SQUARE INCH
EQ	EQUAL	QTY	QUANTITY
FT	FEET	REF	REFERENCE
GA	GAGE	SQ	SQUARE
IN	INCHES	SS	STANDING SEAM ROOF PANEL (MCLEROY)
KSI	KIPS PER SQUARE INCH	TYP	TYPICAL
MAX	MAXIMUM	UNO	UNLESS NOTED OTHERWISE
MIN	MINIMUM	USGS	U.S. GEOLOGICAL SURVEY
MISC	MISCELLANEOUS	W/	WITH

ARCHITECTURAL REQUIREMENTS		
DESCRIPTION		DESIGN VALUES
TYPE OF CONSTRUCTION		II-B
OCCUPANCY CLASSIFICATION		A-2
NUMBER OF STORIES		1
FIRE SPRINKLER SYSTEM		NOT BY ICON/WEIGHT NOT INCLUDED IN DESIGN
MOST COMMON RH20 MIN/MAX SQ.FT (SEE STEP 1)		480/2,080
MOST COMMON RH30 MIN/MAX SQ.FT (SEE STEP 1)		720/3,120
MOST COMMON RH40 MIN/MAX SQ.FT (SEE STEP 1)		960/4,160
AREA OVER 4000 SQ.FT REQUIRES GEOHAZARD REPORT ALLOWABLE ARE FOR II-B / A-3 IS 9500 SQ.FT		

RELATED BUILDING CODES AND STANDARDS  
TITLE 24 CODES:

2022 CALIFORNIA ADMINISTRATIVE CODE (CAC).....(PART 1, TITLE 24, CCR)  
2022 CALIFORNIA BUILDING CODE (CBC),PART 2, TITLE 24 CCR  
2022 CALIFORNIA ELECTRICAL CODE.....(PART 3, TITLE 24, CCR)  
2022 CALIFORNIA MECHANICAL CODE (CMC).....(PART 4, TITLE 24, CCR)  
2022 CALIFORNIA PLUMBING CODE (CPC).....(PART 5, TITLE 24, CCR)  
2022 CALIFORNIA ENERGY CODE.....(PART 6, TITLE 24, CCR)  
2022 CALIFORNIA FIRE CODE (CFC).....(PART 9, TITLE 24, CCR)  
2022 CALIFORNIA GREEN BUILDING STANDARDS CODE.....(PART 11, TITLE 24, CCR)  
2022 CALIFORNIA REFERENCE STANDARDS CODE.....(PART 12, TITLE 24, CCR)  
TITLE 19 CCR, PUBLIC SAFETY, STATE FIRE MARSHAL REGULATIONS

REFERENCE CODE SECTIONS FOR APPLICABLE STANDARDS:  
2022 CBC, CHAPTER 35  
2022 CFC, CHAPTER 80

SCOPE OF WORK NARRATIVE  
THESE DRAWINGS ILLUSTRATE THE FABRICATION AND INSTALLATION REQUIREMENTS FOR A FREE-STANDING PREFABRICATED STEEL SHADE STRUCTURE. THE ENTIRE STRUCTURAL SYSTEM IS COMPRISED OF HOLLOW STRUCTURAL STEEL MEMBERS SUPPORTED BY CONCRETE FOUNDATIONS. THE FLEXIBILITY INCLUDED HEREIN ALLOWS THE STRUCTURE TO COMPLY WITH A WIDE VARIETY OF PROJECT SITES AND LOADING REQUIREMENTS.



GENERAL:

1. GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT.
2. WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE CBC, C.A.C. TITLE 24, AND ALL STATE AND FEDERAL REGULATIONS.
3. OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO PROCEEDING WITH ANY WORK INVOLVED.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS. ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
5. THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISIT TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE CONSTRUCTION.
6. ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS, EXCEPT AS AMENDED BY CBC CHAPTER 35.
7. CONFORM TO APPLICABLE CALIFORNIA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
8. THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED TO ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES.
9. SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE REQUIRED CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK.
10. THE SCHOOL DISTRICT INSPECTOR ON RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO ROOF INSTALLATION.
11. SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIREMENTS.
12. LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA FOR THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE.
13. VIEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS.

STRUCTURAL AND MISCELLANEOUS STEEL:

1. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) SPECIFICATION MANUAL REFERENCED BY THE LATEST EDITION OF THE CALIFORNIA BUILDING CODE.
2. PIPE SECTIONS SHALL CONFORM TO ASTM A53, Fy = 35 KSI, GRADE B OR A501 UNLESS NOTED OTHERWISE.
3. STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A-500, GRADE B (OR C), Fy = 46 KSI, MIN.
4. IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESS CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE DRAWINGS (MAXIMUM INCREASE OF 1/8").
5. ALL CHANNELS, ANGLES, AND MISC. STEEL SHALL CONFORM TO ASTM A-36, Fy = 36 KSI.
6. ALL PLATE STEEL SHALL CONFORM TO ASTM A-572, Fy= 50 KSI.
7. ALL COLD FORM STEEL SHALL CONFORM TO ASTM A-653, CS = TYPE B, Fy = 50 KSI Fu = 65 KSI
8. STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2202A.1.
9. ALL ROOF DECKS SHALL HAVE KYNAR 500 METAL COATING.
10. ALL ROOF DECKS SHALL CONFORM TO ASTM A-792, Fy = 50 KSI.
11. ALL BASE CONNECTIONS ARE A PART OF THE LATERAL FORCE RESISTING SYSTEM

NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

1. PER TITLE 24, PART 1, SECTION 4-316(a) OF THE CALIFORNIA CODE OF REGULATIONS, THIS NOTICE SHALL BE GIVEN TO DSA PRIOR TO THE APPROVAL OF PLANS AND SPECIFICATIONS.
2. FOR THE SITE SPECIFIC PROJECT, J. R. MILLER & ASSOCIATES IS NOT THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE.
3. FOR THE SITE SPECIFIC PROJECT, J.R. MILLER & ASSOCIATES' RESPONSIBILITY IS LIMITED TO THE PREPARATION OF THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC ONLY.
4. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM J.R. MILLER & ASSOCIATES' RESPONSIBILITY FOR THE SITE SPECIFIC PROJECT.
5. ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING SHALL BE DELEGATED TO A QUALIFIED ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE, BUT ARE NOT LIMITED TO, STRUCTURAL OBSERVATION OF CONSTRUCTION, REVIEW OF INSPECTION REPORTS, AND SIGNING OFF OF THE VERIFIED REPORT FOR COMPLETED WORK.
6. J.R. MILLER & ASSOCIATES WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC WHICH ARISE DURING PLAN REVIEW AND CONSTRUCTION.

CONSTRUCTION NOTES

1. A DSA-CERTIFIED CLASS 3 (MINIMUM) PROJECT INSPECTOR IS REQUIRED FOR THIS PROJECT.
2. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE DOCUMENT (CCD), APPROVED BY DSA, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR.
3. A "DSA CERTIFIED" PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY DSA SHALL PROVIDE CONTINUOUS INSPECTION OF WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.
4. A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED TESTS AND INSPECTIONS FOR THE PROJECT.
5. THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS ARE THAT ALL THE WORK OF THE ALTERATION, REHABILITATION OR RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24, CCR. SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHICH IS NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT (CCD), OR A SEPARATE SET OF PLANS AND SPECIFICATIONS, DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK. (SECTION 4-317(c), PART 1, TITLE 24, CCR)
6. GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS SHALL COMPLY WITH ALL LOCAL ORDINANCES

WELDING:

1. ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED AS REQUIRED BY DSA.
2. ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 FT-LB-IP ( 0' F).
3. ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO ENSURE PROPER MATERIAL ID AND WELDING.
4. WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND SPECIFICATIONS.

BOLTING:

1. ALL BOLTS SHOWN ON THESE DRAWINGS ARE HOT DIPPED GALVANIZED ASTM F3125 GRADE A325 HIGH STRENGTH BOLTS (UNF), WITH THE NUTS CONFORMING TO HOT DIPPED GALVANIZED ASTM A-563 GRADE DH.
2. HIGH STRENGTH BOLTS SHALL BE VERIFIED AND INSPECTED PER CBC 1705A2.1.
3. BEFORE ERECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS – INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHASING SOME OF THE BOLTS AND NUTS MAY BE REQUIRED.
4. HARDENED STEEL WASHERS SHALL CONFORM TO ASTM F-436.
5. THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME.  
BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S  
USING HIGH-STRENGTH BOLTS; CBC 1705A2.1; AISC 341-16 (F); AISC 360-16 NS6.  
A) PRETENSIONED JOINTS MUST BE INSTALLED AND INSPECTED TO MEET ONE OF THE FOLLOWING REQUIREMENTS:
  1. TURN-OF-NUT PRETENSIONING: PER SECTION 8.2.1 OF THE SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS, WASHERS ARE NOT REQUIRED FOR THIS METHOD. THE NUT OR HEAD SHALL BE ROTATED AS SPECIFIED IN TABLE 8.2. THE PART NOT TURNED SHALL BE PREVENTED FROM ROTATING.
  2. CALIBRATED WRENCH: PER THE SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS, WASHERS ARE REQUIRED (NOT SUPPLIED BY KCM) THESE SHALL BE INSTALLED PER THE INSTALLATION TORQUE DETERMINED IN THE PRE-INSTALLATION VERIFICATION OF THE FASTENER ASSEMBLY PER SECTION 7. THE PART NOT TURNED SHALL BE PREVENTED FROM ROTATING.
  3. IDENTIFIED ON THE FRAME CONNECTION DETAILS WITH "PT REQUIRED"
- B) ALL OTHER JOINTS MUST BE INSTALLED AND INSPECTED TO MEET THE REQUIREMENTS OF THE SHUCK-TIGHTENED JOINTS. SHUCK TIGHT CONDITION EXISTS WHEN ALL PILES IN A CONNECTION HAVE BEEN PULLED INTO FIRM CONTACT BY THE BOLTS IN THE JOINT AND ALL OF THE BOLTS IN THE JOINT HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT REMOVAL OF THE NUTS WITHOUT THE USE OF A WRENCH.

FOUNDATIONS:

1. ALLOWABLE SOIL PRESSURES ASSUME CLASS 5 SOIL CLASSIFICATION PER CBC TABLE 1806A, UNLESS NOTED OTHERWISE. PASSIVE PRESSURE IS ASSUMED TO START 12" BELOW TOP OF FOOTING.
2. PER CBC SECTION 1803A.2, GEOTECHNICAL REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONES OR SEISMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS; ALLOWABLE FOUNDATION AND LATERAL SOIL PRESSURE VALUES MAY BE DETERMINED FROM TABLE 1806A.2.
3. FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD D-1557 OR AS RECOMMENDED BY THE GEO-TECH ENGINEER. FLOODING NOT PERMITTED.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING, ETC. NECESSARY TO SUPPORT CUT AND/OR FILL BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE.
5. MINIMUM SETBACK FROM TOE OF SLOPE ON AN ASCENDING SLOPE SHALL BE 15 FEET AND MINIMUM SETBACK FROM TOE OF SLOPE ON A DESCENDING SLOPE SHALL BE 40 FEET
6. PER CBC SECTION 1803A.6, GEOHAZARD REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONES OR SEISMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS.
7. GEOHAZARD REPORTS ARE TO COMPLY WITH DSA IR A-4 PER IR-7 SECTION 1.8
8. SITE SPECIFIC GEOTECHNICAL REPORT IS REQUIRED AT THE TIME OF SITE APPLICATION IF USING OTHER THAN CLASS 5 SOIL PER DSA IR PC-7.
9. LATERAL BEARING HAS BEEN INCREASED PER CBC 1806A.3.4 FOR THE 1/2" DEFLECTION & HAS BEEN DESIGNED FOR P-DELTA EFFECTS. NO 1/3 INCREASE HAS BEEN APPLIED.
10. MINIMUM CLEARANCE BETWEEN PIERS SHALL BE 8'-0".

CONCRETE:

1. MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE)

STRENGTH Pci (28 DAYS)	W/C RATIO (NON-AIR ENTRAINED)	W/C RATIO (AIR ENTRAINED)	SLUMP (in")	UNIT WEIGHT (NORMAL WEIGHT)
5000 PSI	0.44	0.35	3"	150 PCF

2. CONCRETE MIX DESIGN PARAMETERS ARE GOOD FOR EXPOSURE CATEGORIES F0, F1 & F2. THE AIR ENTRAINMENT FOR THESE CATEGORIES SHALL BE AS FOLLOWS: F0-0, F1-4.5, F2-6.
3. CHANGES TO THE MIX DESIGN MUST BE APPROVED BY THE ENGINEER OR ARCHITECT OF RECORD AND DSA.
4. AGGREGATES SHALL CONFORM TO THE ASTM C-33 WITH PROVEN SHRINKAGE CHARACTERISTICS OF LESS THAN 0.005. MAX AGGREGATE SIZE = 1".
5. CEMENT SHALL CONFORM TO ASTM C-150 (TYPE V) UNLESS NOTED OTHERWISE ON THE DRAWINGS.
6. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT. ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED.
7. CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
8. CONCRETE DURABILITY SHALL BE PER CBC 1904A.1, ACI 318-19, CHAPTER 19.
9. CONCRETE SHALL BE TESTED PER CBC 1903A, TABLE 1705A.3, AND ACI 318-19, SECTION 26.12.
10. NO ADMIXTURE SHALL CONTAIN CALCIUM CHLORIDE.

REINFORCING STEEL:

1. REINFORCING STEEL SHALL BE DEFORMED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A-615, AS FOLLOWS:
  - OR 60: (#4 BARS AND LARGER)
  - OR 40: (#3 BARS)
2. DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM TO THE ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES."
3. MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:
  - A. CAST AGAINST EARTH .....3"
  - B. CAST AGAINST FORM BELOW GRADE .....2"
  - C. FORMED SLABS (#11 BAR & SMALLER).....3/4"
  - D. SLABS ON GRADE (FROM TOP OF SLAB).....1"
4. BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND. BENDS SHALL BE MADE COLD.
5. REINFORCING SHALL BE LAP SPLICED PER ACI 318-19, SECTION 25.5.
6. PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN POSITION.
7. WELDING OF REINFORCING IS NOT ALLOWED.
8. REINFORCING STEEL SHALL BE INSPECTED PER CBC 1705A.3.

POWDER-COAT FINISH SYSTEM:

ALL BUILDINGS THAT HAVE A POWDER-COATED FINISH SHALL MEET THE FOLLOWING SPECIFICATIONS:

1. THE STEEL FRAME (HSS SECTIONS, COLD FORMED & PLATE STEEL) SHALL BE SHOT-BLASTED TO A NEAR WHITE CONDITION PER SSPC-10 SPECIFICATIONS.
2. THE STEEL SHALL BE WASHED IN A ZINC PHOSPHATE IN AN MINIMUM THREE STAGE ELECTRO DEPOSITION PRE-TREATMENT PROCESS.
3. IMMEDIATELY FOLLOWING PRE-TREATMENT THE STEEL SHALL BE TOTALLY COATED IN AN EPOXY PRIMER TO A UNIFORM THICKNESS OF A MINIMUM OF 0.7 TO 0.9 MILS. THE E-COATING SHALL PROVIDE A MINIMUM OF 1000 HOURS OF SALT SPRAY CORROSION PROTECTION TO THE STEEL.
4. THE STEEL SHALL THEN HAVE A IGIC POLYESTER COLOR COAT APPLIED OVER THE E-COATED SURFACE.
5. THE FINISH THICKNESS OF THESE APPLICATIONS SHALL BE A MINIMUM OF 8 TO 12 MILS.
6. ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES, & COLD FORMED STEEL ETC.) NOT POWDER-COATED SHALL BE PAINTED WITH PRIME COAT PER THE "AISC CODE OF STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3" UNLESS NOTED OTHERWISE).



DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS, 2022 CBC

Application Number: 04-122188  
School Name: PC Update  
School District: PC Update  
Date Created: 2023-04-19 08:36:32  
DSA File Number: Increment Number:

2022 CBC

IMPORTANT: This form is only a summary list of structural tests and some of the special inspections required for the project. Generally, the structural tests and special inspections noted on this form are those that will be performed by the Geotechnical Engineer of Record, Laboratory of Record, or Special Inspector. The actual complete test and inspection program must be performed as detailed on the DSA approved documents. The appendix at the bottom of this form identifies work NOT subject to DSA requirements for special inspection or structural testing. The project inspector is responsible for providing inspection of all facets of construction, including but not limited to, special inspections not listed on this form such as structural wood framing, high-load wood diaphragms, cold-formed steel framing, anchorage of non-structural components, etc., per Title 24, Part 2, Chapter 17A (2022 CBC).

\*\*NOTE: Undefined section and table references found in this document are from the CBC, or California Building Code.

KEY TO COLUMNS

1. TYPE

Continuous – Indicates that a continuous special inspection is required

Periodic – Indicates that a periodic special inspection is required

Test – Indicates that a test is required

2. PERFORMED BY

GE (Geotechnical Engineer) – Indicates that the special inspection shall be performed by a registered geotechnical engineer or his or her authorized representative.

LOR (Laboratory of Record) – Indicates that the test or special inspection shall be performed by a testing laboratory accepted in the DSA Laboratory Evaluation and Acceptance (LEA) Program. See CAC Section 4-335.

PI (Project Inspector) – Indicates that the special inspection may be performed by a project inspector when specifically approved by DSA.

SI (Special Inspection) – Indicates that the special inspection shall be performed by an appropriately qualified/approved special inspector.

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Table 1705A.6, Table 1705A.7, Table 1705A.8  
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S1. GENERAL:

Test or Special Inspection

Type

Performed By

Code References and Notes

☒ a. Verify that:  
• Site has been prepared properly prior to placement of controlled fill and/or excavations for foundations.  
• Foundation excavations are extended to proper depth and have reached proper material.  
• Materials below footings are adequate to achieve the design bearing capacity.

Periodic

GE\*

\* By geotechnical engineer or his or her qualified representative. (See Appendix (end of this form) for exemptions.)

S2. SOIL COMPACTION AND FILL:

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a. Perform classification and testing of fill materials.

Test

LOR\*

\* Under the supervision of the geotechnical engineer.

☒ b. Verify use of proper materials, densities and inspect lift thicknesses, placement and compaction during placement of fill.

Continuous

GE\*

\* By geotechnical engineer or his or her qualified representative. (Refer to specific items identified in the Appendix (end of this form) for exemptions where soils SI and testing may be conducted under the supervision of a geotechnical engineer or LOR's engineering manager. In such cases, the LOR's form DSA 291 shall satisfy the soil SI and test reporting requirements for the exempt items.)

☒ c. Compaction testing.

Test

LOR\*

\* Under the supervision of the geotechnical engineer. (Refer to specific items identified in the Appendix (end of this form) for exemptions where soils testing may be conducted under the supervision of a geotechnical engineer or LOR's engineering manager. In such cases, the LOR's form DSA 291 shall satisfy the soil test reporting requirements for the exempt items.)

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S3. DRIVEN DEEP FOUNDATIONS (PILES):

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a. Verify pile materials, sizes and lengths comply with the requirements.

Continuous

GE\*

\* By geotechnical engineer or his or her qualified representative.

☐ b. Determine capacities of test piles and conduct additional load tests as required.

Test

LOR\*

\* Under the supervision of the geotechnical engineer.

☐ c. Inspect driving operations and maintain complete and accurate records for each pile.

Continuous

GE\*

\* By geotechnical engineer or his or her qualified representative.

☐ d. Verify locations of piles and their plumbness, confirm type and size of hammer, record number of blows per foot of penetration, determine required penetrations to achieve design capacity, record tip and butt elevations and record any pile damage.

Continuous

GE\*

\* By geotechnical engineer or his or her qualified representative.

☐ e. Steel piles.

Provide tests and inspections per STEEL section below.

☐ f. Concrete piles and concrete filled piles.

Provide tests and inspections per CONCRETE section below.

☐ g. For specialty piles, perform additional inspections as determined by the registered design professional in responsible charge.

\*

\*

\* As defined on drawings or specifications.

S4. CAST-IN-PLACE DEEP FOUNDATIONS (PIERS):

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a. Inspect drilling operations and maintain complete and accurate records for each pier.

Continuous

GE\*

\* By geotechnical engineer or his or her qualified representative. (See Appendix (end of this form) for exemptions.)

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Table 1705A.3, ACI 318-19 Sections 26.12 & 26.13  
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C1. CAST-IN-PLACE CONCRETE

Test or Special Inspection

Type

Performed By

Code References and Notes

☒ a. Verify use of required design mix.

Periodic

SI

Table 1705A.3 Item 5, 1910A.1.

☒ b. Identify, sample, and test reinforcing steel.

Test

LOR

1910A.2; ACI 318-19 Ch.20 and Section 26.6.1.2; DSA IR 17-10. (See Appendix (end of this form) for exemptions.)

☒ c. During concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.

Test

LOR

Table 1705A.3 Item 6; ACI 318-19 Sections 26.5 & 26.12.

☒ d. Test concrete (f<sub>c</sub>).

Test

LOR

1905A.1.1.7; ACI 318-19 Section 26.12.

☐ e. Batch plant inspection:

See Notes

SI

Default of 'Continuous' per 1705A.3.3. If approved by DSA, batch plant inspection may be reduced to 'Periodic' subject to requirements in Section 1705A.3.3.1, or eliminated per 1705A.3.3.2. See IR 17-13. (See Appendix (end of this form) for exemptions.)

☐ f. Welding of reinforcing steel.

Provide special inspection per STEEL, Category S/A4(d) & (e) and/or S/A5(g) & (h) below.

C2. PRESTRESSED / POST-TENSIONED CONCRETE (IN ADDITION TO SECTION C1):

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a. Sample and test prestressing tendons and anchorages.

Test

LOR

1705A.3.4, 1910A.3

☐ b. Inspect placement of prestressing tendons.

Periodic

SI

1705A.3.4, Table 1705A.3 Items 1 & 9.

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DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (STEEL AND ALUMINUM), 2022 CBC

Table 1705A.2.1, Table 1705A.2.2, AISC 307-16, AISC 341-16, AISC 358-16, AISC 360-16, AISI 5100-20, RCSC 2014, AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.5  
Application Number: 04-122188  
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S/A1. STRUCTURAL STEEL, COLD-FORMED STEEL AND ALUMINUM USED FOR STRUCTURAL PURPOSES

Test or Special Inspection

Type

Performed By

Code References and Notes

☒ a. Verify identification of all materials and materials certificates indicate material properties that comply with requirements.  
• Material sizes, types and grades comply with requirements.

Periodic

SI

Table 1705A.2.1 Item 3a-3c, 2202A.1; AISI 5100-20 Section A3.1 & A3.2; AWS D200-20 Section A3.8 & A5; AISI 5200-20 Section A4.8 & A5. \* By special inspector or qualified technician when performed off-site.

☒ b. Test unidentified materials

Test

LOR

2202A.1

☒ c. Examine seam welds of HSS shapes

Periodic

SI

DSA IR 17-3.

☒ d. Verify and document steel fabrication per DSA-approved construction documents.

Periodic

SI

Not applicable to cold-formed steel light-frame construction, except for trusses (1705A.2.4).

☐ e. Buckling restrained braces.

Test

LOR

Testing and special inspections in accordance with IR 22-4.

S/A2. HIGH-STRENGTH BOLTS:

Test or Special Inspection

Type

Performed By

Code References and Notes

☒ a. Verify identification markings and manufacturer's certificates of compliance conform to ASTM standards specified in the DSA-approved documents.

Periodic

SI

Table 1705A.2.1 Items 1a & 1b, 2202A.1; AISC 360-16 Section A3.3, J3.1, and N3.2; RCSC 2014 Section 1.5 & 2.1; DSA IR 17-8 & DSA IR 17-9.

☒ b. Test high-strength bolts, nuts and washers.

Test

LOR

Table 1705A.2.1 Item 1c, 2213A.1; RCSC 2014 Section 7.2; DSA IR 17-8.

☒ c. Bearing-type ("snug tight") connections.

Periodic

SI

Table 1705A.2.1 Item 2a, 1705A.2.6, 2204A.2; AISC 360-16 J3.1, J3.2, M2.5 & N3.6; RCSC 2014 Sections 9.2 & 9.3; DSA IR 17-9.

☒ d. Pretensioned and slip-critical connections.

\*

SI

Table 1705A.2.1 Items 2b & 2c, 1705A.2.6, 2204A.2; AISC 360-16 J3.1, J3.2, M2.5 & N3.6; RCSC 2014 Sections 9.2 & 9.3; DSA IR 17-9. \* "Continuous" or "Periodic" depends on the tightening method used.

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C4. SHOTCRETE (IN ADDITION TO SECTION C1):

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a. Inspect shotcrete placement for proper application techniques.

Continuous

SI

1705A.3.9, Table 1705A.3 Item 7, 1908A.1, 1908A.2, 1908A.3. See ACI 506.2-13 Section 3.4, ACI 506R-16.

☐ b. Sample and test shotcrete (f<sub>c</sub>).

Test

LOR

1908A.2, 1705A.3.9

C5. POST-INSTALLED ANCHORS:

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a. Inspect installation of post-installed anchors

See Notes

SI\*

1617A.1.19, Table 1705A.3 Item 4a (Continuous) & 4b (Periodic), 1705A.3.8 (See Appendix (end of this form) for exemptions). ACI 318-14 Sections 17.8 & 26.13. \* May be performed by the project inspector when specifically approved by DSA.

☐ b. Test post-installed anchors.

Test

LOR

1910A.5. (See Appendix (end of this form) for exemptions.)

C6. OTHER CONCRETE:

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a.

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C3. PRECAST CONCRETE (IN ADDITION TO SECTION C1):

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a. Inspect fabrication of precast concrete members.

Continuous

SI

ACI 318-19 Section 26.13.

☐ b. Inspect erection of precast concrete members.

Periodic

SI\*

Table 1705A.3 Item 10. \* May be performed by PI when specifically approved by DSA.

☐ c. For precast concrete diaphragm connections or reinforcement at joints classified as moderate or high deformability elements (MDE or HDE) in structures assigned to Seismic Design Category D, E or F, inspect such connections and reinforcement in the field for:  
1. Installation of the embedded parts  
2. Completion of the continuity of reinforcement across joints.  
3. Completion of connections in the field.

Continuous

SI

Table 1705A.3; ACI 318-19 Section 26.13.1.3; ACI 550.5

☐ d. Inspect installation tolerances of precast concrete diaphragm connections for compliance with ACI 550.5.

Periodic

SI

Table 1705A.3; ACI 318-19 Section 26.13.1.3; ACI 550.5

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S5. RETAINING WALLS:

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a. Placement, compaction and inspection of backfill.

Continuous

GE\*

1705A.6.1. \* By geotechnical engineer or his or her qualified representative. (See section S2 above).

☐ b. Placement of soil reinforcement and/or drainage devices.

Continuous

GE\*

\* By geotechnical engineer or his or her qualified representative. (See Appendix (end of this form) for exemptions.)

☐ c. Segmental retaining walls; inspect placement of units, dowels, connectors, etc.

Continuous

GE\*

\* By geotechnical engineer or his or her qualified representative. See DSA IR 18-2.

☐ d. Concrete retaining walls.

Provide tests and inspections per CONCRETE section below.

☐ e. Masonry retaining walls.

Provide tests and inspections per MASONRY section below.

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☐ c. Verify in-situ concrete strength prior to stressing of post-tensioning tendons.

Periodic

SI

Table 1705A.3 Item 13. Special inspector to verify specified concrete strength test prior to stressing.

☐ d. Inspect application of post-tensioning or prestressing forces and grouting of bonded prestressing tendons.

Continuous

SI

1705A.3.4, Table 1705A.3 Item 9; ACI 318-14 Section 26.13

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Table 1705A.2.1, Table 1705A.2.2, AISC 307-16, AISC 341-16, AISC 358-16, AISC 360-16, AISI 5100-20, RCSC 2014, AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.5  
Application Number: 04-122188  
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S/A1. STRUCTURAL STEEL, COLD-FORMED STEEL AND ALUMINUM USED FOR STRUCTURAL PURPOSES

Test or Special Inspection

Type

Performed By

Code References and Notes

☒ a. Verify identification of all materials and materials certificates indicate material properties that comply with requirements.  
• Material sizes, types and grades comply with requirements.

Periodic

SI

Table 1705A.2.1 Item 3a-3c, 2202A.1; AISI 5100-20 Section A3.1 & A3.2; AWS D200-20 Section A3.8 & A5; AISI 5200-20 Section A4.8 & A5. \* By special inspector or qualified technician when performed off-site.

☒ b. Test unidentified materials

Test

LOR

2202A.1

☒ c. Examine seam welds of HSS shapes

Periodic

SI

DSA IR 17-3.

☒ d. Verify and document steel fabrication per DSA-approved construction documents.

Periodic

SI

Not applicable to cold-formed steel light-frame construction, except for trusses (1705A.2.4).

☐ e. Buckling restrained braces.

Test

LOR

Testing and special inspections in accordance with IR 22-4.

S/A2. HIGH-STRENGTH BOLTS:

Test or Special Inspection

Type

Performed By

Code References and Notes

☒ a. Verify identification markings and manufacturer's certificates of compliance conform to ASTM standards specified in the DSA-approved documents.

Periodic

SI

Table 1705A.2.1 Items 1a & 1b, 2202A.1; AISC 360-16 Section A3.3, J3.1, and N3.2; RCSC 2014 Section 1.5 & 2.1; DSA IR 17-8 & DSA IR 17-9.

☒ b. Test high-strength bolts, nuts and washers.

Test

LOR

Table 1705A.2.1 Item 1c, 2213A.1; RCSC 2014 Section 7.2; DSA IR 17-8.

☒ c. Bearing-type ("snug tight") connections.

Periodic

SI

Table 1705A.2.1 Item 2a, 1705A.2.6, 2204A.2; AISC 360-16 J3.1, J3.2, M2.5 & N3.6; RCSC 2014 Sections 9.2 & 9.3; DSA IR 17-9.

☒ d. Pretensioned and slip-critical connections.

\*

SI

Table 1705A.2.1 Items 2b & 2c, 1705A.2.6, 2204A.2; AISC 360-16 J3.1, J3.2, M2.5 & N3.6; RCSC 2014 Sections 9.2 & 9.3; DSA IR 17-9. \* "Continuous" or "Periodic" depends on the tightening method used.

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C4. SHOTCRETE (IN ADDITION TO SECTION C1):

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a. Inspect shotcrete placement for proper application techniques.

Continuous

SI

1705A.3.9, Table 1705A.3 Item 7, 1908A.1, 1908A.2, 1908A.3. See ACI 506.2-13 Section 3.4, ACI 506R-16.

☐ b. Sample and test shotcrete (f<sub>c</sub>).

Test

LOR

1908A.2, 1705A.3.9

C5. POST-INSTALLED ANCHORS:

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a. Inspect installation of post-installed anchors

See Notes

SI\*

1617A.1.19, Table 1705A.3 Item 4a (Continuous) & 4b (Periodic), 1705A.3.8 (See Appendix (end of this form) for exemptions). ACI 318-14 Sections 17.8 & 26.13. \* May be performed by the project inspector when specifically approved by DSA.

☐ b. Test post-installed anchors.

Test

LOR

1910A.5. (See Appendix (end of this form) for exemptions.)

C6. OTHER CONCRETE:

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a.

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C3. PRECAST CONCRETE (IN ADDITION TO SECTION C1):

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a. Inspect fabrication of precast concrete members.

Continuous

SI

ACI 318-19 Section 26.13.

☐ b. Inspect erection of precast concrete members.

Periodic

SI\*

Table 1705A.3 Item 10. \* May be performed by PI when specifically approved by DSA.

☐ c. For precast concrete diaphragm connections or reinforcement at joints classified as moderate or high deformability elements (MDE or HDE) in structures assigned to Seismic Design Category D, E or F, inspect such connections and reinforcement in the field for:  
1. Installation of the embedded parts  
2. Completion of the continuity of reinforcement across joints.  
3. Completion of connections in the field.

Continuous

SI

Table 1705A.3; ACI 318-19 Section 26.13.1.3; ACI 550.5

☐ d. Inspect installation tolerances of precast concrete diaphragm connections for compliance with ACI 550.5.

Periodic

SI

Table 1705A.3; ACI 318-19 Section 26.13.1.3; ACI 550.5

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S1. GENERAL:

Test or Special Inspection

Type

Performed By

Code References and Notes

☒ a. Verify that:  
• Site has been prepared properly prior to placement of controlled fill and/or excavations for foundations.  
• Foundation excavations are extended to proper depth and have reached proper material.  
• Materials below footings are adequate to achieve the design bearing capacity.

Periodic

GE\*

\* By geotechnical engineer or his or her qualified representative. (See Appendix (end of this form) for exemptions.)

S2. SOIL COMPACTION AND FILL:

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a. Perform classification and testing of fill materials.

Test

LOR\*

\* Under the supervision of the geotechnical engineer.

☒ b. Verify use of proper materials, densities and inspect lift thicknesses, placement and compaction during placement of fill.

Continuous

GE\*

\* By geotechnical engineer or his or her qualified representative. (Refer to specific items identified in the Appendix (end of this form) for exemptions where soils SI and testing may be conducted under the supervision of a geotechnical engineer or LOR's engineering manager. In such cases, the LOR's form DSA 291 shall satisfy the soil SI and test reporting requirements for the exempt items.)

☒ c. Compaction testing.

Test

LOR\*

\* Under the supervision of the geotechnical engineer. (Refer to specific items identified in the Appendix (end of this form) for exemptions where soils testing may be conducted under the supervision of a geotechnical engineer or LOR's engineering manager. In such cases, the LOR's form DSA 291 shall satisfy the soil test reporting requirements for the exempt items.)

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S3. DRIVEN DEEP FOUNDATIONS (PILES):

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a. Verify pile materials, sizes and lengths comply with the requirements.

Continuous

GE\*

\* By geotechnical engineer or his or her qualified representative.

☐ b. Determine capacities of test piles and conduct additional load tests as required.

Test

LOR\*

\* Under the supervision of the geotechnical engineer.

☐ c. Inspect driving operations and maintain complete and accurate records for each pile.

Continuous

GE\*

\* By geotechnical engineer or his or her qualified representative.

☐ d. Verify locations of piles and their plumbness, confirm type and size of hammer, record number of blows per foot of penetration, determine required penetrations to achieve design capacity, record tip and butt elevations and record any pile damage.

Continuous

GE\*

\* By geotechnical engineer or his or her qualified representative.

☐ e. Steel piles.

Provide tests and inspections per STEEL section below.

☐ f. Concrete piles and concrete filled piles.

Provide tests and inspections per CONCRETE section below.

☐ g. For specialty piles, perform additional inspections as determined by the registered design professional in responsible charge.

\*

\*

\* As defined on drawings or specifications.

S4. CAST-IN-PLACE DEEP FOUNDATIONS (PIERS):

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a. Inspect drilling operations and maintain complete and accurate records for each pier.

Continuous

GE\*

\* By geotechnical engineer or his or her qualified representative. (See Appendix (end of this form) for exemptions.)

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C1. CAST-IN-PLACE CONCRETE

Test or Special Inspection

Type

Performed By

Code References and Notes

☒ a. Verify use of required design mix.

Periodic

SI

Table 1705A.3 Item 5, 1910A.1.

☒ b. Identify, sample, and test reinforcing steel.

Test

LOR

1910A.2; ACI 318-19 Ch.20 and Section 26.6.1.2; DSA IR 17-10. (See Appendix (end of this form) for exemptions.)

☒ c. During concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.

Test

LOR

Table 1705A.3 Item 6; ACI 318-19 Sections 26.5 & 26.12.

☒ d. Test concrete (f<sub>c</sub>).

Test

LOR

1905A.1.1.7; ACI 318-19 Section 26.12.

☐ e. Batch plant inspection:

See Notes

SI

Default of 'Continuous' per 1705A.3.3. If approved by DSA, batch plant inspection may be reduced to 'Periodic' subject to requirements in Section 1705A.3.3.1, or eliminated per 1705A.3.3.2. See IR 17-13. (See Appendix (end of this form) for exemptions.)

☐ f. Welding of reinforcing steel.

Provide special inspection per STEEL, Category S/A4(d) & (e) and/or S/A5(g) & (h) below.

C2. PRESTRESSED / POST-TENSIONED CONCRETE (IN ADDITION TO SECTION C1):

Test or Special Inspection

Type

Performed By

Code References and Notes

☐ a. Sample and test prestressing tendons and anchorages.

Test

LOR

1705A.3.4, 1910A.3

☐ b. Inspect placement of prestressing tendons.

Periodic

SI

1705A.3.4, Table 1705A.3 Items 1 & 9.

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Table 1705A.2.1, Table 1705A.2.2, AISC 307-16, AISC 341-16, AISC 358-16, AISC 360-16, AISI 5100-20, RCSC 2014, AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.



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1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

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School Name: PC Update  
School District: PC Update  
DSA File Number: Increment Number: 2023-04-19 08:36:32

S/A3. WELDING:	Type	Performed By	Code References and Notes
<input checked="" type="checkbox"/> a. Verify weld filler material identification markings per AWS designation listed on the DSA-approved documents and the WPS.	Periodic	SI	1705A.2.5, Table 1705A.2.1 Items 4 & 5; AWS D1.1 and AWS D1.8 for structural steel; AWS D1.2 for Aluminum; AWS D1.3 for cold-formed steel; AWS D1.4 for reinforcing steel; DSA IR 17-3.
<input checked="" type="checkbox"/> b. Verify weld filler material manufacturer's certificate of compliance.	Periodic	SI	DSA IR 17-3.
<input checked="" type="checkbox"/> c. Verify WPS, welder qualifications and equipment.	Periodic	SI	DSA IR 17-3.

S/A4. SHOP WELDING (IN ADDITION TO SECTION S/A3):	Type	Performed By	Code References and Notes
<input checked="" type="checkbox"/> a. Inspect groove welds, multi-pass fillet welds, single pass fillet welds > 5/16", plug and slot welds.	Continuous	SI	Table 1705A.2.1 Items 5a.1-4; AISC 360-16 (and AISC 341-16 as applicable); DSA IR 17-3.
<input checked="" type="checkbox"/> b. Inspect single-pass fillet welds ≤ 5/16", floor and roof deck welds.	Periodic	SI	1705A.2.2, Table 1705A.2.1 Items 5a.5 & 5a.6; AISC 360-16 (and AISC 341-16 as applicable); DSA IR 17-3.
<input type="checkbox"/> c. Inspect welding of stairs and railing systems.	Periodic	SI	1705A.2.1; AISC 360-16 (and AISC 341-16 as applicable); AWS D1.1 & D1.3; DSA IR 17-3.
<input type="checkbox"/> d. Verification of reinforcing steel weldability other than ASTM A706.	Periodic	SI	1705A.3.1; AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on mill certificates.
<input type="checkbox"/> e. Inspect welding of reinforcing steel.	Continuous	SI	Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A.8; AWS D1.4; DSA IR 17-3.

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1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

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Test or Special Inspection	Type	Performed By	Code References and Notes
S/A8. SPRAYED FIRE-RESISTANT MATERIALS:			
<input type="checkbox"/> a. Examine structural steel surface conditions, inspect application, take samples, measure thickness and verify compliance of all aspects of application with DSA-approved documents.	Periodic	SI	1705A.15, 1705A.1, 1705A.2, 1705A.3, 1705A.4.
<input type="checkbox"/> b. Test density.	Test	LOR	1705A.15.1, 1705A.15.5, ASTM E736
<input type="checkbox"/> c. Bond strength adhesion/cohesion.	Test	LOR	1705A.15.1, 1705A.15.4, ASTM E605

S/A9. ANCHOR BOLTS AND ANCHOR RODS:	Type	Performed By	Code References and Notes
<input checked="" type="checkbox"/> a. Anchor Bolts and Anchor Rods	Test	LOR	Sample and test anchor bolts and anchor rods not readily identifiable per procedures noted in DSA IR 17-1.1.
<input type="checkbox"/> b. Threaded rod not used for foundation anchorage.	Test	LOR	Sample and test threaded rods not readily identifiable per procedures noted in DSA IR 17-1.1.

S/A10. STORAGE RACK SYSTEMS:	Type	Performed By	Code References and Notes
<input type="checkbox"/> a. Materials used, to verify compliance with one or more of the material test reports in accordance with the approved construction documents.	Periodic	SI	Table 1705A.13.7
<input type="checkbox"/> b. Fabricated storage rack elements.	Periodic	SI	1704A.2.5; Table 1705A.13.7

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Appendix: Work Exempt from DSA Requirements for Structural Tests / Special Inspections

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Exempt items given in DSA IR A-22 or the 2019 CBC (including DSA amendments) and those items identified below with a check mark by the design professional are NOT subject to DSA requirements for the structural tests / special inspections noted. Items marked as exempt shall be identified on the approved construction documents. The project inspector shall verify all construction complies with the approved construction documents.

SOILS:
<input type="checkbox"/> 1. Deep foundations acting as a cantilever footing with a design based on minimum allowable pressures per CBC Table 1806A.2 and without a geotechnical report for the following cases: A) free standing sign or scoreboard, B) cell or antenna towers and poles less than 35'-0" tall (e.g., lighting poles, flag poles, poles supporting open mesh fences, etc.), C) single-story structure with dead load less than 5 psf (e.g., open fabric shade structure), or D) covered walkway structure with an apex height less than 10'-0" above adjacent grade.
<input type="checkbox"/> 2. Shallow foundations, etc. are exempt from special inspections and testing by a Geotechnical Engineer for the following cases: A) buildings without a geotechnical report and meeting the exception item #1 criteria in CBC Section 1803A.2 supported by native soil (any excavation depth) or fill soil (not exceeding 12" depth per CBC Section 1804A.6), B) soil scarification/recompaction not exceeding 12" depth, C) native or fill soil supporting exterior non-structural flatwork (e.g., sidewalks, site concrete ramps, site stairs, parking lots, driveways, etc.), D) unpaved landscaping and playground areas, or E) utility trench backfill.

CONCRETE/MASONRY:
<input type="checkbox"/> 1. Post-installed anchors for the following: A) exempt non-structural components (e.g., mechanical, electrical, plumbing equipment - see item 7 for "Welding" in the Appendix below) given in CBC Section 1617A.1.18 (which replaces ASCE 7-16, Section 13.1.4) or B) interior nonstructural wall partitions meeting criteria listed in exempt item 3 for "Welding" in the Appendix below
<input type="checkbox"/> 2. Concrete batch plant inspection is not required for items given in CBC Section 1705A.3.3.2 subject to the requirements and limitations in that section.
<input type="checkbox"/> 3. Non-bearing non-shear masonry walls may be exempt from certain DSA masonry testing and special inspection items as allowed per DSA IR 21-1. Refer to construction documents for specific exemptions accordingly for each applicable wall condition.
<input type="checkbox"/> 4. Epoxy shear dowels in site flatwork and/or other non-structural concrete.

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DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (STEEL AND ALUMINUM), 2022 CBC

1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

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School Name: PC Update  
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Test or Special Inspection	Type	Performed By	Code References and Notes
S/A5. FIELD WELDING (IN ADDITION TO SECTION S/A3):			
<input type="checkbox"/> a. Inspect groove welds, multi-pass fillet welds, single pass fillet welds > 5/16", plug and slot welds.	Continuous	SI	Table 1705A.2.1 Items 5a.1-4; AISC 360-16 (AISC 341-16 as applicable); DSA IR 17-3.
<input type="checkbox"/> b. Inspect single-pass fillet welds ≤ 5/16".	Periodic	SI	Table 1705A.2.1 Item 5a.5; AISC 360-16 (AISC 341-16 as applicable); DSA IR 17-3.
<input type="checkbox"/> c. Inspect end-welded studs (ASTM A-108) installation (including bend test).	Periodic	SI	2213A.2; AISC 360-16 (AISC 341-16 as applicable); AWS D1.1; DSA IR 17-3.
<input type="checkbox"/> d. Inspect floor and roof deck welds.	Periodic	SI	1705A.2.2, Table 1705A.2.1 Item 5a.6; AISC 360-16 (AISC 341-16 as applicable); AWS D1.3; DSA IR 17-3.
<input type="checkbox"/> e. Inspect welding of structural cold-formed steel.	Periodic	SI*	1705A.2.5; AWS D1.3; DSA IR 17-3. The quality control provisions of AISI S240-20 Chapter D shall also apply. * May be performed by the project inspector when specifically approved by DSA.
<input type="checkbox"/> f. Inspect welding of stairs and railing systems.	Periodic	SI*	1705A.2.1; AISC 360-16 (AISC 341-16 as applicable); AWS D1.1 & D1.3; DSA IR 17-3. * May be performed by the project inspector when specifically approved by DSA.
<input type="checkbox"/> g. Verification of reinforcing steel weldability.	Periodic	SI	1705A.3.1; AWS D1.4; DSA IR 17-3. Verify carbon equivalent reported on mill certificates.
<input type="checkbox"/> h. Inspect welding of reinforcing steel.	Continuous	SI	Table 1705A.2.1 Item 5b, 1705A.3.1, Table 1705A.3 Item 2, 1903A.8; AWS D1.4; DSA IR 17-3.

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1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

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Test or Special Inspection	Type	Performed By	Code References and Notes
S/A11. Other Steel			
<input type="checkbox"/> a.			

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Appendix: Work Exempt from DSA Requirements for Structural Tests / Special Inspections

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CONCRETE/MASONRY:
<input type="checkbox"/> 5. Testing of reinforcing bars is not required for items given in CBC Section 1910A.2 subject to the requirements and limitations in that section.

WELDING:
<input type="checkbox"/> 1. Solid-clad and open-mesh fences, gates with maximum leaf span of 10', and gates with a maximum rolling section of 10' all having an apex height less than 8'-0" above lowest adjacent grade. When located above circulation or occupied space below, these gates/fences are not located within 1.5x gate/fence height (max 8'-0") to the edge of floor or roof.
<input type="checkbox"/> 2. Handrails, guardrails, and modular or relocatable ramps associated with walking surfaces less than 30" above adjacent grade (excluding post base connections per the "Exception" language in Section 1705A.2.1); fillet welds shall not be ground flush.
<input type="checkbox"/> 3. Non-structural interior cold-formed steel framing spanning less than 15'-0", such as in interior partitions, interior soffits, etc. supporting only self weight and light-weight finishes or adhered tile, masonry, stone, or terra cotta veneer no more than 5/8" thickness and apex less than 20'-0" in height and not over an exit way. Maximum tributary load to a member shall not exceed the equivalent of that occurring from a 10'x10' opening in a 15' tall wall for a header or king stud.
<input type="checkbox"/> 4. Manufactured support frames and curbs using hot rolled or cold-formed steel (i.e., light gauge) for mechanical, electrical, or plumbing equipment weighing less than 2000# (equipment only) (connections of such frames to superstructure elements using welding will require special inspection as noted in selected item(s) for Sections S/A3, S/A4 and/or S/A5 of listing above).
<input type="checkbox"/> 5. Manufactured components (e.g., Tolco, B-Line, Alcon, etc.) for mechanical, electrical, or plumbing hanger support and bracing (connections of such components to superstructure elements using welding will require special inspection as noted in selected item(s) for Sections S/A3, S/A4 and/or S/A5 of listing above).
<input type="checkbox"/> 6. TV Brackets, projector mounts with a valid listing (see DSA IR A-5) and recreational equipment (e.g., playground structures, basketball backstops, etc.) (connections of such elements to superstructure elements using welding will require special inspection as noted in selected item(s) for sections S/A3, S/A4 and/or S/A5 located in the Steel/Aluminum category of listing above).
<input type="checkbox"/> 7. Any support for exempt non-structural components given in CBC Section 1617A.1.18 (which replaces ASCE 7-16, Section 13.1.4) meeting the following: A) when supported on a floor/roof, <400# and resulting composite center of mass (including component's center of mass) ≤ 4" above supporting floor/roof, B) when hung from a wall or roof/floor, <20# for discrete units or <5 psf for distributed systems.

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1705A.2.1, Table 1705A.2.1; AISC 303-16, AISC 341-16, AISC 358-16, AISC 360-16; AISI S100-20; RCSC 2014; AWS D1.1, AWS D1.2, AWS D1.3, AWS D1.4, AWS D1.8

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Test or Special Inspection	Type	Performed By	Code References and Notes
S/A6. NONDESTRUCTIVE TESTING:			
<input type="checkbox"/> a. Ultrasonic	Test	LOR	1705A.2.1, 1705A.2.5; AISC 341-16 J6.2, AISC 360-16 N5.5; AWS D1.1, AWS D1.8; DSA IR 17-2.
<input type="checkbox"/> b. Magnetic Particle	Test	LOR	1705A.2.1, 1705A.2.5; AISC 341-16 J6.2, AISC 360-16 N5.5; AWS D1.1, AWS D1.8; DSA IR 17-2.
<input type="checkbox"/> c.	Test	LOR	

S/A7. STEEL JOISTS AND TRUSSES:	Type	Performed By	Code References and Notes
<input type="checkbox"/> a. Verify size, type and grade for all chord and web members as well as connectors and weld filler material; verify joist profile, dimensions and camber (if applicable); verify all weld locations, lengths and profiles; mark or tag each joist.	Continuous	SI	1705A.2.3, Table 1705A.2.3; AWS D1.1; DSA IR 22-3 for steel joists only. 1705A.2.4; AWS D1.3 for cold-formed steel trusses.

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DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (OTHER), 2022 CBC

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X1. OTHER:			
<input type="checkbox"/> a. Load test for identified product(s).	Test	LOR	1709A.2, 1709A.3. Testing is not required for: 1) a product with a valid evaluation service report per DSA IR A-5, or 2) a product that can be justified by structural calculation.
<input type="checkbox"/> b. Installation torque for non-HS bolts	Continuous	SI*	Applicable to communication towers identified as Essential Service Facility Projects (ESFP). Calibrated wrench use required, verified by SI during installation. DSA Policy PL 18-01: Communication Towers, Poles and Buildings Utilized by State Agencies for Essential Services Communications. *EXCEPTION: Non-ESFP may use PI without need for notification to DSA.
<input type="checkbox"/> c.			

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DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SIGNATURE), 2022 CBC

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School Name: PC Update  
School District: PC Update  
DSA File Number: Increment Number: 2023-04-19 08:36:32

Name of Architect or Engineer in general responsible charge:

Name of Structural Engineer (When structural design has been delegated):

Signature of Architect or Structural Engineer: Date:

Note: To facilitate DSA electronic mark-ups and identification stamp application, DSA recommends against using secured electronic or digital signatures.

DSA STAMP

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PRE-CHECK (PC) DOCUMENT  
Code: 2022 CBC  
A separate project application for construction is required.

ICOM STD RH/DSA-PC  
DRAWN BY: JD  
DATE: 7/25/2023  
REV  
REV DATE

JRMA  
ARCHITECTS ENGINEERS  
2705 SALINAS STREET, SUITE 100, CA 92021  
TEL: 619.454.1870 FAX: 619.454.1871  
WWW.JRMA.COM

U.S. DEPARTMENT OF THE ARMY  
ENGINEER REGIMENT  
OCT 04, 2023

APPROVED  
DIV. OF THE STATE ARCHITECT  
APP: 04-122275 PC  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☒ CG ☐  
DATE: 10/10/2023

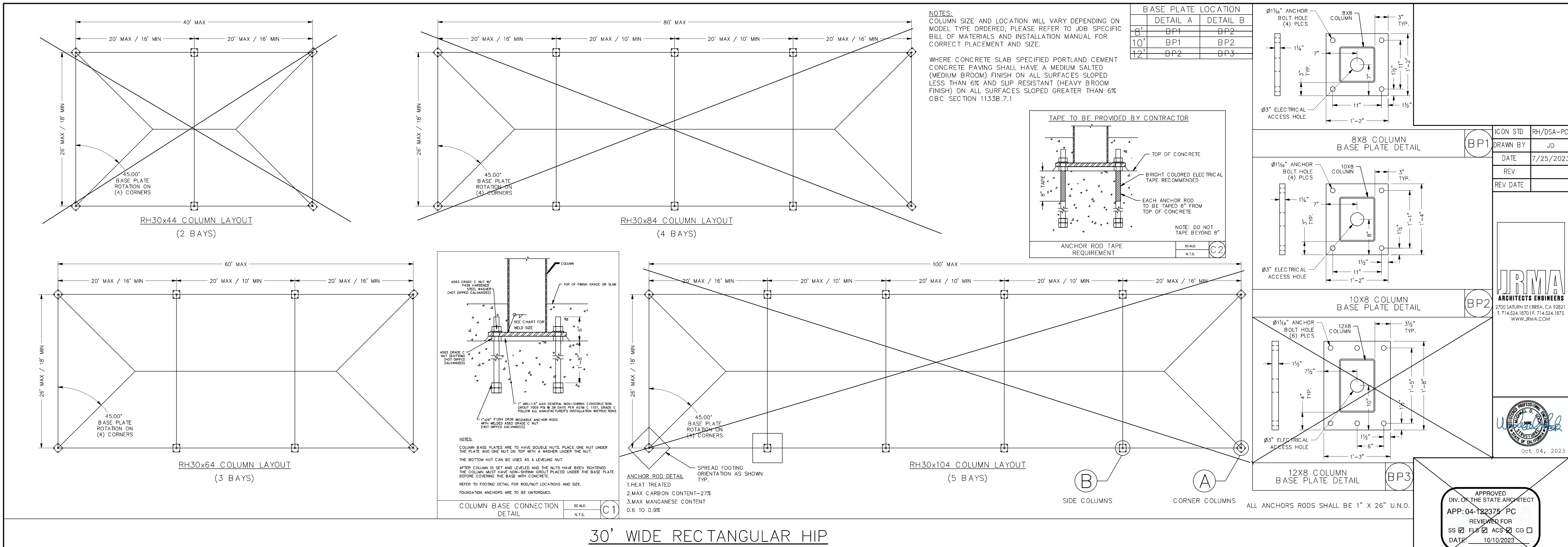
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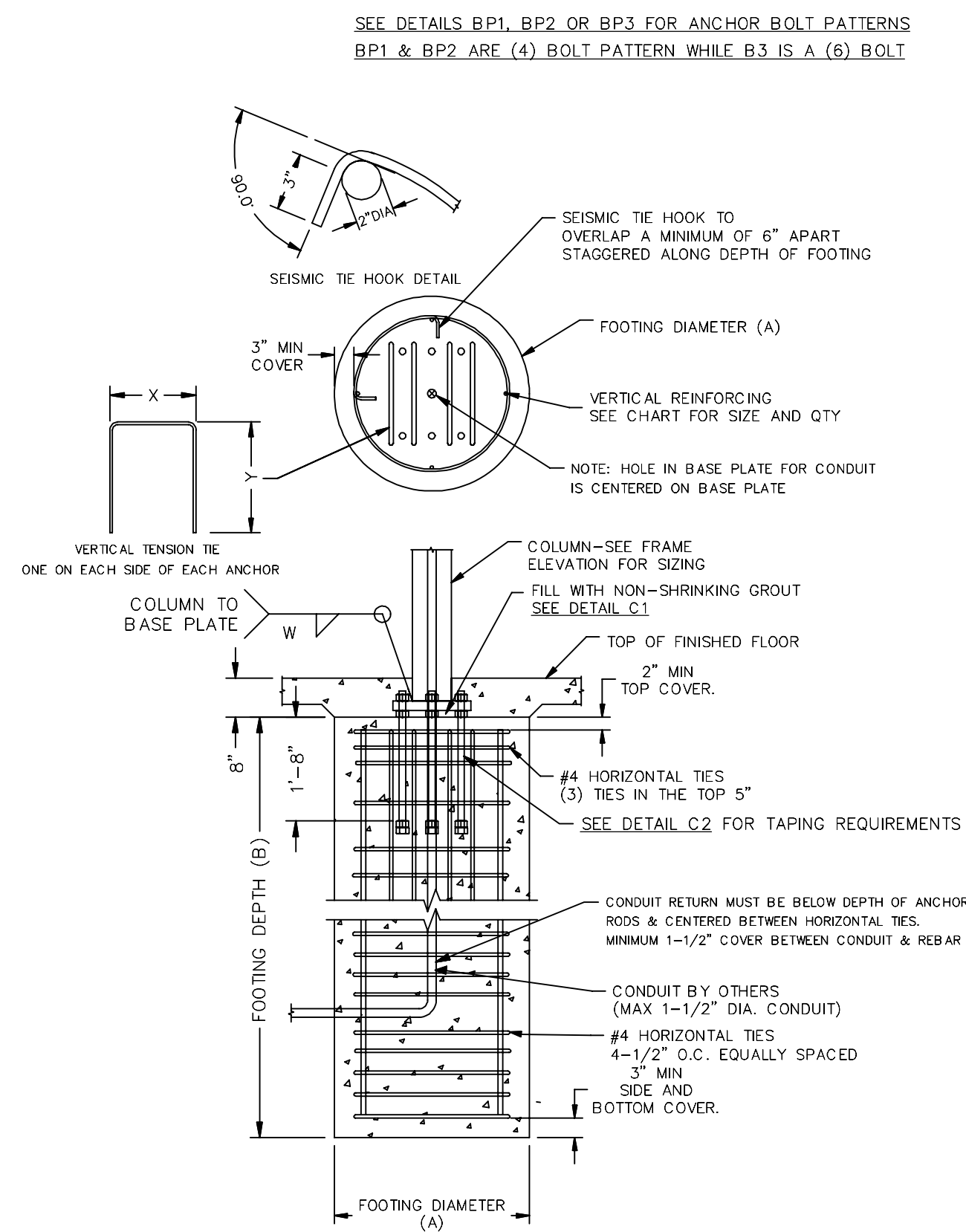
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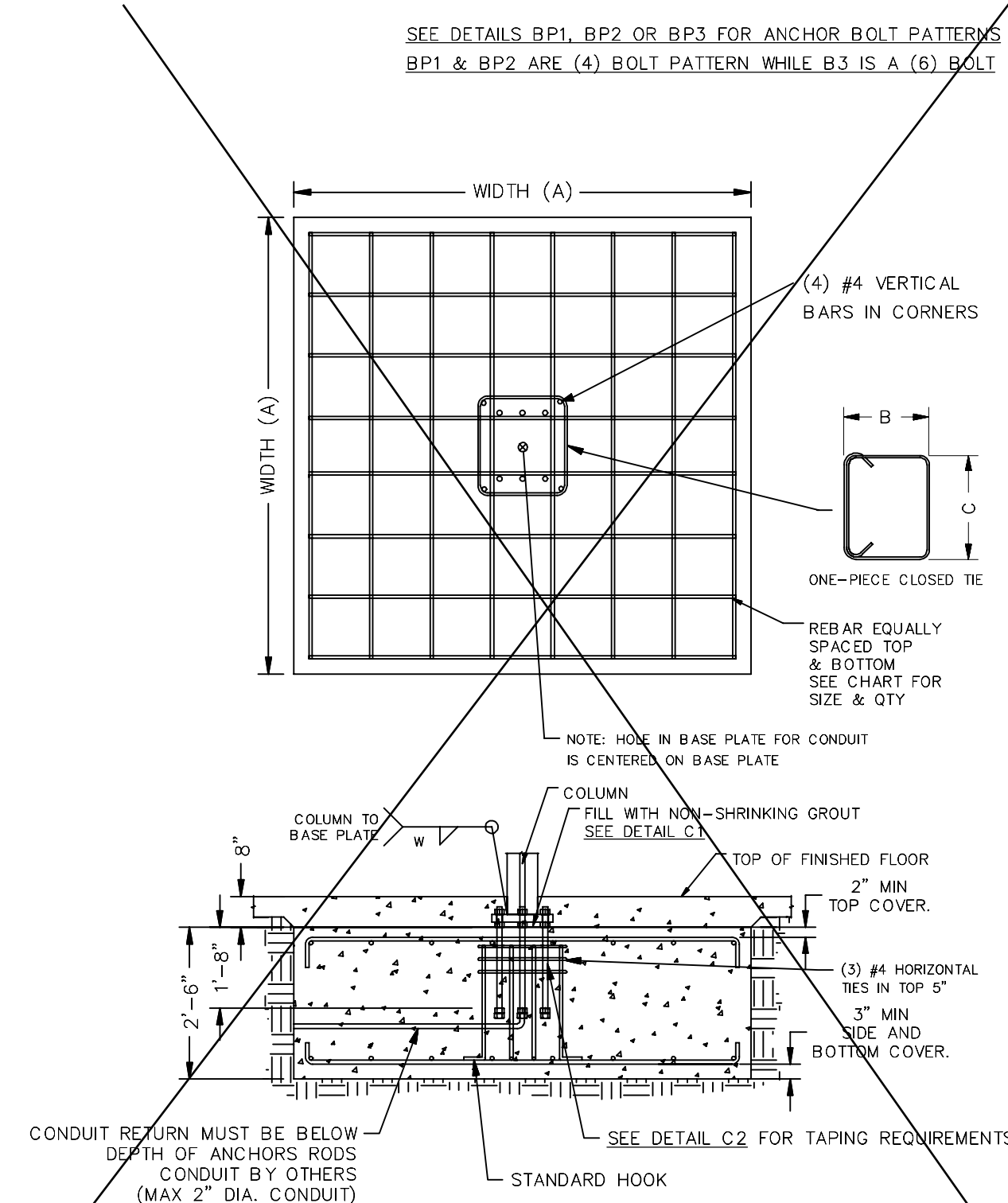




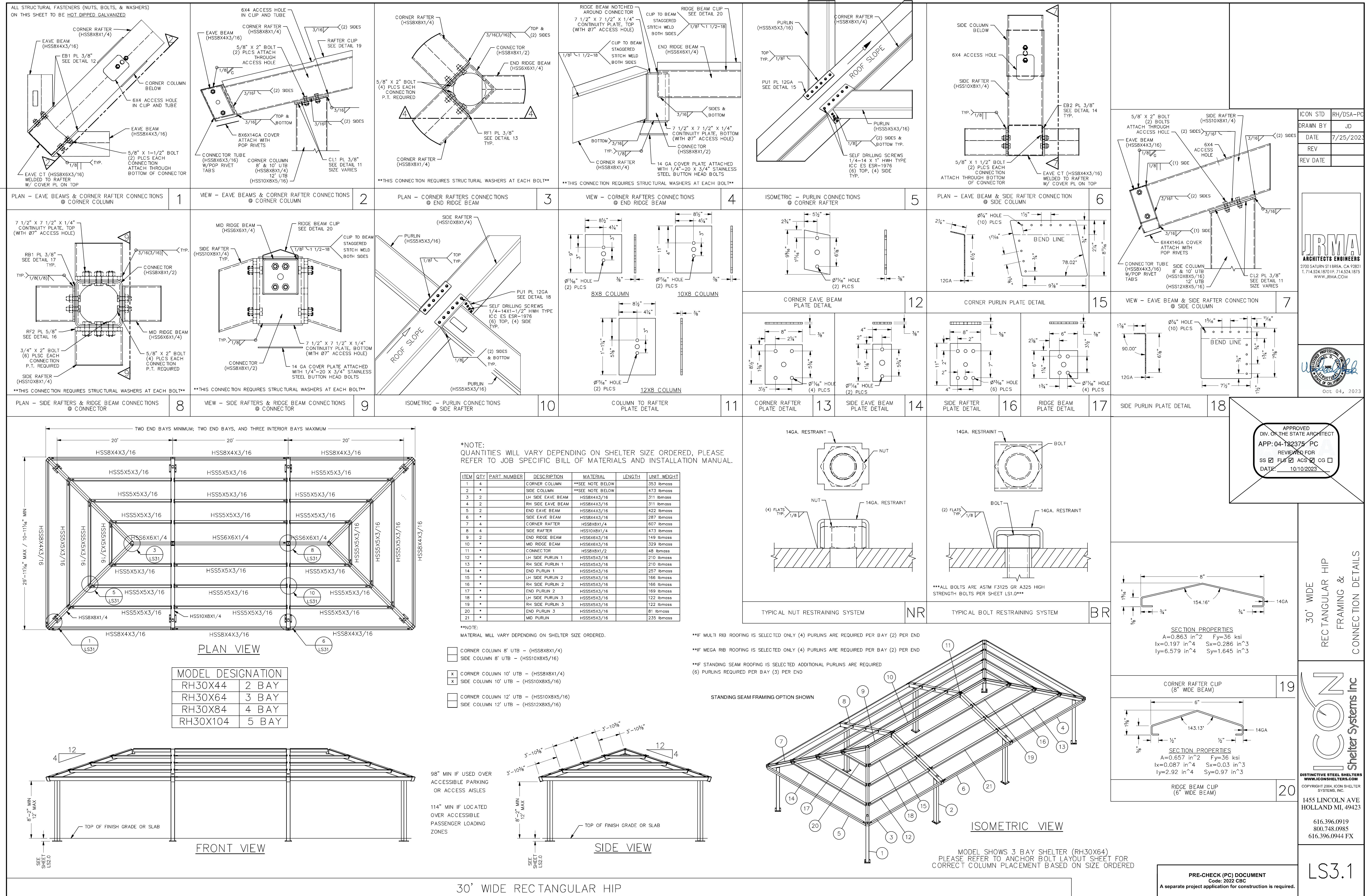
<b>RH30 - PIER</b>																			
<b>8' height - Corner Columns</b>					<b>8' height - Corner Columns</b>					<b>8' height - Corner Columns</b>					<b>8' height - Corner Columns</b>				
Soil Class = 2-1000 psf bearing					Soil Class = 2-1000 psf bearing					Soil Class = 3-1000 psf bearing					Soil Class = 3-1000 psf bearing				
Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar Size	Rebar Spacing (in)	Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar Size	Rebar Spacing (in)	Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar Size	Rebar Spacing (in)	X (in)	Y (in)	Rebar Size	Filllet Width (in)	Rebar Dimensions
36	10	8	6	30	36	10	8	6	30	36	10	8	6	30	12	47	6	1/4"	Rebar Dimensions
36	10	8	6	30	36	10	8	6	30	36	10	8	6	30	12	47	6	1/4"	Rebar Dimensions
Soil Class = 1500 psf bearing					Soil Class = 2-1000 psf bearing					Soil Class = 3-1000 psf bearing					Soil Class = 3-1000 psf bearing				
Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar Size	Rebar Spacing (in)	Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar Size	Rebar Spacing (in)	Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar Size	Rebar Spacing (in)	X (in)	Y (in)	Rebar Size	Filllet Width (in)	Rebar Dimensions
36	14	8	6	30	36	14	8	6	30	36	14	8	6	30	12	47	6	1/4"	Rebar Dimensions
36	14	8	6	30	36	14	8	6	30	36	14	8	6	30	12	47	6	1/4"	Rebar Dimensions
<b>8' Eave-1500 psf</b>					<b>8' Eave-2000 psf</b>					<b>8' Eave-3000 psf</b>					<b>8' Eave-Rebar &amp; Weld</b>				
Soil Class = 1500 psf bearing					Soil Class = 2-1000 psf bearing					Soil Class = 3-1000 psf bearing					Soil Class = 3-1000 psf bearing				
Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar Size	Rebar Spacing (in)	Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar Size	Rebar Spacing (in)	Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar Size	Rebar Spacing (in)	X (in)	Y (in)	Rebar Size	Filllet Width (in)	Rebar Dimensions
36	14	8	6	30	36	14	8	6	30	36	14	8	6	30	12	47	6	1/4"	Rebar Dimensions
36	14	8	6	30	36	14	8	6	30	36	14	8	6	30	12	47	6	1/4"	Rebar Dimensions
Soil Class = 1500 psf bearing					Soil Class = 2-1000 psf bearing					Soil Class = 3-1000 psf bearing					Soil Class = 3-1000 psf bearing				
Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar Size	Rebar Spacing (in)	Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar Size	Rebar Spacing (in)	Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar Size	Rebar Spacing (in)	X (in)	Y (in)	Rebar Size	Filllet Width (in)	Rebar Dimensions
36	14	8	6	30	36	14	8	6	30	36	14	8	6	30	12	47	6	1/4"	Rebar Dimensions
36	14	8	6	30	36	14	8	6	30	36	14	8	6	30	12	47	6	1/4"	Rebar Dimensions
<b>10' Eave-1500 psf</b>					<b>10' Eave-2000 psf</b>					<b>10' Eave-3000 psf</b>					<b>10' Eave-Rebar &amp; Weld</b>				
Soil Class = 1500 psf bearing					Soil Class = 2-1000 psf bearing					Soil Class = 3-1000 psf bearing					Soil Class = 3-1000 psf bearing				
Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar	Rebar Spacing (in)	Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar	Rebar Spacing (in)	Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar	Rebar Spacing (in)	X (in)	Y (in)	Rebar Size	Filllet Width (in)	Rebar Dimensions
36	10	8	6	30	36	10	8	6	30	36	10	8	6	30	12	47	6	1/4"	Rebar Dimensions
36	10	8	6	30	36	10	8	6	30	36	10	8	6	30	12	47	6	1/4"	Rebar Dimensions
Soil Class = 1500 psf bearing					Soil Class = 2-1000 psf bearing					Soil Class = 3-1000 psf bearing					Soil Class = 3-1000 psf bearing				
Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar	Rebar Spacing (in)	Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar	Rebar Spacing (in)	Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar	Rebar Spacing (in)	X (in)	Y (in)	Rebar Size	Filllet Width (in)	Rebar Dimensions
36	14	8	6	30	36	14	8	6	30	36	14	8	6	30	12	47	6	1/4"	Rebar Dimensions
36	14	8	6	30	36	14	8	6	30	36	14	8	6	30	12	47	6	1/4"	Rebar Dimensions
<b>10' Eave-1500 psf</b>					<b>10' Eave-2000 psf</b>					<b>10' Eave-3000 psf</b>					<b>10' Eave-Rebar &amp; Weld</b>				
Soil Class = 1500 psf bearing					Soil Class = 2-1000 psf bearing					Soil Class = 3-1000 psf bearing					Soil Class = 3-1000 psf bearing				
Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar	Rebar Spacing (in)	Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar	Rebar Spacing (in)	Di (in)	Depth (ft)	Vertical Rebar (in)	Rebar	Rebar Spacing (in)	X (in)	Y (in)	Rebar Size	Filllet Width (in)	Rebar Dimensions
36	14	8	6	30	36	14	8	6	30	36	14	8	6	30	12	47	6	1/4"	Rebar Dimensions
36	14	8	6	30	36	14	8	6	30	36	14	8	6	30	12	47	6	1/4"	Rebar Dimensions



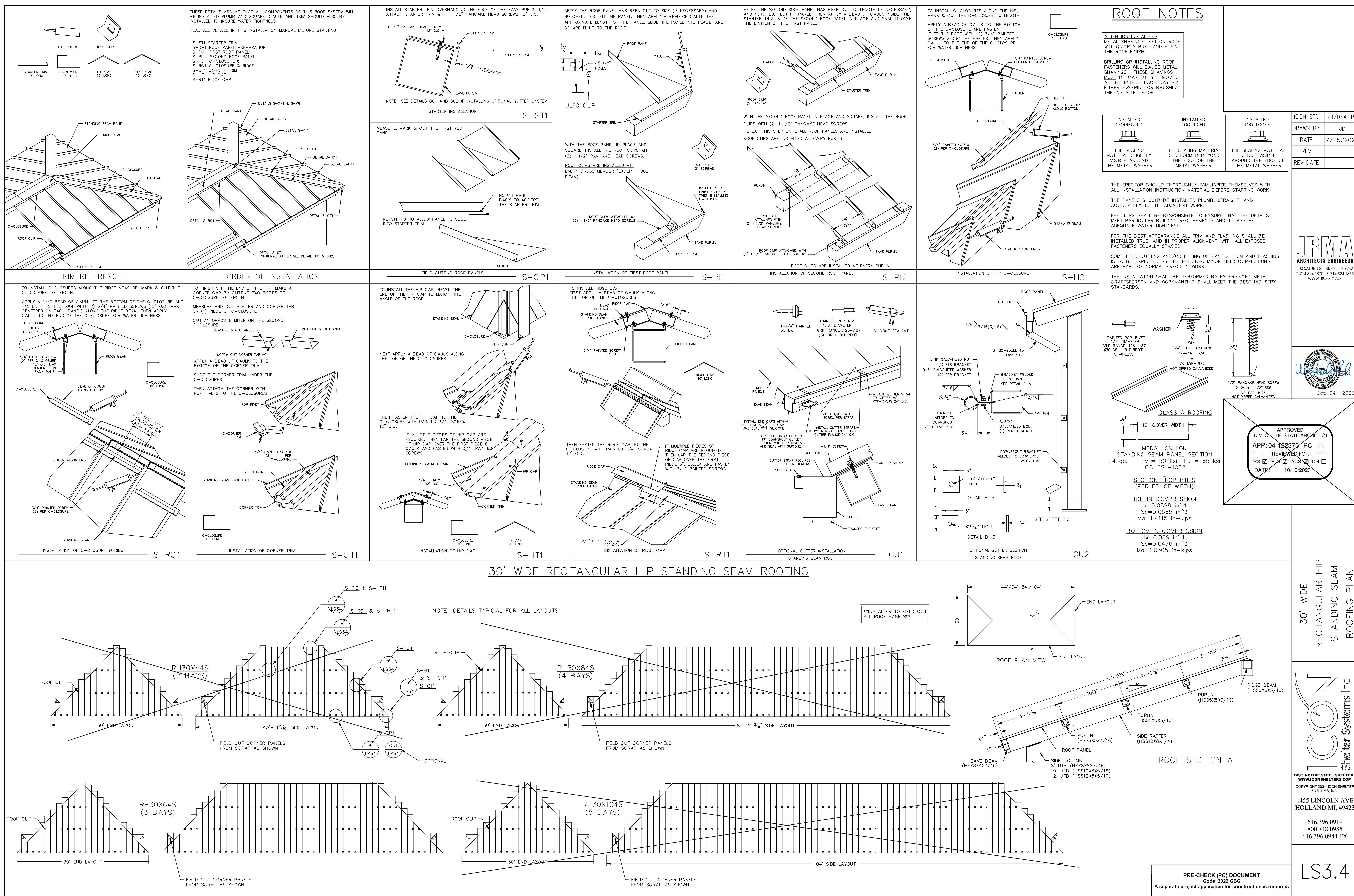
R300 - SPREAD										R300 - CORNER									
4' height - Corner Columns					8' height - Corner Columns					8' height - Corner Columns					8' height - Corner Columns				
Soil Class 5 - 1500 psf bearing					Soil Class 4 - 2000 psf bearing					Soil Class 3 - 3000 psf bearing					Soil Class 2 - 4000 psf bearing				
Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar
Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6
60	60	60	5	60	60	60	5	60	60	60	5	60	60	60	5	60	60	60	5
8' height - Side Columns					8' height - Side Columns					8' height - Side Columns					8' height - Side Columns				
Soil Class 5 - 1500 psf bearing					Soil Class 4 - 2000 psf bearing					Soil Class 3 - 3000 psf bearing					Soil Class 2 - 4000 psf bearing				
Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar
Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6
60	60	60	5	60	60	60	5	60	60	60	5	60	60	60	5	60	60	60	5
8' Eave - 1500 psf [ ]					8' Eave - 2000 psf [ ]					8' Eave - 3000 psf [ ]					8' Eave - 4000 psf [ ]				
10' height - Corner Columns					10' height - Corner Columns					10' height - Corner Columns					10' height - Corner Columns				
Soil Class 5 - 1500 psf bearing					Soil Class 4 - 2000 psf bearing					Soil Class 3 - 3000 psf bearing					Soil Class 2 - 4000 psf bearing				
Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar
Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6
60	60	60	5	60	60	60	5	60	60	60	5	60	60	60	5	60	60	60	5
10' height - Side Columns					10' height - Side Columns					10' height - Side Columns					10' height - Side Columns				
Soil Class 5 - 1500 psf bearing					Soil Class 4 - 2000 psf bearing					Soil Class 3 - 3000 psf bearing					Soil Class 2 - 4000 psf bearing				
Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar
Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6
81	81	81	5	81	81	81	5	81	81	81	5	81	81	81	5	81	81	81	5
10' Eave - 1500 psf [ ]					10' Eave - 2000 psf [ ]					10' Eave - 3000 psf [ ]					10' Eave - 4000 psf [ ]				
12' height - Corner Columns					12' height - Corner Columns					12' height - Corner Columns					12' height - Corner Columns				
Soil Class 5 - 1500 psf bearing					Soil Class 4 - 2000 psf bearing					Soil Class 3 - 3000 psf bearing					Soil Class 2 - 4000 psf bearing				
Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar
Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6
78	78	78	10	78	78	78	10	78	78	78	10	78	78	78	10	78	78	78	10
12' height - Side Columns					12' height - Side Columns					12' height - Side Columns					12' height - Side Columns				
Soil Class 5 - 1500 psf bearing					Soil Class 4 - 2000 psf bearing					Soil Class 3 - 3000 psf bearing					Soil Class 2 - 4000 psf bearing				
Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar	Size (in)	16	18	Rebar
Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6	Depth (in)	10	9	6
84	84	84	14	84	84	84	14	84	84	84	14	84	84	84	14	84	84	84	14
12' Eave - 1500 psf [ ]					12' Eave - 2000 psf [ ]					12' Eave - 3000 psf [ ]					12' Eave - 4000 psf [ ]				









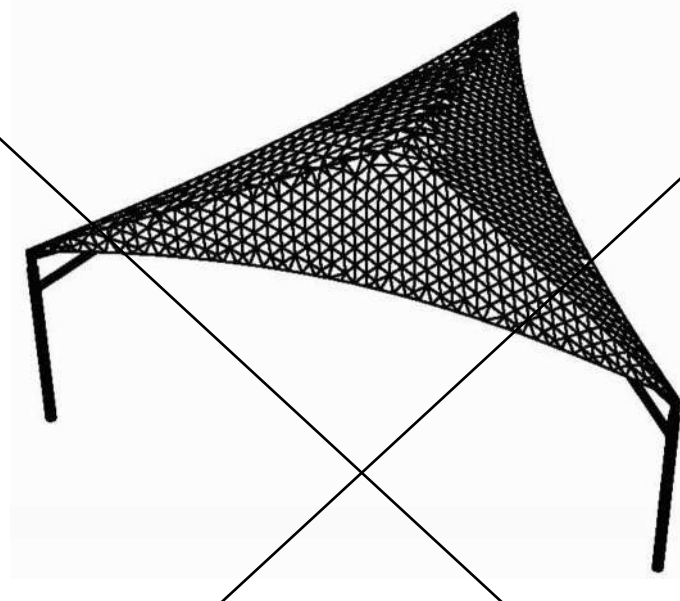
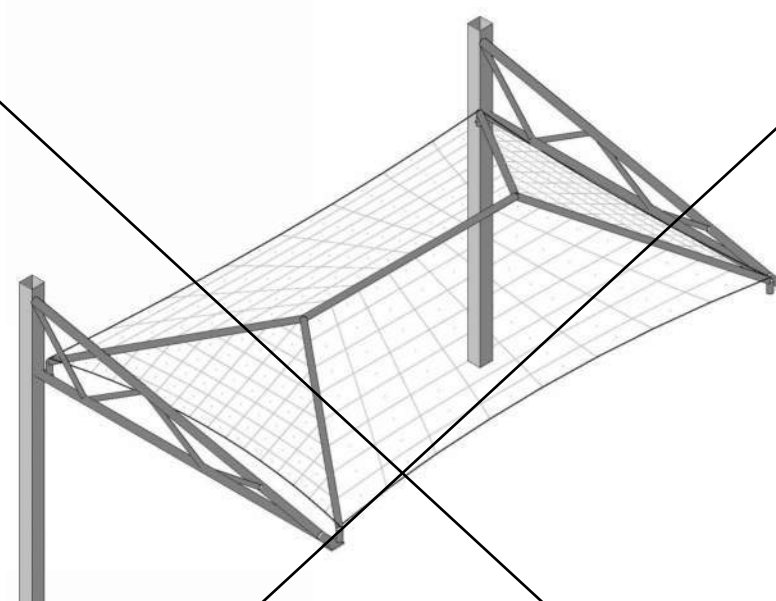
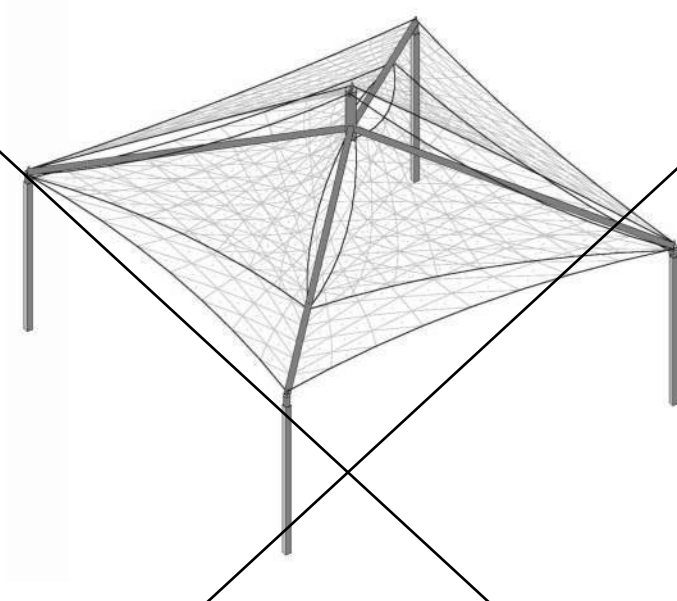
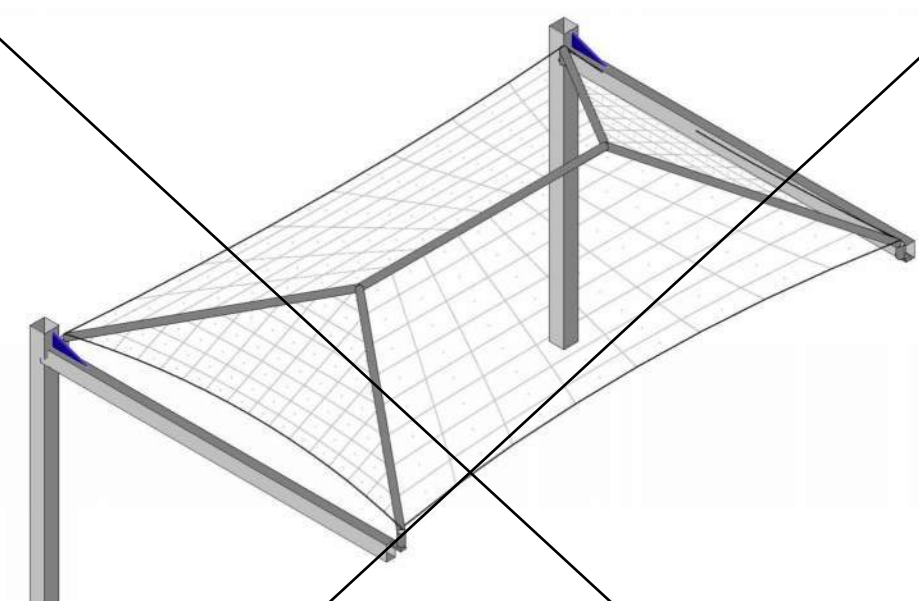
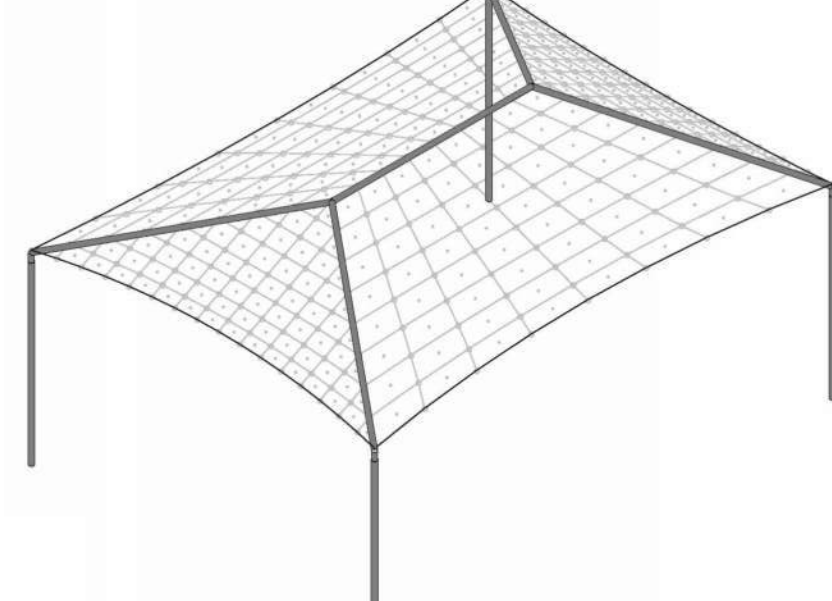

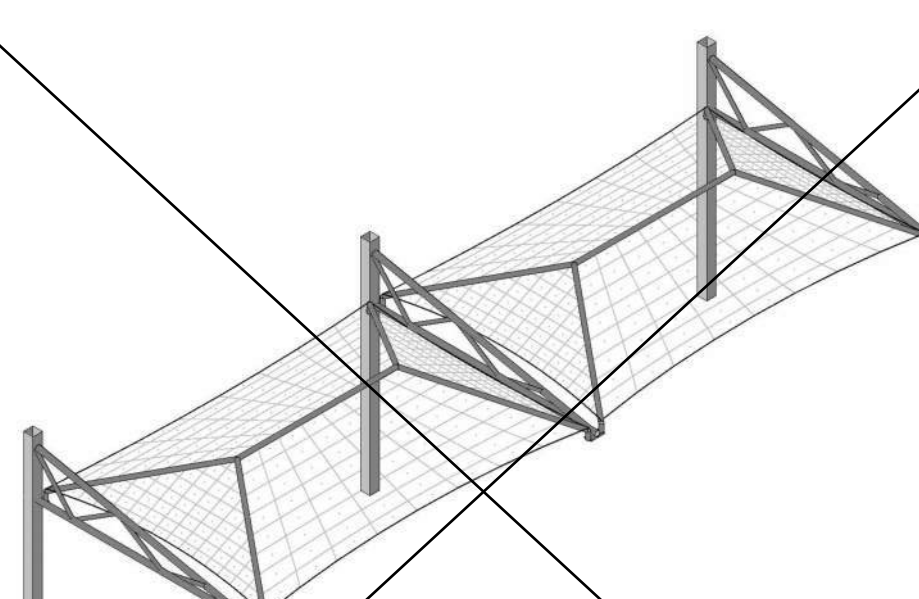
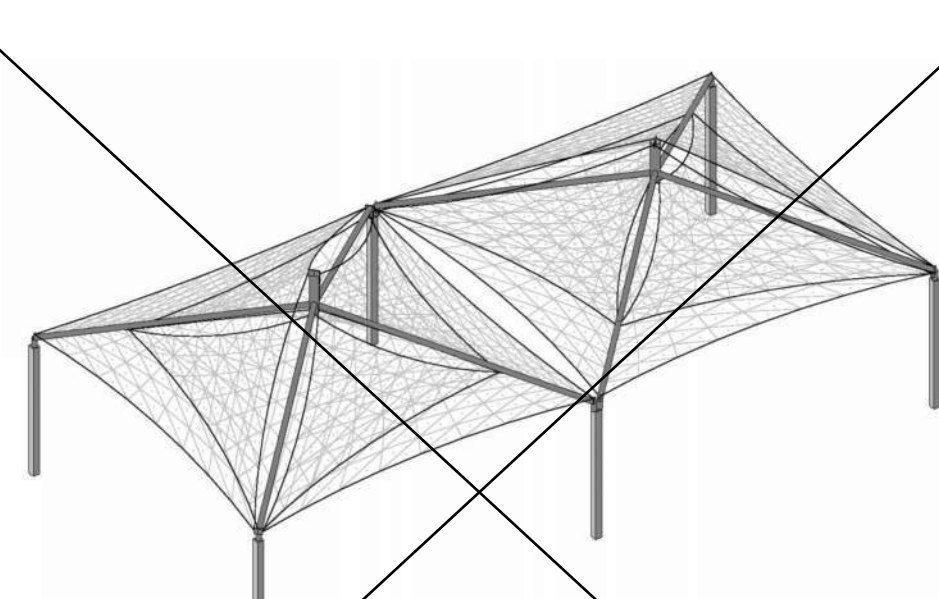
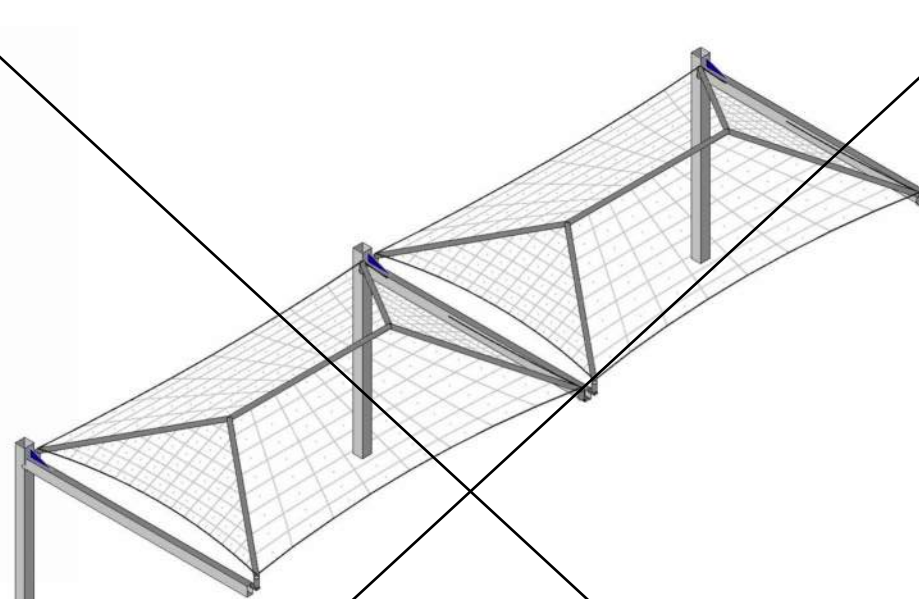
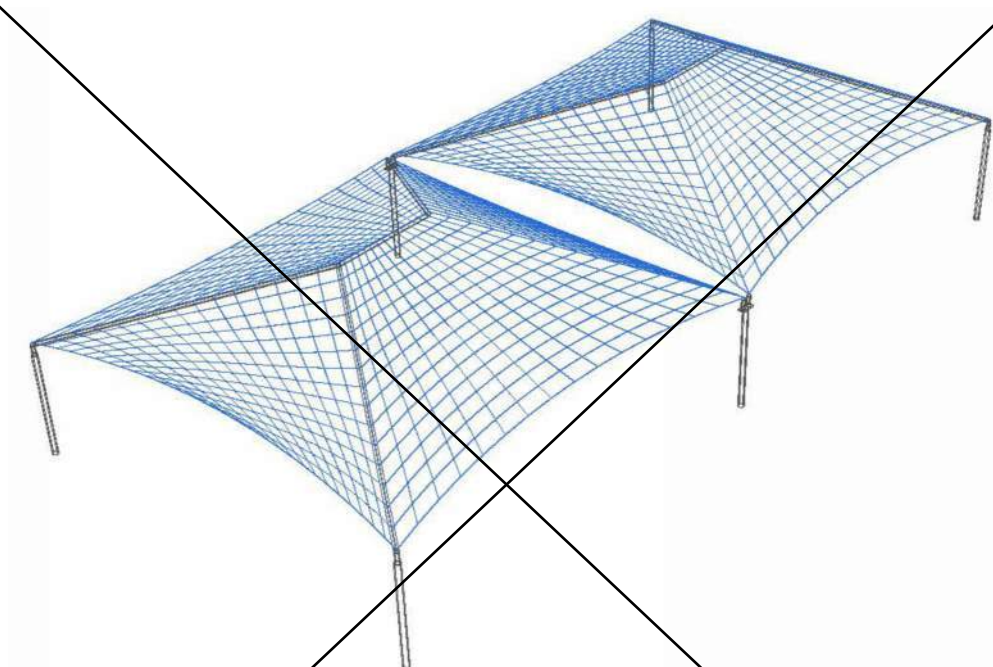
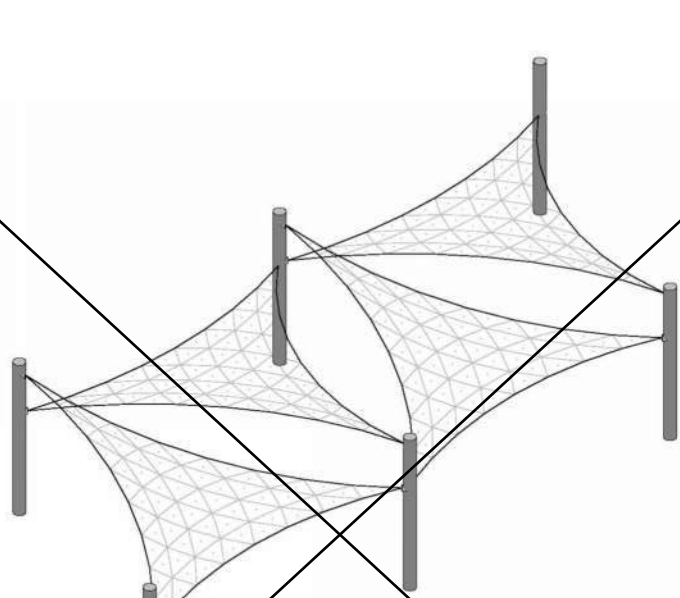
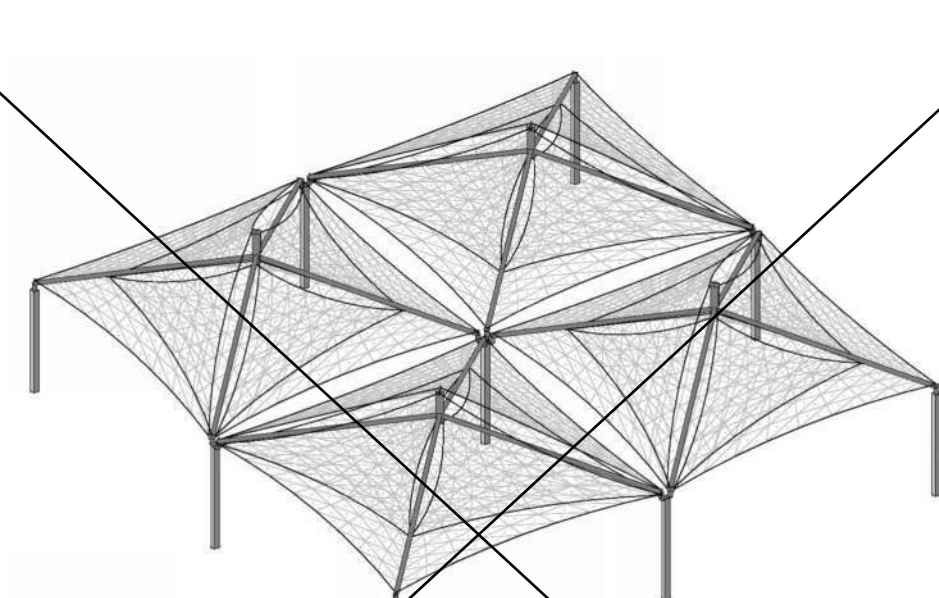
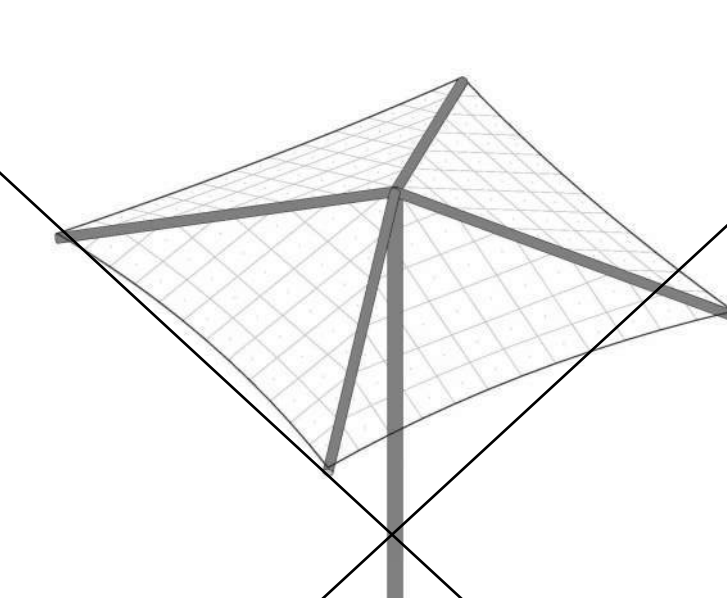
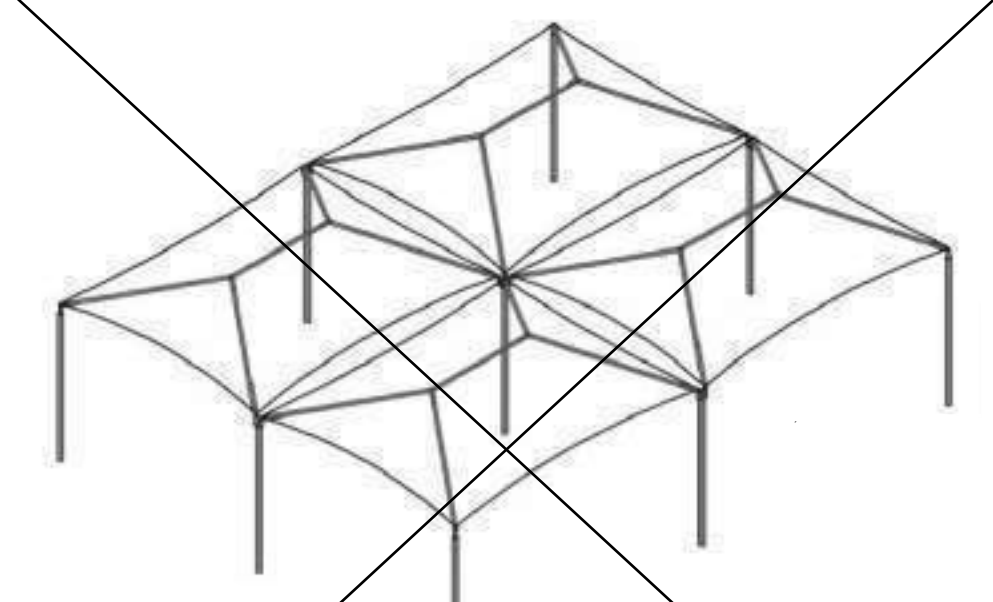
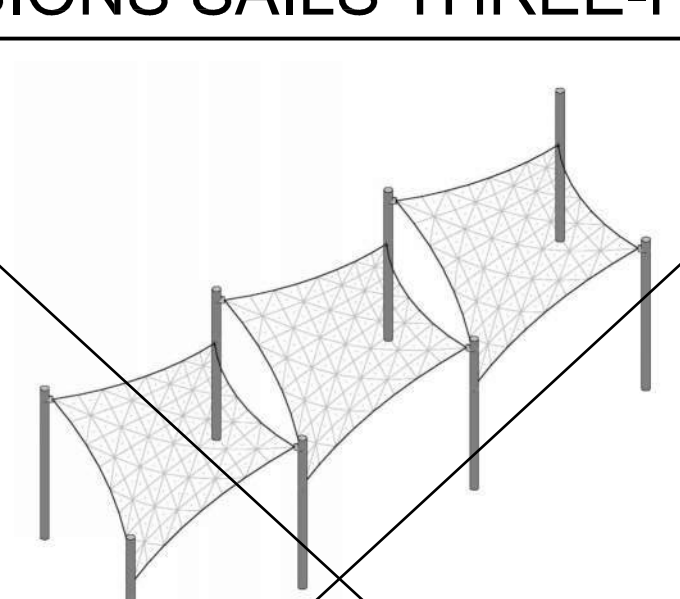
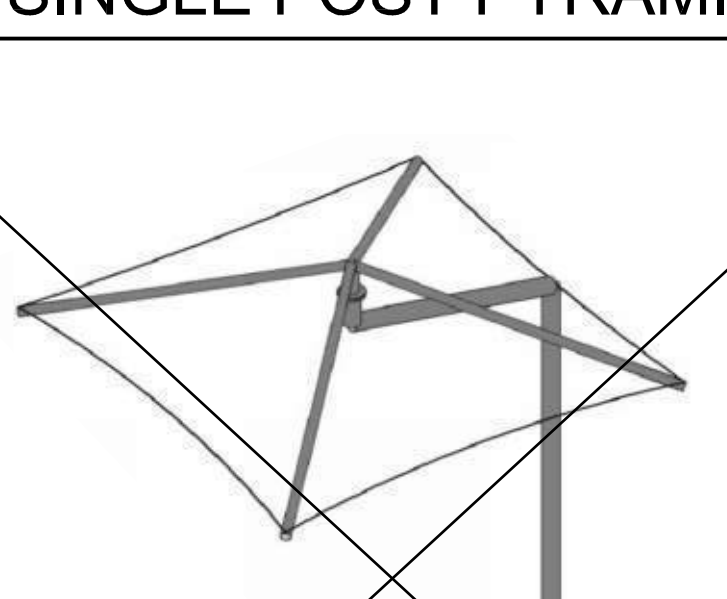

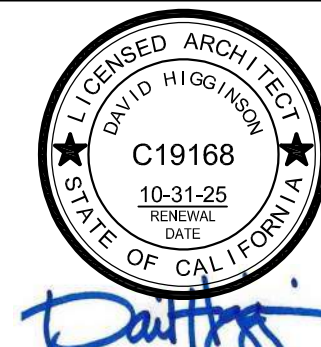




IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122273 INC:  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☒  
DATE: 05/03/2024

[illegible]



	<table><tr><td>STRUCTURE MODEL:</td><td>DSA0125-22</td><td>SEE SHEET 26.1-1000</td></tr><tr><td>MAX. SIZE:</td><td>29' x 29' x 19'</td><td></td></tr><tr><td>MAX. AREA:</td><td>271 SQ. FT.</td><td></td></tr><tr><td>MAX. OCCUPANCY:</td><td>16 PERSONS</td><td></td></tr></table> <table><tr><td>STRUCTURE MODEL:</td><td>DSA0140-22</td><td>SEE SHEET 27.1-1000</td></tr><tr><td>MAX. SIZE:</td><td>47' x 47' x 19'</td><td></td></tr><tr><td>MAX. AREA:</td><td>692 SQ. FT.</td><td></td></tr><tr><td>MAX. OCCUPANCY:</td><td>40 PERSONS</td><td></td></tr></table> <p>FOR DSA 103 TESTING &amp; INSPECTIONS SAMPLE, SEE PC T-3.0 &amp; PC T-4.0</p>	STRUCTURE MODEL:	DSA0125-22	SEE SHEET 26.1-1000	MAX. SIZE:	29' x 29' x 19'		MAX. AREA:	271 SQ. FT.		MAX. OCCUPANCY:	16 PERSONS		STRUCTURE MODEL:	DSA0140-22	SEE SHEET 27.1-1000	MAX. SIZE:	47' x 47' x 19'		MAX. AREA:	692 SQ. FT.		MAX. OCCUPANCY:	40 PERSONS			<table><tr><td>STRUCTURE MODEL:</td><td>DSA02030-22</td><td>SEE SHEET 21.1-1000</td></tr><tr><td>MAX. 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OCCUPANCY:</td><td>100 PERSONS</td><td></td></tr></table> <table><tr><td>STRUCTURE MODEL:</td><td>DSA0152030-22 (20 psf SNOW LOAD)</td><td>SEE SHEET 8.1-1000</td></tr><tr><td>MAX. SIZE:</td><td>27' x 30' x 19'</td><td></td></tr><tr><td>MAX. AREA:</td><td>693 SQ. FT.</td><td></td></tr><tr><td>MAX. OCCUPANCY:</td><td>40 PERSONS</td><td></td></tr></table> <p>FOR DSA 103 TESTING &amp; INSPECTIONS SAMPLE, SEE PC T-3.0 &amp; PC T-4.0</p>	STRUCTURE MODEL:	DSA012030-22	SEE SHEET 1.1-1000	MAX. SIZE:	27' x 30' x 19'		MAX. AREA:	693 SQ. FT.		MAX. OCCUPANCY:	40 PERSONS		STRUCTURE MODEL:	DSA013030-22	SEE SHEET 2.1-1000	MAX. SIZE:	39' x 30' x 19'		MAX. AREA:	693 SQ. FT.		MAX. OCCUPANCY:	40 PERSONS		STRUCTURE MODEL:	DSA013040-22	SEE SHEET 3.1-1000	MAX. SIZE:	39' x 42' x 19'		MAX. AREA:	1,200 SQ. FT.		MAX. OCCUPANCY:	60 PERSONS		STRUCTURE MODEL:	DSA01303012-22	SEE SHEET 4.1-1000	MAX. SIZE:	29' x 30' x 12'		MAX. AREA:	693 SQ. FT.		MAX. OCCUPANCY:	40 PERSONS		STRUCTURE MODEL:	DSA01303012-22	SEE SHEET 5.1-1000	MAX. SIZE:	39' x 30' x 12'		MAX. AREA:	693 SQ. FT.		MAX. OCCUPANCY:	40 PERSONS		STRUCTURE MODEL:	DSA01304012-22	SEE SHEET 6.1-1000	MAX. SIZE:	39' x 42' x 19'		MAX. AREA:	1,200 SQ. FT.		MAX. OCCUPANCY:	60 PERSONS		STRUCTURE MODEL:	DSA014040-22	SEE SHEET 7.1-1000	MAX. SIZE:	47' x 47' x 19'		MAX. AREA:	1,000 SQ. FT.		MAX. OCCUPANCY:	100 PERSONS		STRUCTURE MODEL:	DSA0152030-22 (20 psf SNOW LOAD)	SEE SHEET 8.1-1000	MAX. SIZE:	27' x 30' x 19'		MAX. AREA:	693 SQ. FT.		MAX. OCCUPANCY:	40 PERSONS	
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THESE PLANS AND SPECIFICATIONS ARE THE PROPERTY OF USA SHADE AND FABRIC STRUCTURES AND SHALL NOT BE REPRODUCED WITHOUT THEIR WRITTEN

**USASHADE & Fabric Structures**

**CORPORATE HEADQUARTERS**  
2580 ESTERS BLVD, SUITE 100  
DFW AIRPORT, TX, 75261  
800-966-5005

**CERTIFICATIONS:**  
IAS CERTIFICATION No: FA-428  
CLARK COUNTY MANUFACTURER  
CERTIFICATION NUMBER (NEVADA): 355

**CUSTOMER:**  
Washington U.S.D.

**PROJECT NAME:**  
Riverbank Elementary School

**LOCATION:**  
1100 Carrie Street  
West Sacramento, CA 95605

**MODEL NUMBER:**

**STRUCTURE TYPE:**

**SCALE : VARIES**

**DRAWING SIZE:**  
**D**

**PRE-CHECK (PC) DOCUMENT**  
Code : 2022 CBC  
A separate project application for construction is required.

Eng. By :	DWH	2/14/23
Design By :	DWH	2/14/23
Approved By :	DWH	2/14/23

**DRAWING DESCRIPTION:**

DWG. UNIT SELECTION
SHEET <b>T-2.0</b>
REV.







#### GENERAL NOTES

- 1.- SPECIAL INSPECTION REQUIREMENTS SHALL FOLLOW THE ATTACHED SAMPLE TEST AND INSPECTION LIST (I & LIST) APPROVED BY DSA. THE SHOP WELDING INSPECTION SHALL INCLUDE WELDING OF ALL STEEL MEMBERS AND IDENTIFICATION OF STEEL THROUGH MILL CERTIFICATE OR MATERIAL TESTING. UNCERTIFIED STEEL SHALL BE TESTED TO THE REQUIREMENTS OF CBC 2022 CHAPTER 17A. THE FIELD SPECIAL INSPECTION SHALL INCLUDE COMPRESSION CYLINDER TESTS FOR THE CONCRETE FOUNDATION.
- 2.- STRUCTURE SHALL BE IN THE LOCATION SHOWN ON THE SITE SPECIFIC DSA APPLICATION DRAWING.
- 3.- FOUNDATION DESIGN BASED ON CBC 2022, TABLE 1806A.2, SOIL CLASS 5 (ALLOWABLE FOUNDATION PRESSURE 1500 PSF)
- 4.- DESIGN PER FOLLOWING CODES: CBC 2022(CHAPTER 35), ASCE 7-16, AISC 360-16, AISC 341-16, ACI 318-19, ASCE 55-16 & ASCE 19-16

#### STRUCTURAL STEEL

- 1.- FABRICATION OF THE STEEL STRUCTURES SHALL BE PERFORMED BY SHADE STRUCTURES OR AN AUTHORIZED LICENSEE. MATERIAL TESTING (OR MILL CERTIFICATES) AND INSPECTION OF WELDING SHALL BE CONDUCTED PER CBC 2022 SECTIONS 1704A, 1705A, 1705A.2, AND TABLE 1705A.2.1.
- 2.- ONLY CALIFORNIA LICENSED CONTRACTORS AUTHORIZED BY SHADE STRUCTURES SHALL INSTALL THE SHADE STRUCTURES.
- 3.- ALL WORK SHALL CONFORM TO CBC 2022 EDITION, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR)
- 4.- ALL GALVANIZED STEEL TUBE PRODUCTS MANUFACTURED BY ALLIED TUBE & CONDUIT FOR THIS STRUCTURE SHALL BE AND CONFORM TO ASTM A500-16 GRADE C, IN ITS ENTIRETY.  
TYPICAL MECHANICAL PROPERTIES ARE:  
ROUND TUBE GRADE C 46,000 PSI YIELD STRESS MINIMUM / 62,000 PSI TENSILE STRESS MINIMUM
- 5.- ALL STRUCTURAL SHAPES SHALL BE COLD FORMED HSS ASTM A500 GRADE C, UNLESS OTHERWISE NOTED. TYPICAL MECHANICAL PROPERTIES ACHIEVED FOR HSS PRODUCTS:  
SQUARE AND RECTANGULAR 50,000 PSI YIELD STRESS / 62,000 PSI TENSILE STRESS  
ROUND PIPE 50,000 PSI YIELD STRESS / 62,000 PSI TENSILE STRESS
- 6.- ALL PLATES PRODUCTS SHALL COMPLY WITH ASTM A572 GRADE 50.
- 7.- STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH A.I.S.C. SPECIFICATIONS.
- 8.- ALL WELDING TO CONFORM WITH AMERICAN WELDING SOCIETY STANDARDS AND SHALL BE INSPECTED BY AN AWS/COWI INSPECTOR. AWS D1.1 FOR HOT ROLLED. AWS D1.3 FOR SHEET/COLD FORMED. AWS D1.8 SEISMIC SUPPLEMENT.
- 9.- ALL FULL PENETRATION WELD SHALL BE CONTINUOUSLY INSPECTED PER AWS D1.1 & D1.8.
- 10.- SHOP CONNECTIONS SHALL BE WELDED UNLESS NOTED OTHERWISE. ALL FILLET WELDS SHALL BE A MINIMUM OF 3/16" ER70S-X ELECTRODES UNLESS OTHERWISE NOTED. GMAW IS ACCEPTABLE.
- 11.- ALL STAINLESS STEEL BOLTS SHALL COMPLY WITH ASTM F-959, YIELD STRENGTH= 65 KSI, TENSILE STRENGTH=100 KSI MINIMUM. ALLOY GROUP 2, CONDITION CW1. REFER TO RCSC, ASTM F-959 IS NOT CONSIDERED AS HIGH STRENGTH BOLTS. BOLTS SHALL BE TIGHTENED TO A SNUG TIGHT CONDITION (ST).
- 12.- ALL STRUCTURAL STEEL (ITEMS FROM NOTE 5) SHALL BE POWDER COATED WITH ONE SHOP COAT (2.5 MILS MIN.) OF ZINC-RICH PRIMER, UNDERCOAT, AND FINISH COAT, OR EQUIVALENT PAINT SYSTEM. THIS COAT IS A WEATHER RESISTANT POWDER COATING BASED ON POLYESTER TGIC (MANUFACTURED BY SHERWIN WILLIAMS, ASKO NOBEL, PPG OR TIGER DRYLAC), TO ACHIEVE OPTIMUM ADHESION. IT IS RECOMMENDED THAT THE PROPER TREATMENT AND DRYING TAKE PLACE BEFORE COATING. POLYESTER POWDER (TGIC) SPECIFICATIONS SHALL BE AS FOLLOWS:  
- PENCIL HARDNESS (ASTM D-3363) - HUMIDITY (ASTM D-2247)  
- SOLVENT RESISTANCE (PCI METHOD) - 50 DBL RUBS SL, SOFTNESS.
- 13.- ALL STEEL ROUND TUBING (ITEMS FROM NOTE 4) SHALL BE TRIPLE COATED FOR RUST PROTECTION USING THE IN-LINE ELECTROPLATING COAT PROCESS. TUBING SHALL BE INTERNALLY COATED WITH ZINC AND ORGANIC COATINGS TO PREVENT CORROSION AS MANUFACTURED BY ALLIED TUBE & CONDUIT.
- 14.- ALL EXPOSED STEEL FASTENERS SHALL BE STAINLESS STEEL (TYPE 304 MINIMUM), HOT DIP GALVANIZED (ASTM A153, CLASS D MINIMUM OR ASTM F2329) AS APPLICABLE, OR PROTECTED WITH CORROSION PREVENTIVE COATING THAT DEMONSTRATED NO MORE THAN 2% OF RED RUST IN MINIMUM 1,000 HOURS OF EXPOSURE IN SALT SPRAY TEST PER ASTM B117. ZINC-PLATED FASTENERS DO NOT COMPLY WITH THIS REQUIREMENT.

#### CONCRETE SPECIFICATION

- 1.- CONCRETE SHALL BE SAMPLED AND TESTED PER CBC 2022 SECTION 1903A & SHALL BE INSPECTED PER SECTION 1903A.
- 2.- CONCRETE TO BE F<sub>cy</sub> 4500 PSI, TYPE V CEMENT PLUS POZZOLAN OR SLAG CEMENT, MAXIMUM WATER/CEMENT RATIO OF 0.45, PER ACI 318-19 CHAPTER 19, (NO ADMIXTURES CONTAINING CALCIUM CHLORIDE WILL BE USED.) REINFORCING STEEL SHALL CONFORM TO ASTM A-615 GRADE 60 AND TO BE F<sub>y</sub> 60,000 PSI, MIN. GR. 60, ALSO COATED ACCORDING TO ASTM A117. STANDARD SPECIFICATION FOR ZINC-COATING (GALVANIZED) STEEL BARS FOR CONCRETE REINFORCEMENT.
- 3.- ALL ANCHOR BOLTS SET IN NEW CONCRETE (WHEN APPLICABLE) SHALL COMPLY WITH ASTM F-1554 GRADE 36 (GALVANIZED PER ASTM A153, CLASS D MINIMUM OR ASTM F2329). ANCHOR BOLT'S DIAMETER NEEDS TO BE AS FOLLOWS:  
A) ANCHOR BOLT Ø1 1/4"
- 4.- CERTIFIED MILL TEST REPORTS ARE TO BE PROVIDED FOR EACH SHIPMENT OF REINFORCEMENT.
- 5.- ALL NON-SHRINK GROUT SHALL HAVE A MINIMUM 28 DAYS COMPRESSIVE STRENGTH OF 5000 PSI, AND SHALL COMPLY THE REQUIREMENTS OF ASTM C109, ASTM C939, ASTM C1090, ASTM C1107, WHEN APPLICABLE.
- 6.- CONCRETE EXPOSED TO FREEZING-AND-THAWING CYCLES SHALL BE AIR ENTRAINED PER ACI 318 SECTION 19.3.3.

#### FABRIC SPECIFICATION

- 1.- FABRIC SHALL BE MANUFACTURED BY MULTIKNIT LTD., WHICH MEETS THE SPECIFICATIONS LISTED ON PAGE 2000, AND SHALL BE FABRICATED FROM POLYETHYLENE MATERIALS. MINIMUM SEAM LENGTH 3/4".
- 2.- THE FABRIC SHALL RETAIN 80% OF ITS TENSILE AND TEARING STRENGTH AFTER ULTRAVIOLET EXPOSURE PER ASTM G53 USING A 313 NM LIGHT SOURCE FOR 500 HOURS WHILE MOISTENED FOR 1 HOUR EVERY 12 HOURS.
- 3.- PROVIDE CERTIFICATION BY MANUFACTURER AND STATE FIRE MARSHAL TO SCHOOL'S DISTRICT INSPECTOR OF RECORD AT SITE SPECIFIC INSTALLATION. COPY OF FIRE CERTIFICATION SHALL BE SENT TO DSA.
- 4.- FABRIC SHALL REQUIRE ANNUAL INSPECTION AND MAINTENANCE BY THE DISTRICT FIRE TEST ON FABRIC. NFPA 701 TEST 2 AND ASTM E 84 EXTENDED 30 MINUTES TEST. FLAME SPREAD INDEX (FSI): 10, SMOKE DEVELOPED INDEX (SDI): 50. FABRIC IS ACCEPTABLE FOR USE IN WILDLIFE URBAN INTERFACE AREA.
- 5.- FABRIC TOP NEEDS TO BE REMOVED IF SNOW EXCEEDING 5 PSF ARE ANTICIPATED, FABRIC TOP NEEDS TO BE REMOVED IF WINDS EXCEEDING 115 MPH ARE ANTICIPATED.
- 6.- A VISUAL INSPECTION LOOKING FOR TEAR AND ABNORMAL WEAR IN FABRIC MATERIAL AND THREAD IS REQUIRED PRIOR TO RE-INSTALLATION. USA SHADE & FABRIC STRUCTURES SHALL BE NOTIFIED IF SIGNIFICANT DAMAGE IS PRESENT BEFORE RE-INSTALLATION.

#### AIRCRAFT DAMAGE

- 1.- FOR FABRIC ATTACHMENT USE 3/8" 7x19 GALV. CABLE PER ASTM A1023/A1023M, WITH A BREAKING STRENGTH VALUE OF 14,400 LBS. CABLE SHALL BE TENSIONED TO 300 LBS MINIMUM AND 500 LBS MAXIMUM. THE MAXIMUM CALCULATED CABLE ALLOWABLE CAPACITY IS 594x4909 LB.
- 2.- CABLES SHALL BE FED THROUGH THE FABRIC SLEEVES AROUND THE PERIMETER OF THE CANOPY AND TENSIONED UNTIL THE FABRIC PANELS (DESIGNED PURPOSELY UNDERSIZED) REACH A TAUT APPEARANCE. ANY LONG TERM CABLE SAG SHALL BE MINIMIZED DURING THE MAINTENANCE RE-TIGHTING VISITS AS REQUIRED.

MAXIMUM OCCUPANT LOAD (PER CBC 2022 TABLE 1604A.5)  
-K-12: 250 PERSONS  
-PUBLIC ASSEMBLY: 300 PERSONS  
-EDUCATIONAL OCCUPANCIES  
ABOVE 12TH GRADE: 500 PERSONS

#### CBC PC DESIGN NOTES

BUILDING CODE	CBC 2022 (BASED ON IBC 2021)
FLOOR LIVE LOAD	N/A
ROOF LIVE LOAD	5 PSF
ALLOWABLE SOIL PRESSURE:	
DL + LL (CONC FTG)	1500 PSF
DL + LL + SEISMIC (CONC FTG)	1500 PSF
LATERAL BEARING DESIGN VALUE	100 PSF/FT BELOW NATURAL GRADE, PER TABLE 1806A.2
TWO TIMES THE TABULAR VALUE IS USED (200 PSF/FT)	
PER CBC SECTION 1806A.3.4	
ALLOWABLE PIER FRICTIONAL RESISTANCE 250 PSF MAXIMUM	
BASED ON SECTION 1810A.3.3.1.4 (ONE-SIXTH OF THE BEARING VALUE).	
UPLIFT FRICTIONAL RESISTANCE HAVE A SAFETY FACTOR OF 3.	

ROOF SNOW LOAD	5 PSF
ICE LOAD	ZERO PSF
FLOOD HAZARD AREA	ZONE X
WHEN A SITE SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEER IS NEEDED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED IN THE PC ARE STILL APPLICABLE.	
WIND DESIGN DIRECTIONAL PROCEDURE: ASCE 7-16, SECTION 27.3.2	
NOTE: WIND DESIGN IS LIMITED TO UNOBSTRUCTED CLEAR FLOW CONDITION	
-BASIC DESIGN WIND SPEED (3 SEC GUST)	V 115 MPH
-ASD WIND LOAD (CBC 2022 SEC. 1603A.1.4)	V <sub>ASD</sub> 90 MPH
-WIND EXPOSURE FACTOR	C 1.25
-TOPOGRAPHIC FACTOR	K <sub>zt</sub> 1
-RISK CATEGORY	II
-VELOCITY PRESSURE EXPOSURE COEFFICIENT	K <sub>c</sub> 0.85
-VELOCITY PRESSURE	q <sub>z</sub> 24.46 PSF
SEISMIC DESIGN:	
-SITE CLASS	D
NOTE: UNLESS A SITE-SPECIFIC GROUND MOTION ANALYSIS IS PERFORMED, THE S <sub>m1</sub> VALUE INCREASED BY 50% SHALL BE LESS THAN THE DESIGN CRITERIA STATED HEREIN.	
-SPECTRAL RESPONSE COEFFICIENTS	S <sub>DS</sub> 3.00g S <sub>1</sub> 1.389g SD1 1.39
-LATERAL FORCE RESISTING SYSTEM G-2 ORDINARY CANTILEVERED COLUMN SYSTEM.	
-SEISMIC IMPORTANCE FACTOR	I <sub>e</sub> 1.0
-DESIGN BASE SHEAR AT BASE	V 3072 LB
-SEISMIC RESPONSE COEFFICIENTS	C <sub>s</sub> 1.6
-RESPONSE MODIFICATION FACTOR	R 1.25
-ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE
-RISK CATEGORY	II
-SEISMIC DESIGN CATEGORY	E
-SITE COEFFICIENT CATEGORY	F <sub>a</sub> 1.2 F <sub>v</sub> 1.5 ρ 1.3
-REDUNDANCY FACTOR	

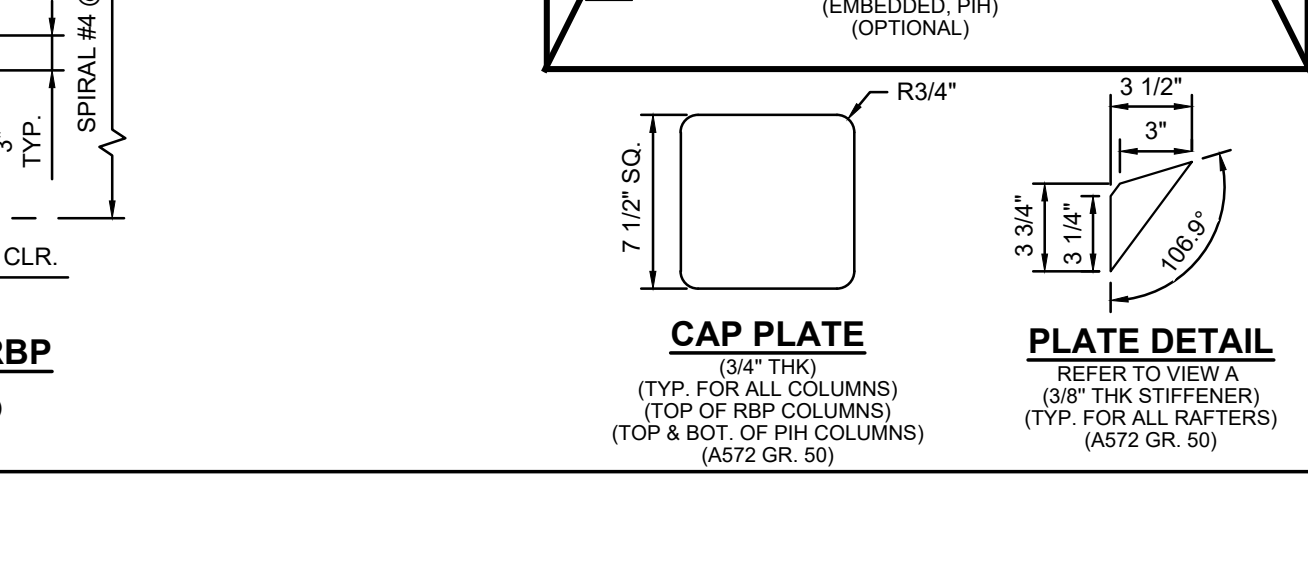
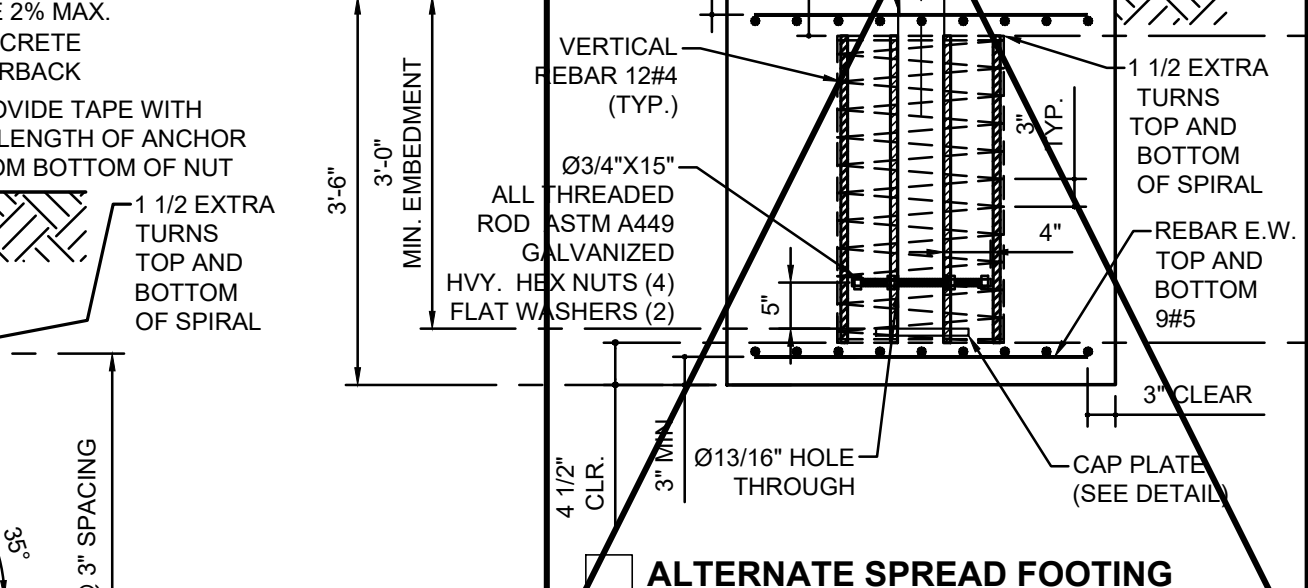
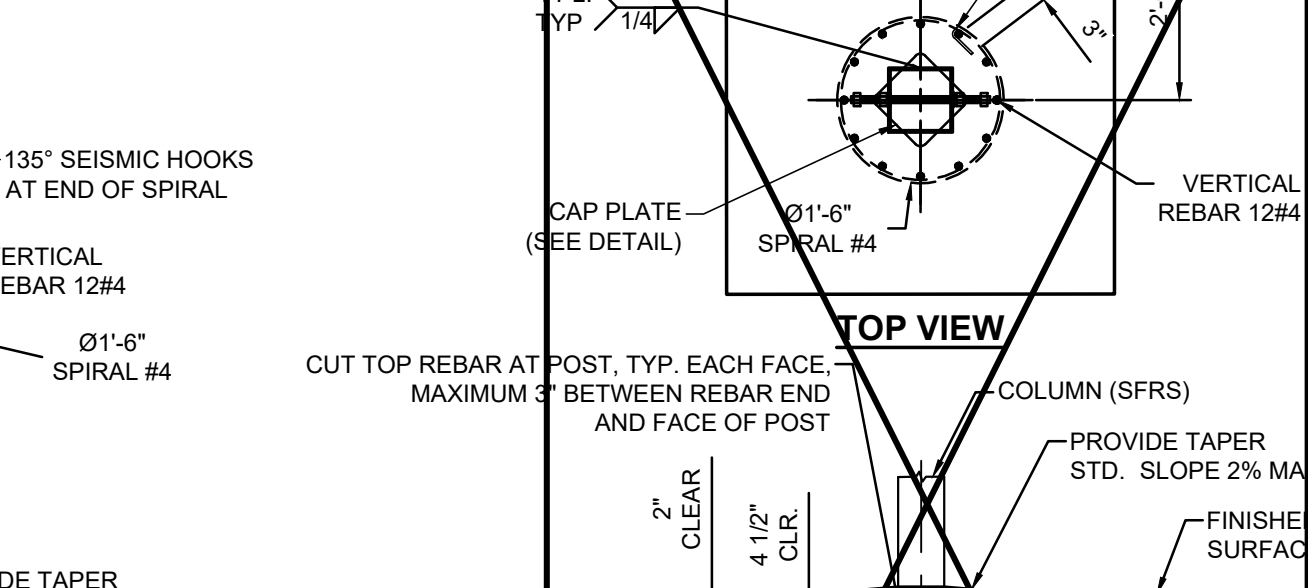
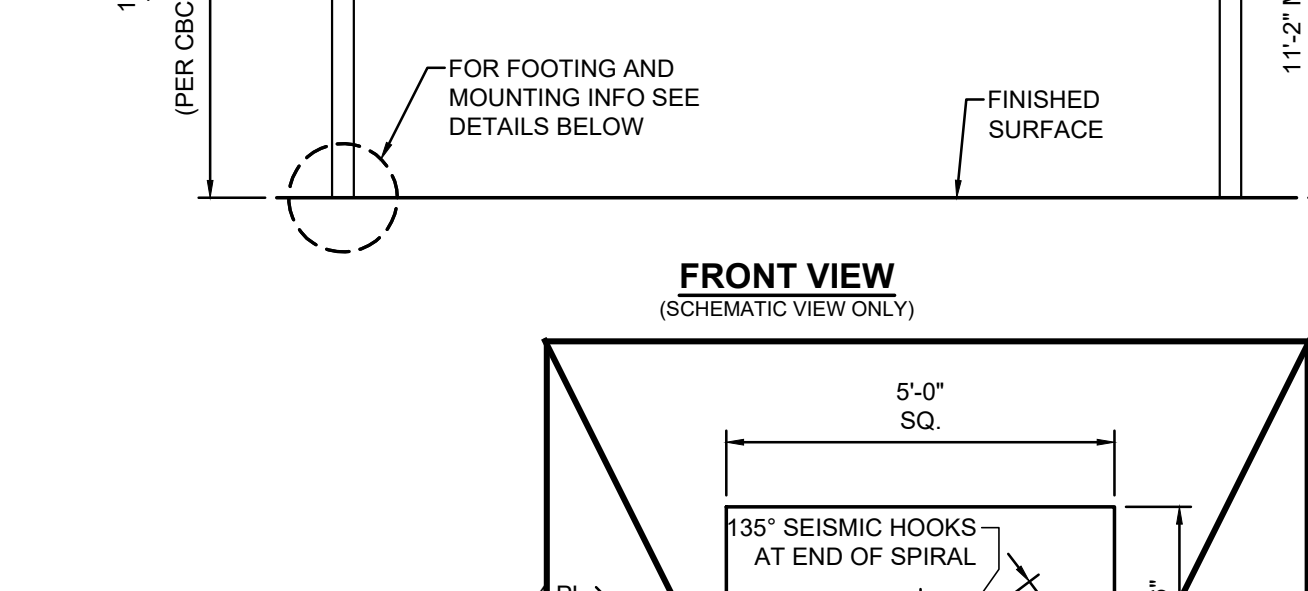
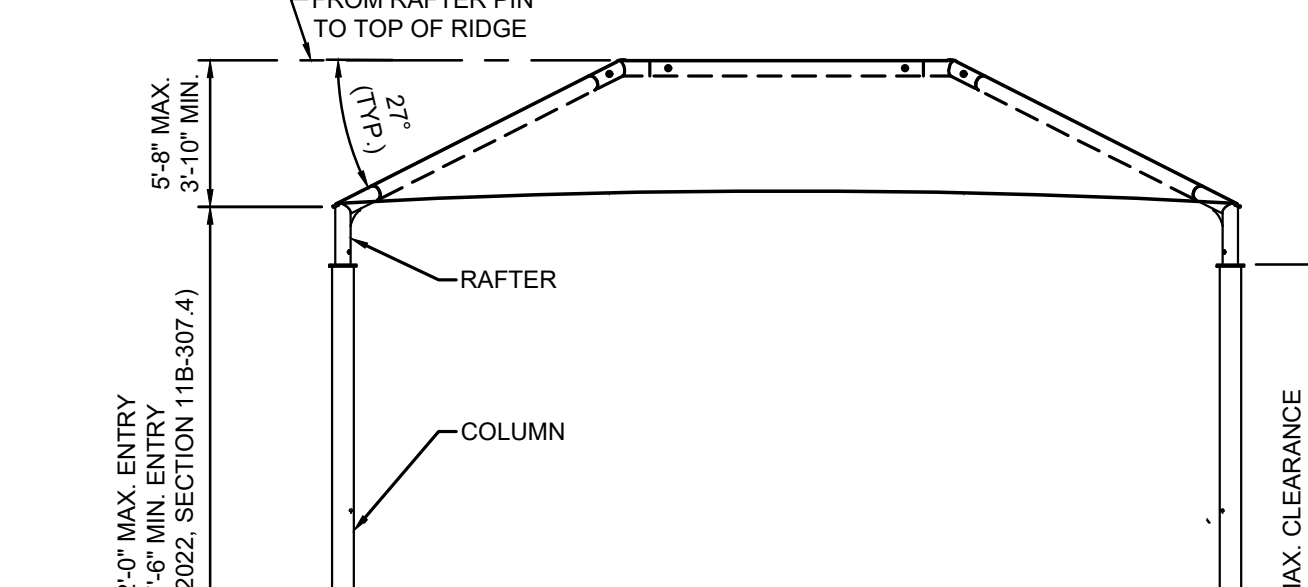
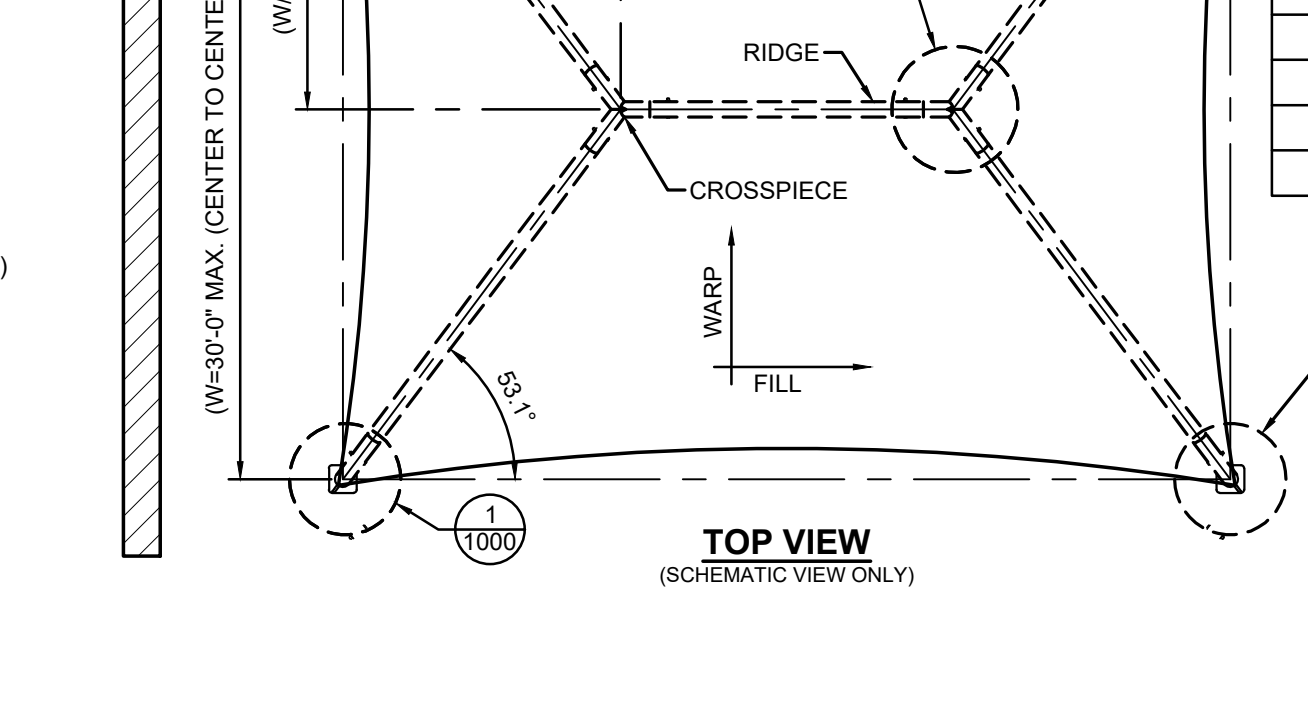
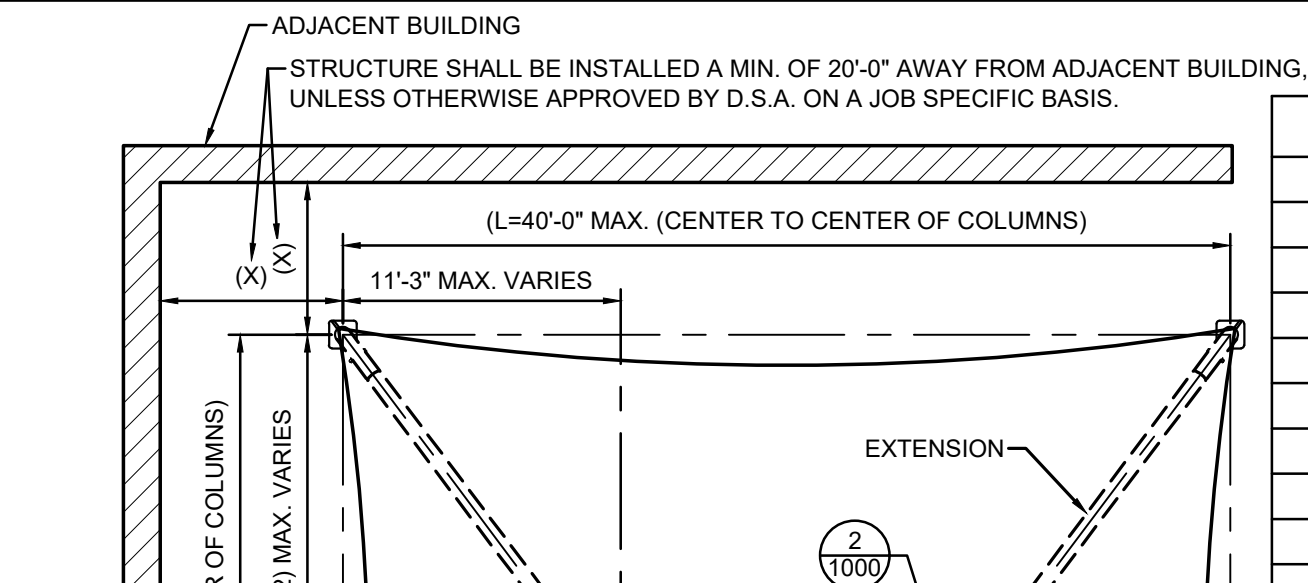
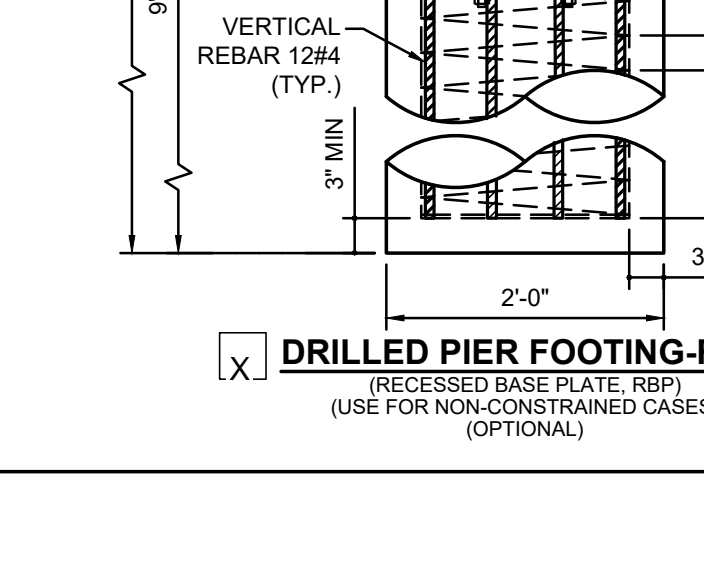
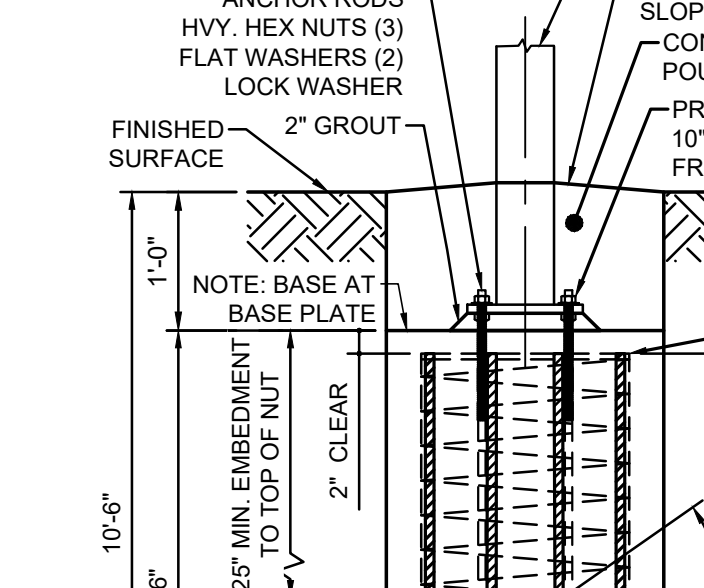
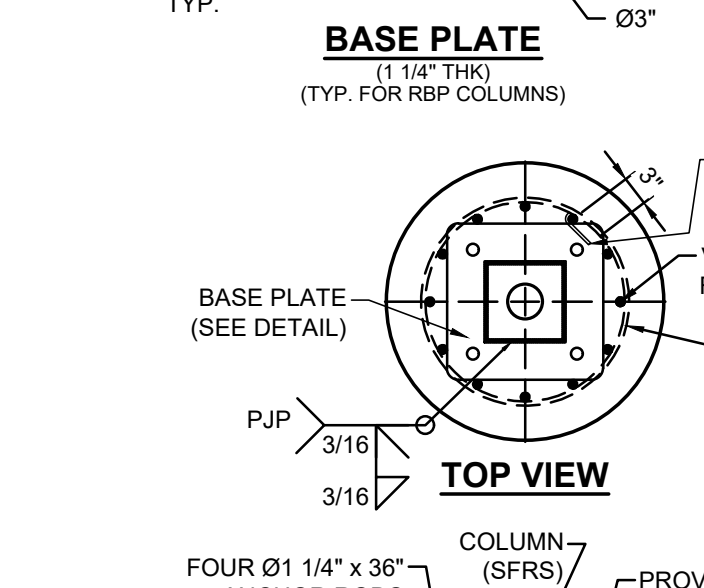
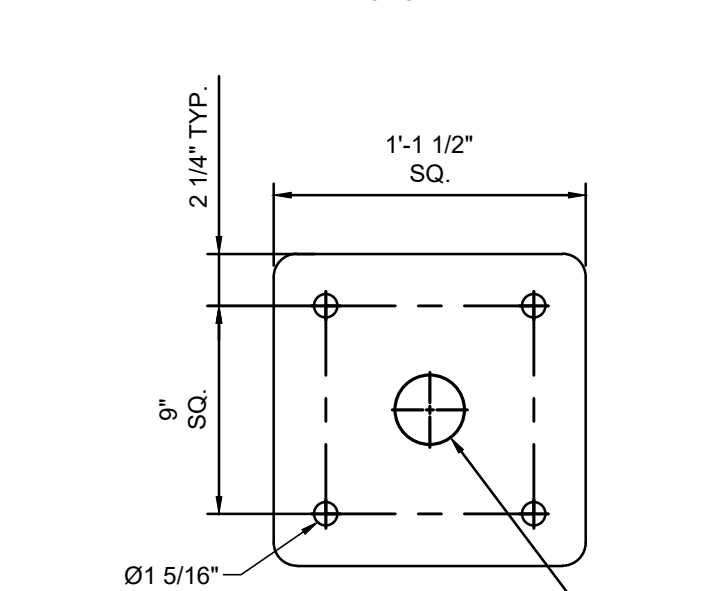
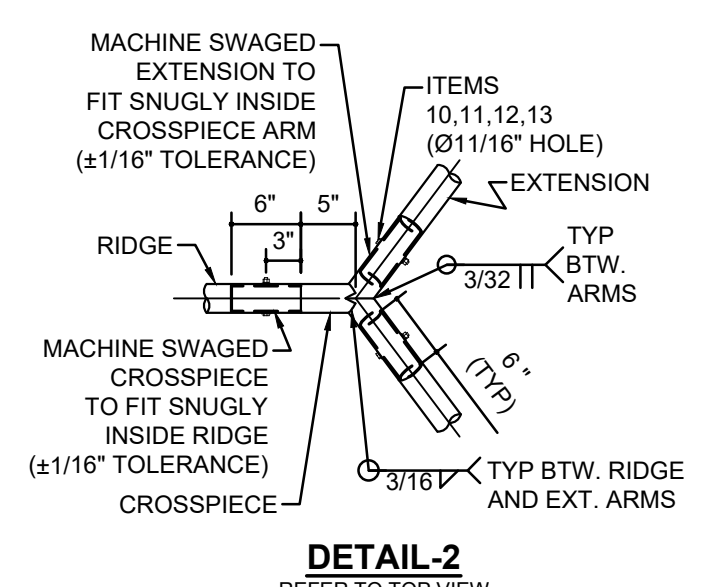
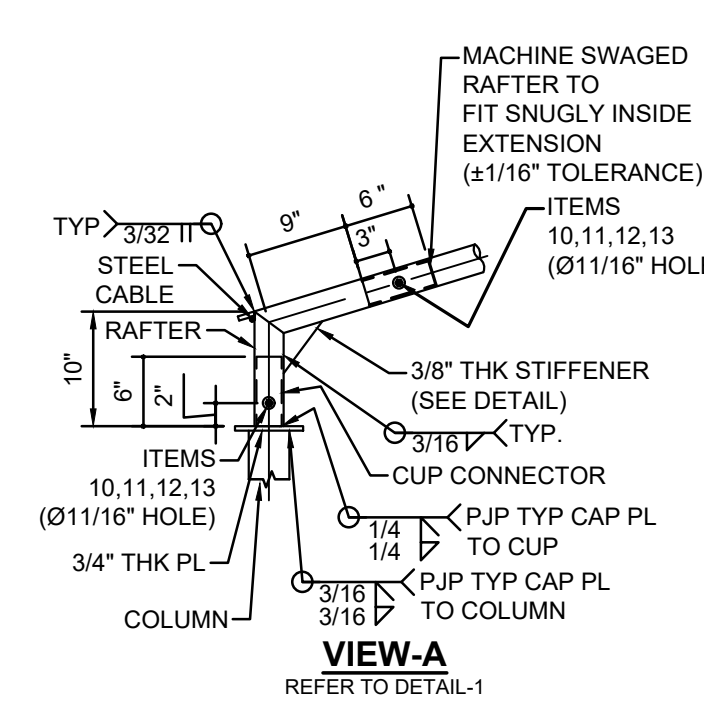
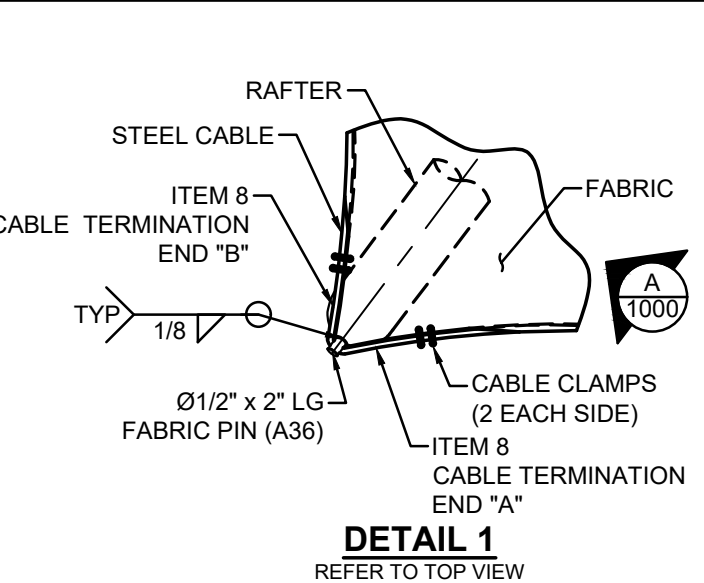
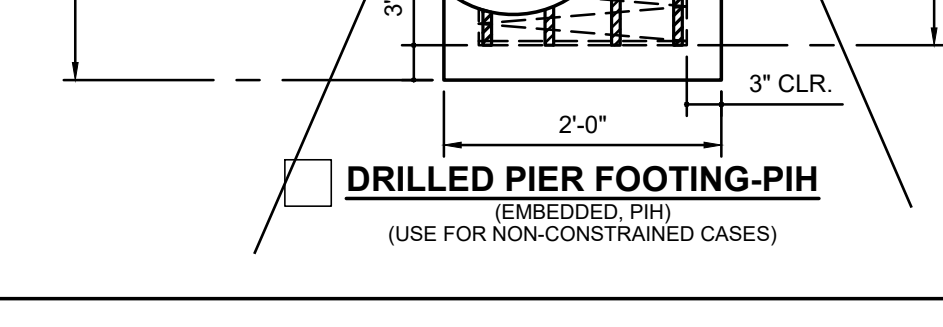
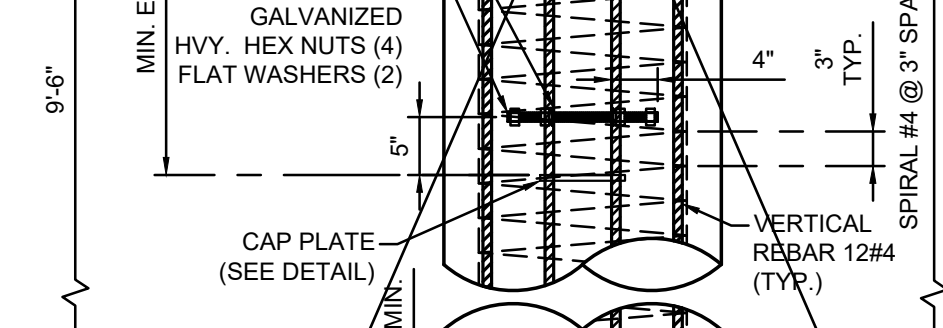
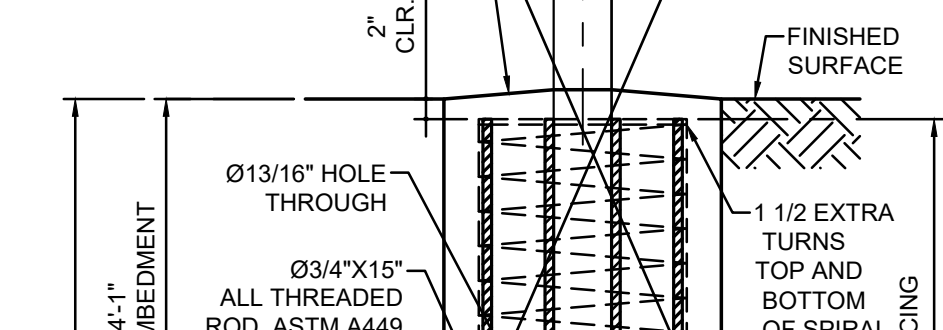
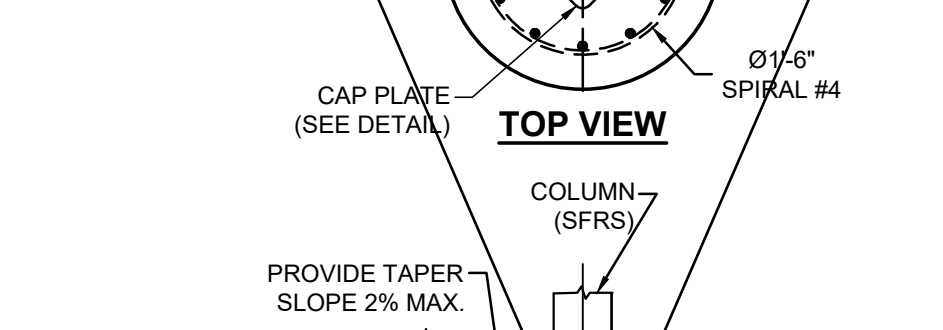
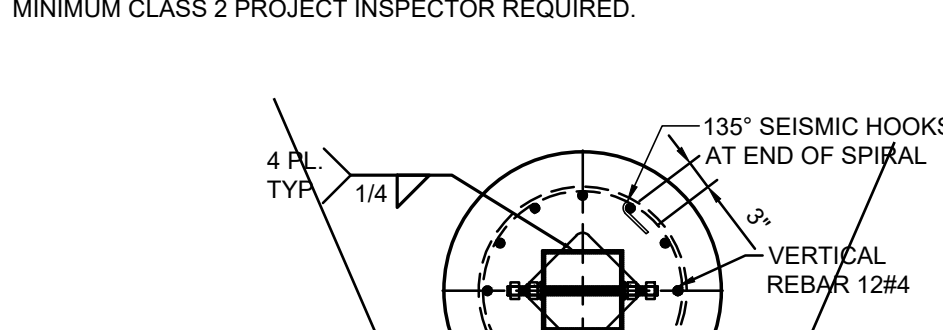
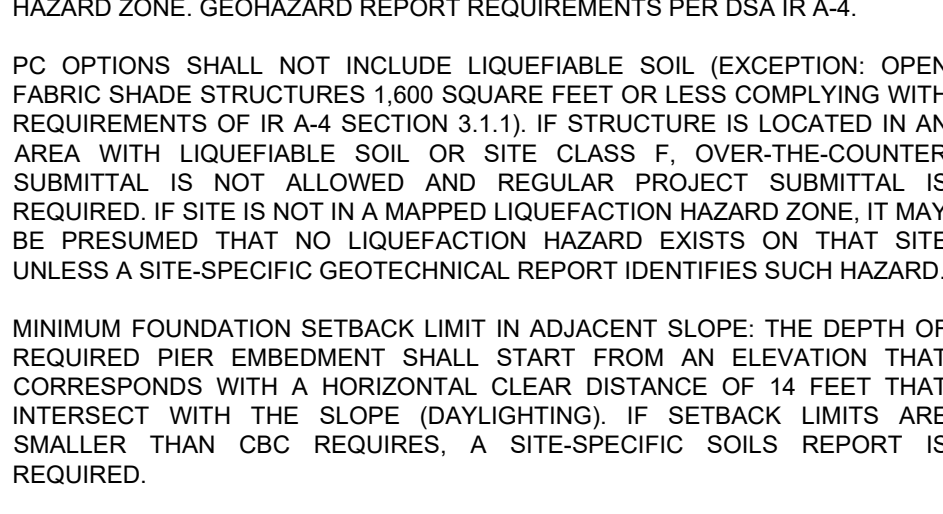
GEOHAZARD REPORT IS NOT REQUIRED FOR OPEN FABRIC STRUCTURES 1,600 SQ. FT. OR LESS COMPLYING WITH THE REQUIREMENTS OF IR A-4 SECTION 3.1.1. OPEN FABRIC SHADE STRUCTURES GREATER THAN 1,600 SQUARE FEET UP TO A MAXIMUM OF 4,000 SQUARE FEET AND COMPLYING WITH THE REQUIREMENTS NOTED IN IR A-4 SECTION 3.1.1 DO NOT REQUIRE A GEOHAZARD REPORT PROVIDED A GEOTECHNICAL REPORT INDICATES THAT NO LIQUEFACTION POTENTIAL EXISTS.

ARCHITECT OF RECORD TO DETERMINE IF SPECIFIC SITE IS IN GEOLOGIC HAZARD ZONE. GEOHAZARD REPORT REQUIREMENTS PER DSA IR A-4.

PC OPTIONS SHALL NOT INCLUDE LIQUEFIABLE SOIL (EXCEPTION: OPEN FABRIC SHADE STRUCTURES 1,600 SQUARE FEET OR LESS COMPLYING WITH REQUIREMENTS OF IR A-4 SECTION 3.1.1). IF STRUCTURE IS LOCATED IN AN AREA WITH LIQUEFIABLE SOIL OR SITE CLASS F, OVER-THE-COUNTER SUBMITTAL IS NOT ALLOWED AND REGULAR PROJECT SUBMITTAL IS REQUIRED. IF SITE IS NOT IN A MAPPED LIQUEFACTION HAZARD ZONE, IT MAY BE PRESUMED THAT NO LIQUEFACTION HAZARD EXISTS ON THAT SITE UNLESS A SITE-SPECIFIC GEOTECHNICAL REPORT IDENTIFIES SUCH HAZARD.

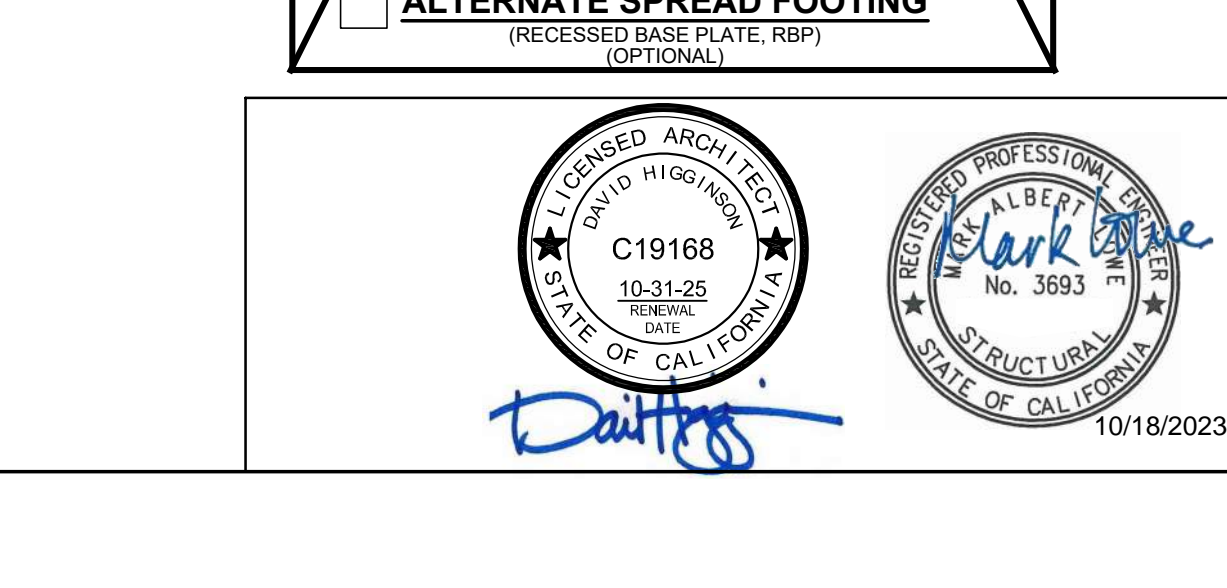
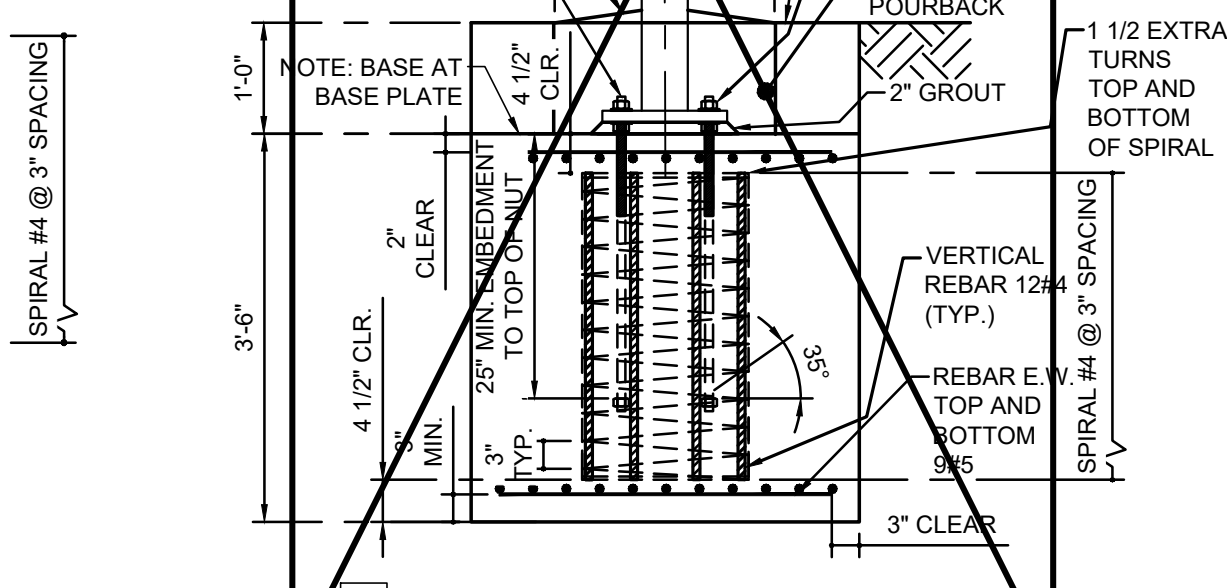
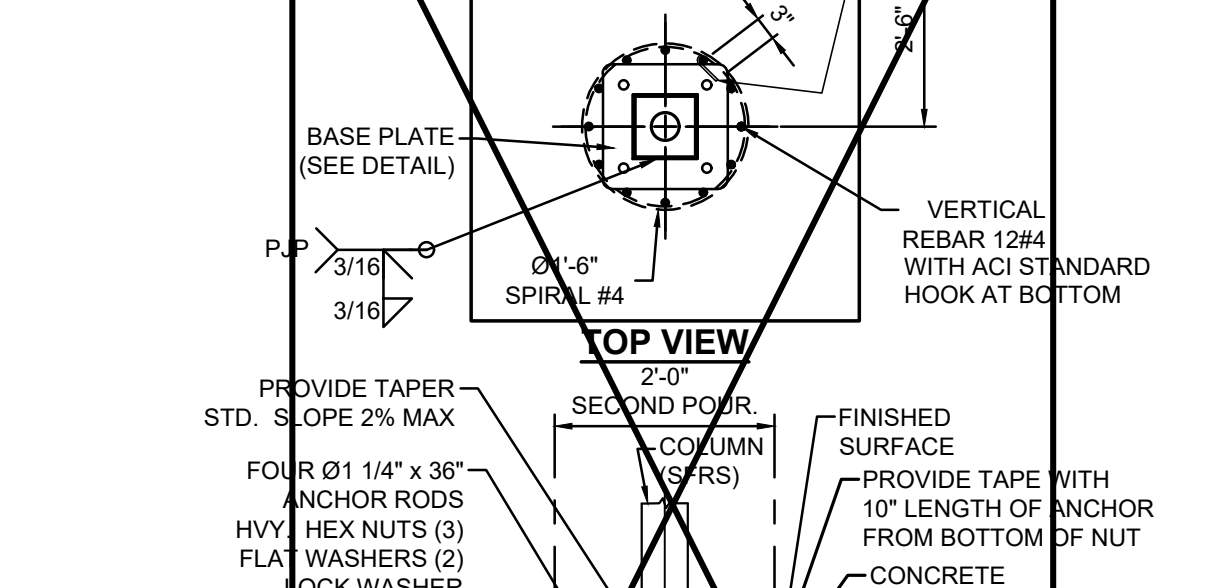
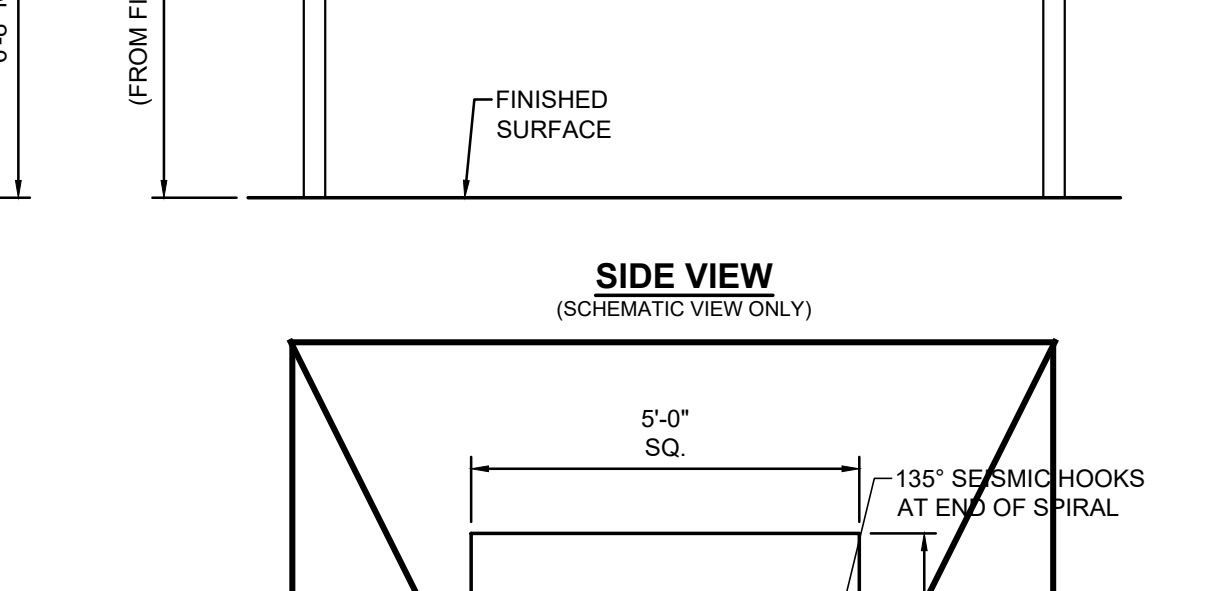
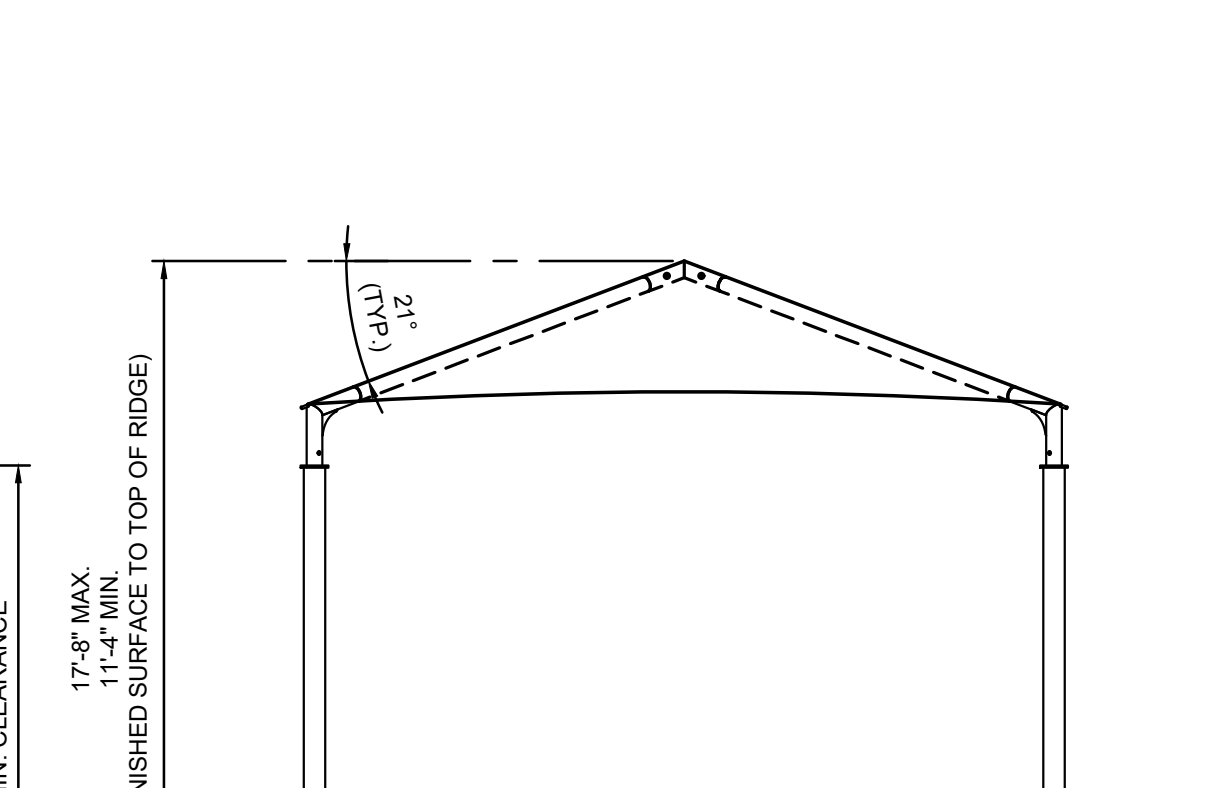
MINIMUM FOUNDATION SETBACK LIMIT IN ADJACENT SLOPE: THE DEPTH OF REQUIRED PIER EMBEDMENT SHALL START FROM AN ELEVATION THAT CORRESPONDS WITH A HORIZONTAL CLEAR DISTANCE OF 14 FEET THAT INTERSECT WITH THE SLOPE (DAYLIGHTING). IF SETBACK LIMITS ARE SMALLER THAN CBC REQUIRE, A SITE-SPECIFIC SOILS REPORT IS REQUIRED.

MINIMUM CLASS 2 PROJECT INSPECTOR REQUIRED.



ITEM	QTY	DESCRIPTION	MATERIAL
1	4	COLUMN	HSS 7.0 x 7.0 x 0.250
2	4	CUP CONNECTOR (6\" LG)	HSS 4.5 x 0.375
3	4	RAFTER (GALVANIZED STEEL TUBE)	5.00 GA 7 RD. TUBE (HSS 5.0 x 0.188)
4	4	EXTENSION (GALVANIZED STEEL TUBE)	5.00 GA 7 RD. TUBE (HSS 5.0 x 0.188)
5	2	CROSSPIECE (GALVANIZED STEEL TUBE)	5.00 GA 7 RD. TUBE (HSS 5.0 x 0.188)
6	1	RIDGE (GALVANIZED STEEL TUBE)	5.00 GA 7 RD. TUBE (HSS 5.0 x 0.188)
7	1	FABRIC TOP	FR COLOURSHADE 1900F5
8	1	Ø3/8\" CABLE	GALVANIZED STEEL
9	4	Ø3/8\" CABLE CLAMP	GALVANIZED STEEL
10	14	Ø5/8\"-11NC x 6 1/2\" HEX BOLT (ST)	316 SS
11	14	Ø5/8\"-11NC HEX NUT	316 SS
12	28	Ø5/8\" FLAT WASHER	316 SS
13	14	Ø5/8\" SPLIT LOCK WASHER	316 SS

THE MINIMUM CLEARANCE REQUIRED BETWEEN DRILLED PIERS WHEN PLACING MULTIPLE OPEN FABRIC SHADE STRUCTURES ADJACENT TO EACH OTHER, FROM CENTER TO CENTER, IS THREE TIMES THE LEAST HORIZONTAL DIMENSION OF THE PIER PER CBC 2022 SEC. 1810A.2.5.



THESE PLANS AND SPECIFICATIONS ARE THE PROPERTY OF USA SHADE AND FABRIC STRUCTURES AND SHALL NOT BE REPRODUCED WITHOUT THEIR WRITTEN PERMISSION.



CORPORATE HEADQUARTERS  
2580 ESTERS BLVD. SUITE 100  
DFW AIRPORT, TX, 75261  
800-966-5005

CERTIFICATIONS:  
IAS CERTIFICATION No: F4-428  
CLARK COUNTY MANUFACTURER  
CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER:  
Washington U.S.D.

PROJECT NAME:  
Riverbank Elementary School

LOCATION:  
1100 Carrie Street  
West Sacramento, CA 95605

MODEL NUMBER:  
DSA401304012-22

STRUCTURE TYPE:  
H I P  
DSA  
SIZE:  
30' x 40' x 12'e MAX.  
SCALE : NONE  
DRAWING SIZE:  
D

PRE-CHECK (PC)  
DOCUMENT  
Code : 2022 CBC  
A separate project application for construction is required.

Eng. By : HH 12/01/22  
Design By : OS 12/01/22  
Approved By : MB 12/01/22

DRAWING DESCRIPTION:  
PRODUCT INFORMATION

DWG.  
DSA401304012-22

SHEET  
7.1-1000

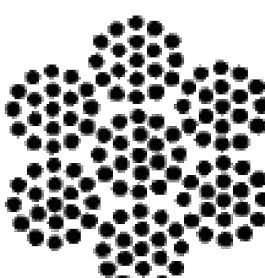
REV.  
NC



## Aircraft Cable

Preformed, made in accordance with commercial specifications military and federal specification rope available.

**Carbon Steel (Aircraft Cable)** - Galvanized cable has the highest strength and greatest fatigue life of the materials offered. It has good to fair corrosion resistance in rural to industrial atmosphere environments. This material is most widely used for small diameter cables. Tin over galvanized cable offers greater corrosion resistance and reduced friction over pulleys.



7 x 19

7 x 19		Galvanized Min. Breaking Strengths (lbs)
Dia. (In)	Approx. Wt 1000 Ft/lbs	
3/32	17.	1,000
1/8	29.	2,000
5/32	45.	2,800
3/16	65.	4,200
7/32	86.	5,600
1/4	110.	7,000
9/32	139.	8,000
5/16	173.	9,800
3/8	243.	14,400



## 190/F5 Fire rated specifications

### Standard range

Revision 0 28-Oct-12

Colour	Shade %	UV Block %	Average GSM	Average Warp break strength kgs	Average Elongation %	Average Weft break strength kgs	Average Elongation %	Average Burst Kpa	Average Burst to Mass ratio
Desert Sand	80	92	185	50	40	72	73	156	0.84
Blue	80	85	185	50	40	72	73	156	0.84
Brown	85		185	50	40	72	73	156	0.84
Green	80	85	185	50	40	72	73	156	0.84
Red	80	86	185	50	40	72	73	156	0.84
Silver	80	81	185	50	40	72	73	156	0.84
Terracotta	75	82	185	50	40	72	73	156	0.84
Yellow	80	89	185	50	40	72	73	156	0.84
				110 LB		159 LB		3258 PSF	

CONVERSION TO IMPERIAL UNITS:  
185 GSM = .0378 psf  
50 KGS = 110 Lb  
72 KGS = 159 Lb  
156 Kpa = 3258 psf

Notes:  
- 190/F5 conforms to The California State Fire Marshal Title 19 Test for Small scale Fabrics  
- Tear tests are done using a 50mm wide strip and a cross head speed of 500mm/min  
- This report has been compiled using the mean results from all tests conducted on the given sample by our Quality Control Laboratory, the information provided is considered to be a good reflection of the relevant properties of the fabric tested. These results must only be used as an indication of the quality and characteristics of the fabric tested.  
- Company cannot be held responsible or liable in any way whatsoever should this information differ to that of a registered testing institution.

Deon Joubert  
General Manager - Multiknit (Pty) Ltd

Tommy Rogers  
Managing Director - Multiknit (Pty) Ltd



## FLAME RETARDANT

### Fabric Registration

LICENSE NUMBER: F-052001

COLOURSHADE 190/F5

#### Product Marketed by:

MULTIKNIT (PTY) LTD  
BOX 798 WHITE RIVER 1240  
MPUMALANGA SOUTH AFRICA, .  
Issue Date : 05/08/2023  
Expiration Date : 06/30/2024

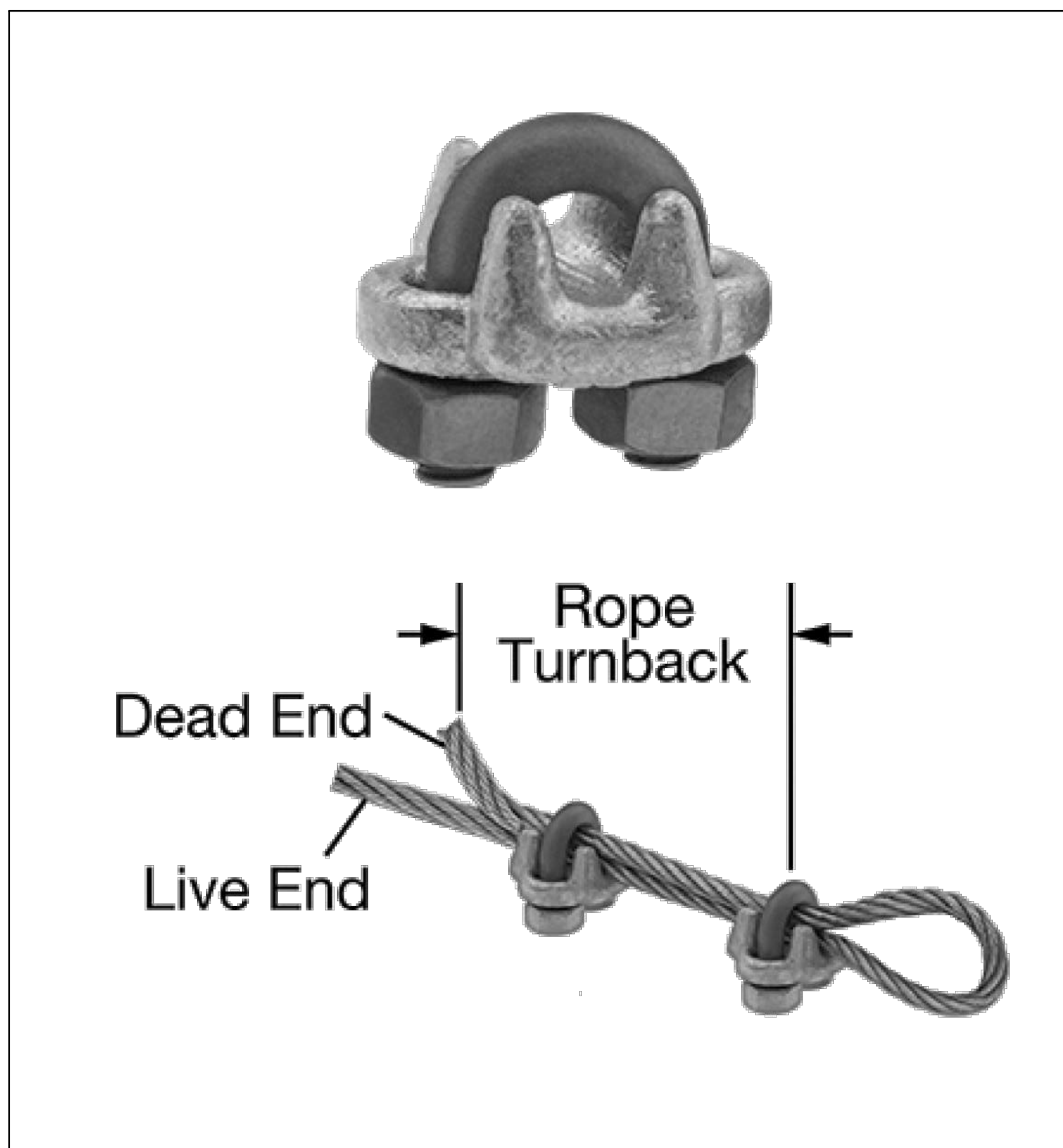
This product meets the minimum requirements of flame resistance established by the California State Fire Marshal for products identified in Section 13115, California Health and Safety Code. The scope of the approved use of this product is provided in the current edition of the CALIFORNIA APPROVED LIST OF FLAME RETARDANT CHEMICALS AND FABRICS, GENERAL AND LIMITED APPLICATIONS CONCERNS published by the California State Fire Marshal.

Issued By Cortney Walker  
Fire Engineering License Manager  
Fire Engineering & Investigations Division

Reviewed and Approved By Patricia Setter  
Deputy State Fire Marshal III  
Fire Engineering & Investigations Division

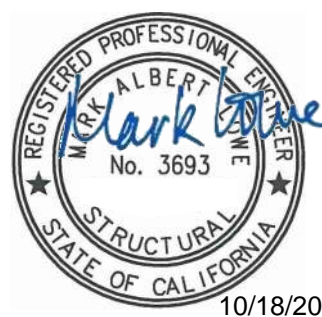
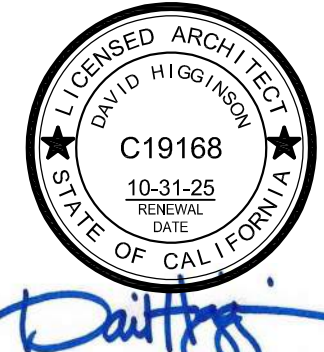
OFFICE OF THE STATE FIRE MARSHAL

Please visit calfire.gov/motus.org for more information on Licensing and Permitting with CAL FIRE



#### FORGED WIRE ROPE CLAMP

FITTING TYPE ROPE CLAMP  
FABRICATION: FORGED  
MATERIAL: GALVANIZED STEEL  
FOR WIRE ROPE DIAMETER 3/8"  
NUMBER OF CLAMPS REQUIRED: 2  
ROPE TURNBACK: 6 1/2"  
FOR WIRE ROPE CONSTRUCTION 7 x 19  
ATTACHMENT TYPE: LOOP  
CLAMP WIDTH 2", HEIGHT 1 15/16", THICKNESS 1 11/16"  
REQUIRED INSTALLATION TOOL TORQUE WRENCH  
REQUIRED TORQUE 45 FT.-LBS.  
CAPACITY 80% OF THE ROPE'S CAPACITY  
SPECIFICATIONS MET ASME B30.26, FED. SPEC. FF-C-450



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CORPORATE HEADQUARTERS  
2580 ESTERS BLVD. SUITE 100  
DFW AIRPORT, TX, 75261  
800-966-5005

#### CERTIFICATIONS:

IAS CERTIFICATION No: FA-428  
CLARK COUNTY MANUFACTURER  
CERTIFICATION NUMBER (NEVADA): 355

#### CUSTOMER:

Washington U.S.D.

#### PROJECT NAME:

Riverbank Elementary School

#### LOCATION:

1100 Carrie Street  
West Sacramento, CA 95605

#### MODEL NUMBER:

DSA401304012-22

#### STRUCTURE TYPE:

H I P

DSA

SIZE: MAXIMUM  
30' x 40' x 12'e MAX.

SCALE : NONE

#### DRAWING SIZE:

D

#### PRE-CHECK (PC)

##### DOCUMENT

Code : 2022 CBC  
A separate project application for construction is required.

Eng. By : HH 12/01/22

Design By : OS 12/01/22

Approved By : MB 12/01/22

#### DRAWING DESCRIPTION:

##### SPECIFICATIONS

DWG. DSA401304012-22

SHEET 7.2-2000

REV. NC