

# WESTMORE OAKS ES

## ESSR III

### 1504 FALLBROOK ST.

### WEST SACRAMENTO, CA 95691

### WASHINGTON UNIFIED SCHOOL DISTRICT

DSA File No. 57-31  
App. No. 02-122280  
PTN. 72694-126

#### DSA REQUIREMENTS

- ALL WORK SHALL CONFORM TO THE 2022 EDITION OF THE TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR).
- AS A FACILITY WHICH COMES UNDER THE APPROVAL AND AUTHORITY OF THE DIVISION OF THE STATE ARCHITECT (DSA), THIS PROJECT IS SUBJECT TO DRAWING AND JOB SITE REVIEW BY A REPRESENTATIVE OF DSA.
- CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS AFFECTING FLS, SSS, AND/OR ACS SHALL BE MADE BY ADDENDA OR A CHANGE ORDER APPROVED BY THE DIVISION OF THE STATE ARCHITECT, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR AND DSA IR A-6.
- A DSA CERTIFIED PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY THE DIVISION OF THE STATE ARCHITECT SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR. GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS SHALL COMPLY WITH ALL LOCAL ORDINANCES.
- A COPY OF PART 1 TO PART 5 OF TITLE 24 SHALL BE KEPT AND BE AVAILABLE IN THE FIELD DURING CONSTRUCTION.
- DSA SHALL BE NOTIFIED OF THE START OF CONSTRUCTION AND PRIOR TO THE PLACEMENT OF CONCRETE PER SECTION 4-331, PART 1, TITLE 24, CCR. THE DIVISION OF THE STATE ARCHITECT IS EXEMPT FROM ARBITRATION OR MEDIATION PROCEDURES.
- SUPERVISION BY THE DIVISION OF THE STATE ARCHITECT IS PER SECTION 4-334, PART 1, TITLE 24, CCR.
- ADMINISTRATION OF CONSTRUCTION PER PART 1, TITLE 24, CCR:
  - VERIFIED REPORTS PER SECT 4-336, PART 1, TITLE 24 CCR
  - DUTIES OF ARCHITECT PER SECT 4-331, 4-341; PART 1, TITLE 24 CCR
  - DUTIES OF CONTRACTOR PER SECT 4-343, PART 1, TITLE 24
- TESTING AND INSPECTION:
  - INSPECTOR APPROVED BY DSA AS PER SECT. 4-333(D); PART 1, TITLE 24, CCR
  - TESTS AND TESTING LABORATORIES PER SECT 4-335
  - SPECIAL INSPECTION PER SECT. 4-333(C)
- CHANGES IN LEVEL FOR FLOOR FINISHES SHALL CONFORM WITH CBC SECTION 1120B.2 AND 1120B.3.
- ALL TESTS TO CONFORM TO REQUIREMENTS OF SECTION 4-335, PART 1, TITLE 24, CCR.
- TESTS OF MATERIALS AND TESTING LABORATORY SHALL BE IN ACCORDANCE WITH SECTION 4-335, PART 1, TITLE 24, CCR AND THE DISTRICT SHALL EMPLOY AND PAY THE DSA ACCEPTED LABORATORY. COSTS OF RE-TEST MAY BE BACK CHARGED TO THE CONTRACTOR.
- INSPECTOR SHALL BE APPROVED BY DSA. INSPECTION SHALL BE IN ACCORDANCE WITH SECTION 4-333(B).
- THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS IS THAT THE WORK OF THE ALTERATION, REHABILITATION OR RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24, CCR. SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHICH IS NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT (CCD), OR A SEPARATE SET OF PLANS AND SPECIFICATIONS, DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK (SECTION 4-317(C), PART 1, TITLE 24, CCR).
- INSPECTOR OF RECORD REQUIREMENTS:
  - ONE OR MORE INSPECTORS EMPLOYED BY THE OWNER IN ACCORDANCE WITH THE REQUIREMENTS OF TITLE 24 OF THE CALIFORNIA CODE OF REGULATIONS WILL BE ASSIGNED TO THE WORK. THE INSPECTOR'S DUTIES ARE SPECIFICALLY DEFINED IN SECTION 4-342 OF SAO TITLE 24, PART 1 AND IN ADDITION, SHALL BE STIPULATED IN INTERPRETATION OF REGULATION DOCUMENT IR A-8.
  - INSPECTOR SHALL BE CERTIFIED AS A CLASS 2 INSPECTOR THROUGH THE DIVISION OF THE STATE ARCHITECT INSPECTOR EXAMINATION PROGRAM. INSPECTOR SHALL ALSO BE SPECIFICALLY APPROVED BY THE DIVISION OF THE STATE ARCHITECT FOR THIS PROJECT AT LEAST 10 DAYS PRIOR TO THE START OF ANY WORK FOR THIS PROJECT.

#### DEFERRED APPROVALS

- NONE

#### ADD ALTERNATES

- NONE

#### PROJECT DESCRIPTION

APN: 058-220-043

THE PROJECT INCLUDES NEW METAL AND FABRIC SHADE STRUCTURES.

FABRIC SHADE STRUCTURE TO HAVE 340°F FABRIC FOR FLAME RETARDANT, COMPLYING WITH TITLE 19, SECTION 315(a)

WIND EXPOSURE = +93 MPH

#### STATEMENT OF GENERAL CONFORMANCE

FOR ARCHITECTS/ENGINEERS WHO UTILIZE PLANS, INCLUDING BUT NOT LIMITED TO SHOP DRAWINGS, PREPARED BY OTHER LICENSED DESIGN PROFESSIONALS AND/OR CONSULTANTS

Application No. 02-122280  
File No. 57-31

- [X] The drawings or sheets listed on the cover or index sheet (all SHADE STRUCTURE drawings)
- [ ] This drawing, page of specifications/calculations

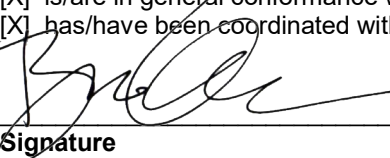
have been prepared by other design professionals or consultants who are licensed and/or authorized to prepare such drawings in this state. It has been examined by me for:

- design intent and appears to meet the appropriate requirements of Title 24, California Code of Regulations and the project specifications prepared by me, and
- coordination with my plans and specifications and is acceptable for incorporation into the construction of this project.

The Statement of General Conformance "shall not be construed as relieving me of my rights, duties, and responsibilities under Sections 17302 and 81138 of the Education Code and Sections 4-336, 4-341 and 4-344" of Title 24, Part 1, (Title 24, Part 1, Section 4-317 [b])

I find that:  
[X] All drawings or sheets listed on the cover or index sheet  
[ ] This drawing or page

[X] is/are in general conformance with the project design and  
[X] has/have been coordinated with the project plans and specifications

Signature  Date 03/12/2024

Architect or Engineer designated to be in general responsible charge.

Brian P. Whitmore

Print Name

C 30345

License Number

09-30-2025

Expiration Date

REFERENCE CODE SECTION FOR NFPA STANDARDS - 2022 CBC (SFM) CHAPTER 35. SEE CHAPTER 35 FOR STATE OF CALIFORNIA AMENDMENTS TO NFPA STANDARDS.

STATEMENT OF GENERAL CONFORMANCE AND SIGNATURE BLOCK PER IR A-18

#### DRAWING INDEX

SHT.  
NO.

DESCRIPTION

GENERAL

- |      |   |
|------|---|
| A0.1 | COVER SHEET                             |
| A0.2 | GENERAL NOTES                           |
| A0.3 | ARCHITECTURAL SYMBOLS AND ABBREVIATIONS |
| A0.5 | CODE ANALYSIS SITE PLAN                 |
| A0.6 | CODE ANALYSIS FLOOR PLAN                |

CIVIL

- |    |                    |
|----|--------------------|
| T1 | TOPOGRAPHIC SURVEY |
|----|--------------------|

ARCHITECTURAL

- |      |                               |
|------|-------------------------------|
| A1.1 | SITE PLAN OVERALL             |
| A1.2 | ENLARGED SHADE STRUCTURE PLAN |

METAL SHADE STRUCTURE (PC 04-122375)

- |       |   |
|-------|---|
| LS1.0 | GENERAL INFO  |
| LS1.1 | GENERAL INFO  |
| LS3.0 | 30' WIDE RECTANGULAR HIP FOUNDATION PLAN              |
| LS3.1 | 30' WIDE RECTANGULAR HIP FRAMING & CONNECTION DETAILS |
| LS3.4 | 30' WIDE RECTANGULAR STANDING SEAM ROOFING PLAN       |

FABRIC SHADE STRUCTURE (PC 04-121917)

- |           |                     |
|-----------|---------------------|
| T1.0      | TITLE SHEET         |
| T-2.0     | UNIT SELECTION      |
| 11.1-1000 | PRODUCT INFORMATION |
| 11.2-2000 | SPECIFICATIONS      |

TOTAL SHEET COUNT: 17

SHT.  
NO.

DESCRIPTION

#### PROJECT DIRECTORY

##### CLIENT

WASHINGTON UNIFIED SCHOOL DISTRICT

930 WESTACRE ROAD  
[T] (916) 375-7600  
DANIEL BANOWETZ  
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##### ARCHITECT

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BYUN PARTNERS

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David@byunpartners.com

##### PC SHADE STRUCTURE

USA SHADE

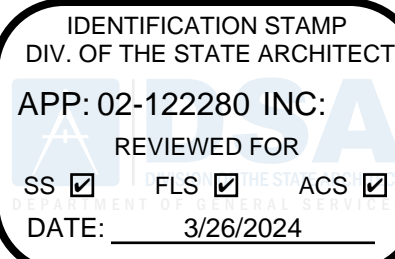
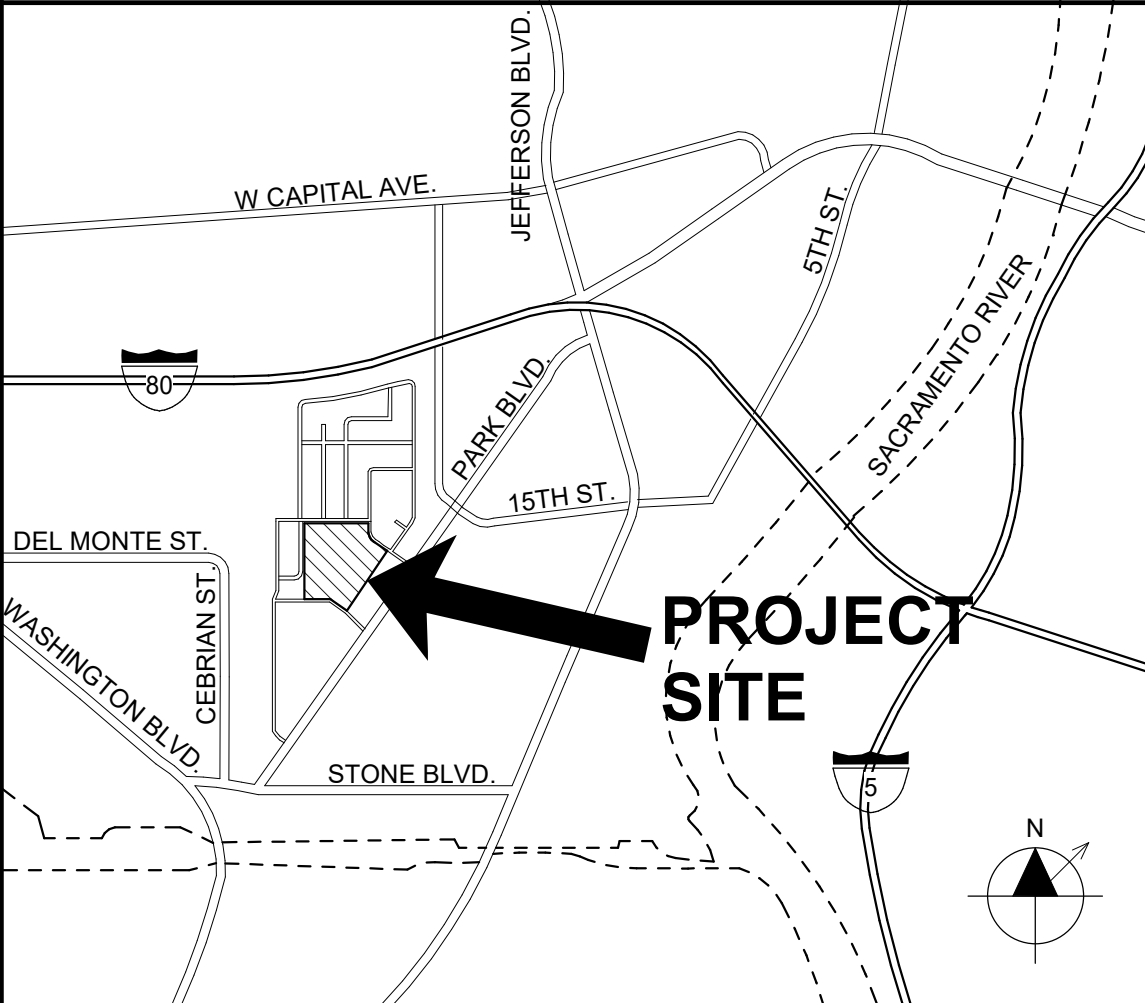
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erik.anslinger@usa-shade.com

##### PC SHADE STRUCTURE

PARK PLANET

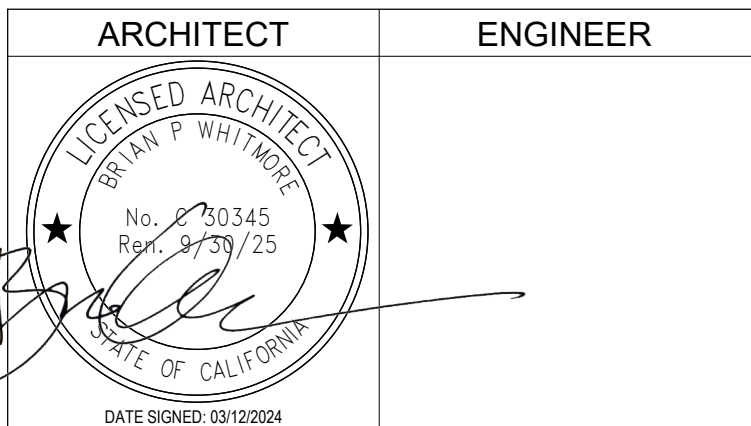
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kyle@parkplanet.com

#### VICINITY MAP



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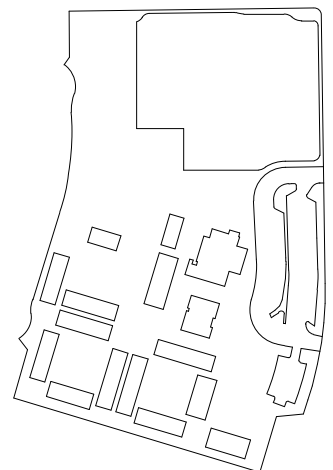


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NO.	REMARKS	DATE

DRAWING STATUS	DATE
<input type="radio"/> DSA PLAN CHECK	
<input type="radio"/> DSA BACK CHECK	
<input type="radio"/> BIDDING	
<input type="radio"/> CONSTRUCTION	

#### KEY PLAN



WASHINGTON UNIFIED  
SCHOOL DISTRICT  
930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

#### PROJECT STATUS

WESTMORE OAKS ES  
ESSR III  
1504 FALLBROOK ST.  
WEST SACRAMENTO, CA 95691

#### COVER SHEET

Date  
03/12/2024

Application Number  
02-122280

Drawn  
Author

Project Number  
22046

Drawing Number

02-122280

Checked  
Checker

A0.1



<

IDIFICATION STAMP DIV. OF THE STATE ARCHITECT	
APP: 02-122280 INC:	
REVIEWED FOR	
SS <input checked="" type="checkbox"/>	FLS <input checked="" type="checkbox"/> ACS <input checked="" type="checkbox"/>
DATE: 3/26/2024	

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ARCHITECT	ENGINEER
<div>LICENSED ARCHITECT BRIAN P. WHITMAN No. 00345 Ren. 3/29/25 STATE OF CALIFORNIA DATE SIGNED 03/20/24</div>	

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	NO.	REMARKS	DATE
REVISION HISTORY			

DRAWING STATUS

☐ DSA PLAN CHECK

☐ DSA BACK CHECK

☐ BIDDING

☐ CONSTRUCTION

DATE

KEY PLAN

WASHINGTON UNIFIED  
SCHOOL DISTRICT  
930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

PROJECT STATUS

WESTMORE OAKS ES  
ESSR III  
1504 FALLBROOK ST.  
WEST SACRAMENTO, CA 95691

GENERAL NOTES

Date 03/12/2024	Project Number 22046
Application Number 02-122280	Drawing Number
Drawn Author	Checked Checker

A0.2



ARCHITECTURAL DRAWING ABBREVIATIONS

# & * 2X @ L	POUND OR NUMBER AND ITEMS IDENTIFIED AS "NIC" ARE NOT PART OF THIS DSA APPROVAL NOMINAL LUMBER SIZE (4X, 6X, 8X, ETC.) PERPENDICULAR	DG DH DIA DIAG DIFF DIM DISP DIV DMPF DMT DN DR DRB DRLV DS DSP DT DRTL DW DWG DWL DWR	DECOMPOSED GRANITE DOUBLE HUNG DIAMETER DIAGONAL DIFFUSION DIMENSION DISPENSER DIVISION DAMP-PROOFING ARCHITECT/ENGINEER ANCHOR BOLT ABANDON AGGREGATE BASE COURSE ABOVE ASPHALTIC CONCRETE ACCESS(IBLE) ALUMINUM COMPOSITE PANEL ACOUSTICAL ACOUSTICAL CEILING TILE AREA DRAIN ADDENDUM ADHESIVE ADJUSTABLE ADJACENT ABOVE FINISH FLOOR AGGREGATE AIR HANDLING UNIT ASSISTED LISTENING SYSTEM ALTERNATE ALUM./AL. ALUMINUM ANCHOR, ANCHORAGE APPLIED APPROXIMATELY ARCHITECT(URAL) ASC ABOVE SUSPENDED CEILING ASF ABOVE STAGE FINISH ASPH ASPHY ASSEMBLY ASYM ASYMMETRICAL AUTOMATIC AV AUDIO VISUAL AWG AMERICAN WIRE GAUGE	DG DH DIA DIAG DIFF DIM DISP DIV DMPF DMT DN DR DRB DRLV DS DSP DT DRTL DW DWG DWL DWR  E (E) E EA EAR EBS EE EF EFS EHD EHS EJ EL ELAST ELEC ELEV EM EMER EN ENCL ENGR ENTR EP EQ EQUIP ESC ESCL ESMT EW EWC EWH EWS EXC EXH EXP EXPN EXT  F (F) F/F FA FAB FBD FBRK FCBRK FD FDN FE FEC FF FFA FB FFEL FFL FGL FHC FHM FHMS FHS FIN FLG FLD FLG FLR FLU FN FOB FOC FOF FOG FOM FOS FPL FPR FR FRG FRP FRTW FRZ FS FSTN FT FTG FURG FWC  G GA GAL GALV GB GFRG GI GL GLULAM GLZ GLZCMU GND GPC GR GRBM GRLN GSB GSM GSS GST GT GVL GY GYB H HB HC HD HDAS HDT HDR HDW HOWD HEX HGR HLDN HM	HMD HMF HMF HNDRL HPRZ PH HR HT HTG HVAC HWH  I ID IN INCL INFO INSTL INSUL INT INV IPS ISA  J JAN JST JT  K KIT KO KPL  L LAB LAD LAM LAV LB(S) LBL LBR LDR LF LG LH LHR LKN LKR LKWASH LLH LLV LMS LNSCP LNTL LNTL LP LPT LT LTWT LV LVL LWC LWIC  M MAINT MAS MATL MAX MB MBR MC MCB MDO MECH MED MEMB MEZZ MFD MFR MH MIB MIR MISC ML MLDG MLWK MO MOD MR MRB MRD MS MTD MTL MTR MULL  N NEW N NAT NCOMBL NE NF NIC NLB NM NO NOM NR NRC NRCA NS NTS	HOLLOW METAL DOOR HOLLOW METAL DOOR AND FRAME HOLLOW METAL FRAME HANDRAIL HORIZONTAL HIGH POINT HOUR HEIGHT HEATING HEATING, VENTILATING, AIR CONDITIONING HOT WATER HEATER  INSIDE DIAMETER DOWNSPOUT INCLUDE(D), (ING) INFORMATION INSTALL INSULATED(D), (ION) INTERIOR INVERT IRON PIPE SIZE INTERNATIONAL SYMBOL OF ACCESSIBILITY  JANITOR JOIST JOINT  KITCHEN KNOCKOUT KICKPLATE  LABORATORY LADDER LAMINATE LAVATORY POUND(S) LABEL LUMBER LEADER LINEAL FOOT LENGTH, LONG LEFT HAND LEFT HAND REVERSE LOCKNUT LOOKER LOOKWASHER LONG LEG HORIZONTAL LONG LEG VERTICAL LANDSCAPE(D) LINTEL LIGHTPROOF LOW POINT LIGHT LIGHTWEIGHT LOUVER VENT LEVEL(ER) LIGHTWEIGHT CONCRETE LIGHTWEIGHT INSULATING CONCRETE  MAINTAIN(ANCE) MASONRY MATERIAL MAXIMUM MACHINE BOLT MEMBER MEDICINE CABINET METAL CORNER BEAD MEDIUM DENSITY OVERLAD MECHANICAL MEDIUM MEMBRANE MEZZANINE METAL FLOOR DECKING MANUFACTURER MANHOLE MINOR MIRROR MISCELLANEOUS METAL LATH MOLDING JOINT MILLWORK MASONRY OPENING MODULE(AR) MOISTURE RESISTANT MARBLE METAL ROOF DECKING MACHINE SCREW MOUNTED METAL MORTAR MULLION  NEW NORTH NATURAL NONCOMBUSTIBLE NOT EXCEEDING NEAR FACE NOT IN CONTRACT NON-LOAD BEARING NONMETALLIC NUMBER NOMINAL NOISE REDUCTION NOISE REDUCTION COEFFICIENT NATIONAL ROOFING CONTRACTOR'S ASSOCIATION NEAR SIDE NOT TO SCALE	PEDESTAL PERFORATED PERIMETER PERPENDICULAR PERGOLA PHASE PHILLIPS HEAD SCREW POINT OF INTERSECTION POST INDICATOR VALVE PLATE, PROPERTY LINE PLASTIC LAMINATE PLASTER PLYWOOD PRESSED METAL PRESSED VINYL FRAME PNEUMATIC PANEL PAINT(ED) POLISHED POLYETHYLENE PORCELAIN PORTABLE PAIR PRECAST PREFABRICATED PREFINISHED PREFORMED PARKING PREMOLDED PROJECT PROPERTY PRESTRESSED CONCRETE POINT PAPER TOWEL DISPENSER PRESSURE TREATED DOUGLAS FIR PARTITION PAPER TOWEL RECEPTACLE POLYVINYL CHLORIDE PAVE(ED), (ING) PAVEMENT  Q QT QTB QTF QTR QTY  R R RA RAB RAD RB RBR RCP RCVR RD RDWY REBAR REC RECT RECYCLING REFERENCE REFLECT(ED), (IVE), (OR) REFRIGERATOR REGISTER REINFORCED REMOVE(ABLE) REPAIR REPLACE REQUIRED RESILIENT RETURN REVISION(S), REVISED RESILIENT FLOORING ROOF RFG RFH RGDINS RH RHMS RHR RHS RL RLG RM RND RO ROW RR RS RTF RTU RV RVL RVS RVT RWD RWL  S S S2S S4S SA SALV SAM SAT SB SBSSTR SC SCD SCHED SCP SCRN SD SDBL SEC SECT SEP SF SGL SHR SHT SHTG SHV SIM SK SKLT SLD SLDG SLDR SLNT SLV SM SMACNA SMLS SMS SND SNDINS SNDU SNT SP SPC SPD SPEC SPT SQ SS SSK	STREET STATION STAGGERED STC STD STG STIF STIRRUP STR STOR STL STRAIGHT STRUCTURAL PLYWOOD SUSPENDED SHEET VINYL SYMMETRICAL SYNTHETIC SYSTEM  T T24 T&B TAG TB TBE TD TDR TEL TEMP TER TER TFA TFB THD THERM THK THRES THRU THROUG TKBD TMPD TO TOB TOC TOF TOFF TOJ TOL TOM TOP TOPV TOS TOSL TOST TOW TPD TPTN TRANS TS TV TWLB TYP  U UC UGND UL UNFIN UNF UR URM UTILITY  V VAR VB VCT VER VERT VEST VF VFAT VIF VJ VNR VR VTR VWC  W W W.O WITH W/O W/W WBL WC WD WDP WDW WF WFS WGL WH WH WI WID WLD WM WP WPT WR WS WSCT WT WWF  X XBRACE XFMR XSECT  Y YCO YD	STREET STATION STAGGERED SOUND TRANSMISSION CLASS STANDARD SEATING STIFFENER STIRRUP STEEL STORAGE STRAIGHT STRUCTURAL PLYWOOD SUSPENDED SHEET VINYL SYMMETRICAL SYNTHETIC SYSTEM  TEMPERED, TOILET, TREAD TITLE 24 TOP AND BOTTOM TONGUE & GROOVE THRU BOLT THREADED BOTH ENDS TOWEL DISPENSER TOWEL DISPENSER/RECEPTACLE TELEPHONE TEMPORARY TERRAZZO TO FLOOR ABOVE TO FLOOR BELOW THREADED THERMAL THICK THRESHOLD THROUGH TACKBOARD TEMPERED TOP OF BEAM TOP OF CURB OR TOP OF CONCRETE TOP OF FOOTING TOP OF FINISH FLOOR TOP OF JOIST TOLERANCE TOP OF MASONRY TOP OF PARAPET TOP OF PAVEMENT TOP OF SHEATHING TOP OF SLAB TOP OF STEEL TOP OF WALL OR TOP OF WALK TOILET PAPER DISPENSER TOILET PARTITION TRANSITION TUBE DRAIN TELEVISION TOWEL BAR TYPICAL  UNDERCUT UNDERGROUND UNDERWRITER'S LABORATORY UNFINISHED UNLESS OTHERWISE NOTED URINAL UNREINFORCED MASONRY UTILITY  VARIES VINYL BASE VINYL COMPOSITION TITLE VERIFY VERTICAL VESTIBULE VINYL FABRIC VINYL FACED ACOUSTIC TILE RIGHT HAND REVERSE V-JOINT(ED) VENEER VAPOR RETARDER VENT THROUGH ROOF VINYL WALL COVERING  WEST WHERE OCCURS WITH WITHOUT WALL TO WALL WOOD BLOCKING WATER CLOSET WOOD WOOD PANELING WINDOW WIDE FLANGE WOOD FURRING STRIP WIRED GLASS WATER HEATER WALL HUNG WROUGHT IRON WIDTH, WIDE WELD(ED) WIRE MESH WATERPROOFING WORKING POINT WIRE ROPE WOOD SCREW WAINSCOT WEIGHT WELED WIRE FABRIC  CROSS BRACE TRANSFORMER CROSS SECTION  YARD CLEANOUT YARD
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SYMBOLS LEGEND

	N = PLAN NORTH ARROW ADDITIONAL ARROW INDICATES TRUE NORTH
	X = BUILDING SECTION NUMBER AXX = SHEET NUMBER
	X = WALL SECTION NUMBER AXX = SHEET NUMBER
	X = EXTERIOR ELEVATION NUMBER AXX = SHEET NUMBER
	X = INTERIOR ELEVATION NUMBER AXX = SHEET NUMBER N.S.E.W. = INDICATES CARDINAL DIRECTION
	GRID LINE, FACE OF STRUCTURE
	GRID LINE, CENTER OF STRUCTURE
	ELEVATION OR DATUM POINT
	WORK POINT
	REFERENCE DETAIL X = DETAIL DRAWING NUMBER AXX = SHEET NUMBER
	MATCH LINE AND AREA DESIGNATOR SHADED PORTION IS THE SIDE CONSIDERED
	ROOM NAME A119
	ROOM NAME A101A
	KEYNOTE 08 = SPECIFICATION DIVISIONAL PREFIX 2 = SPECIFICATION SUBSECTION PREFIX 11 = NOTE NUMBER
	REVISION 1
	CASEWORK TAG 102 = ARCHITECTURAL WOODWORK STANDARD (AWS) NUMBER 36" x 24" x 24" = WIDTH x HEIGHT x DEPTH LOCKABLE = MODIFYING NOTE
	PATH OF EGRESS 41 = OCCUPANT LOAD STARTING POINT OF PATH OF TRAVEL TO EXIT MARKED BY DOT AT THE BEGINNING OF EGRESS LINE
	PANIC HARDWARE DEVICE - REFERENCE DOOR SCHEDULE AND HARDWARE GROUP
	SIGNAGE TAG S1

MATERIALS LEGEND

	EARTH		BATT INSULATION
	POROUS FILL (STONE, GRAVEL, ETC.)		RIGID INSULATION
	CONCRETE		GYPSUM BOARD
	GROUT		PLYWOOD
	STEEL		METAL LATH AND PLASTER
	FINISHED WOOD		
	TERRAZZO		
	WOOD BLOCKING		
	WOOD FRAMING		

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122280 INC:  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☒  
DATE: 3/28/2024

# STUDIO W

## ARCHITECTS

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www.StudioW-Architects.com

ARCHITECT  
ENGINEER  

DATE SIGNED: 03/28/2024

GENERAL NOTES  
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REVISION HISTORY  

NO.	REMARKS	DATE

DRAWING STATUS  
☐ DSA PLAN CHECK  
☐ DSA BACK CHECK  
☐ BIDDING  
☐ CONSTRUCTION

### KEY PLAN

### WASHINGTON UNIFIED SCHOOL DISTRICT

930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

### PROJECT STATUS

### WESTMORE OAKS ES

ESSR III  
1504 FALLBROOK ST.  
WEST SACRAMENTO, CA 95691

### ARCHITECTURAL SYMBOLS AND ABBREVIATIONS

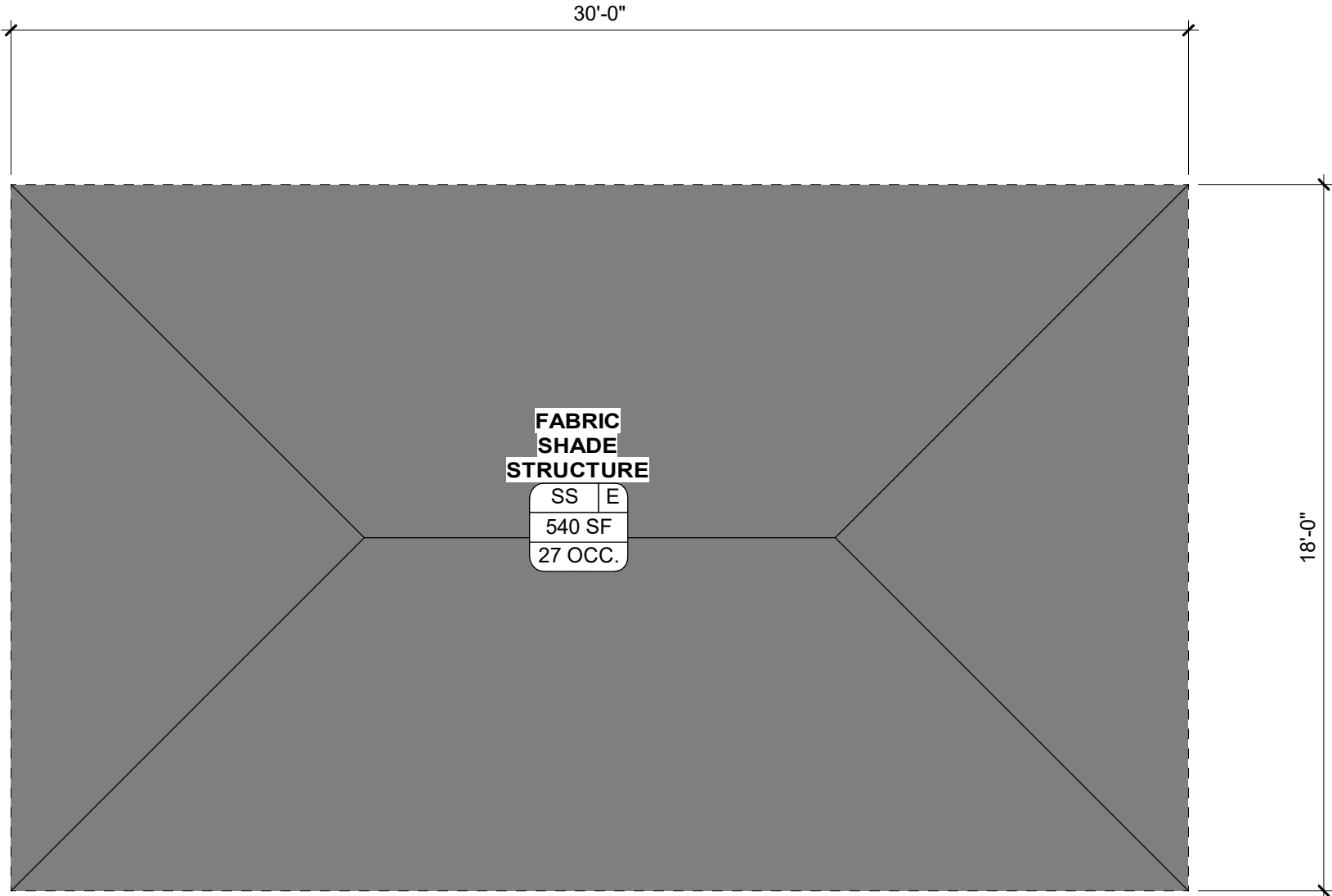
Date  
03/12/2024  
Application Number  
02-122280  
Drawn  
Author

Project Number  
22046  
Drawing Number  
A0.3  
Checked  
Checker

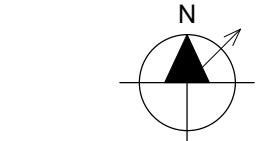




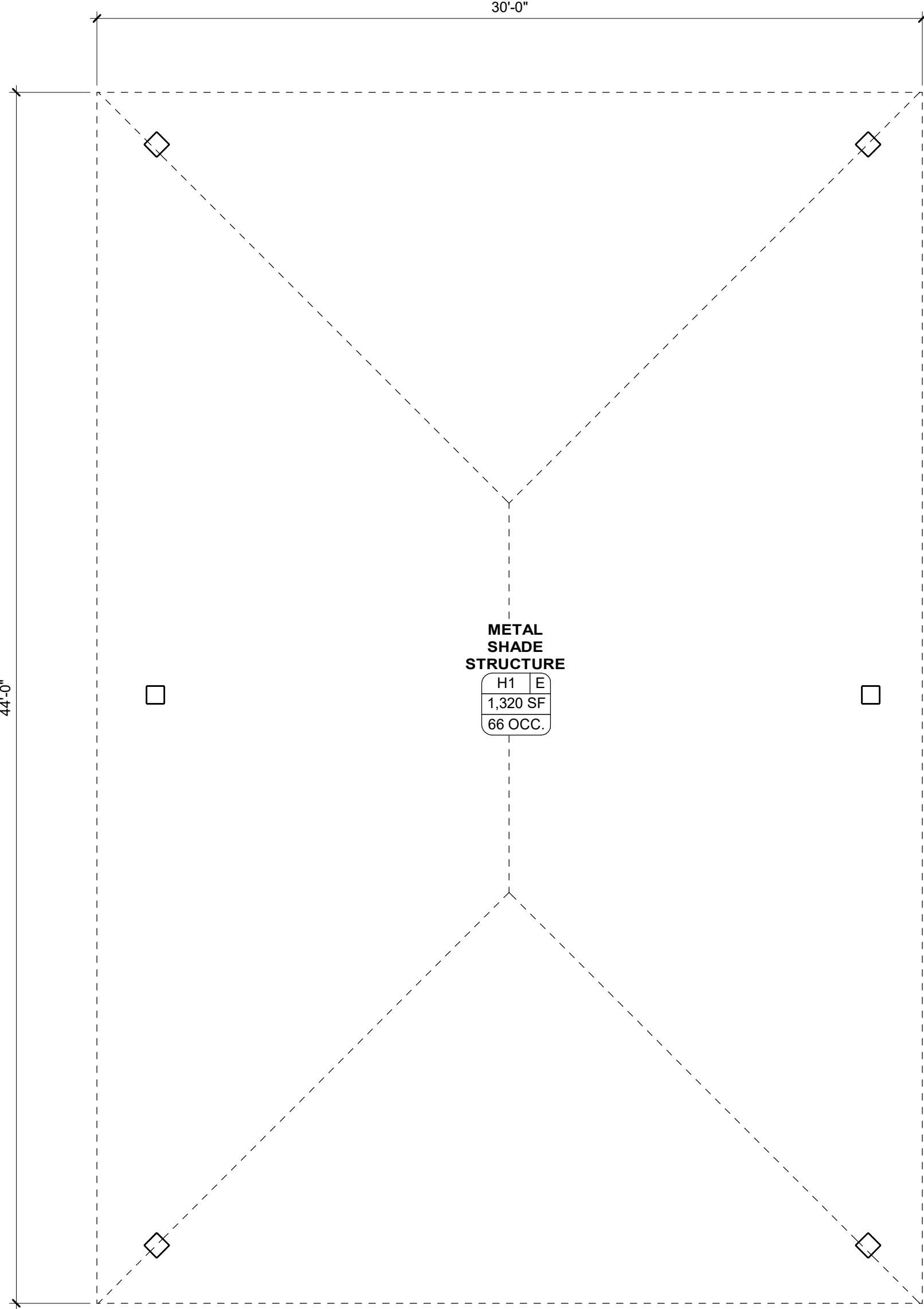


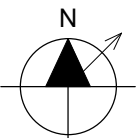
CODE ANALYSIS FLOOR PLAN - FABRIC SHADE STRUCTURE



1/4" = 1'-0" 7



CODE ANALYSIS FLOOR PLAN - METAL SHADE STRUCTURE



1/4" = 1'-0" 10

KEYNOTES

NUMBER	NOTE

GENERAL NOTES

1. NONE

OCCUPANT LOAD CHART

ROOM NUMBER	ROOM NAME	FUNCTION OF SPACE (CBC TABLE 1004.1.2)	SQ. FT. (NET)	OCCUPANT LOAD FACTOR	OCCUPANT LOAD
H1	METAL SHADE STRUCTURE	Educational Classroom area	1,320 SF	20	66
			1,320 SF		66

OCCUPANCY CLASS

ROOM #	ROOM NAME	OCCUPANCY CLASS	SQ. FT. (NET)
H1	METAL SHADE STRUCTURE	E	1,320 SF
SS	FABRIC SHADE STRUCTURE	E	540 SF
TOTAL AREA (NET)			1,860 SF

PLUMBING ANALYSIS

PER CBC 2902.3.2 LOCATION OF TOILET FACILITIES IN OCCUPANCIES OTHER THAN MALLS, THE PATH OF TRAVEL TO SUCH FACILITIES SHALL NOT EXCEED A DISTANCE OF 500 FEET.

NEAREST ACCESSIBLE FACILITIES ARE LOCATED IN EXISTING BUILDING'S C AND G. SEE SITE PLAN FOR TRAVEL DISTANCE.

LEGEND

ILLUMINATED EXIT SIGN, SEE ELECTRICAL DRAWINGS.  
EXIT SIGNAGE TO ACCOMPANY ALL ILLUMINATED EXIT SIGNS.  
L = LOW LEVEL ILLUMINATED EXIT SIGN (WHERE OCCURS)

**WORKROOM**

A101   B	= ROOM NAME
1,500 SF	= ROOM NUMBER
B	= OCCUPANCY CLASS (CBC 302)
1,500 SF	= FLOOR AREA
10	= OCCUPANT LOAD
(15)	= OCCUPANT LOAD PLUS ANCILLARY SPACE WHERE OCCURS

**PATH OF EGRESS**

41 = OCCUPANT LOAD  
STARTING POINT OF PATH OF TRAVEL TO EXIT MARKED BY DOT AT THE BEGINNING OF EGRESS LINE

**PANIC HARDWARE DEVICE** - REFERENCE DOOR SCHEDULE AND HARDWARE GROUP

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DIV. OF THE STATE ARCHITECT  
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REVIEWED FOR  
SS ☒ FLS ☒ ACS ☒  
DATE: 3/26/2024

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ARCHITECT

ENGINEER

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REVISION HISTORY

NO.	REMARKS	DATE

DRAWING STATUS

☐ DSA PLAN CHECK  
☐ DSA BACK CHECK  
☐ BIDDING  
☐ CONSTRUCTION

KEY PLAN

WASHINGTON UNIFIED  
SCHOOL DISTRICT  
930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

PROJECT STATUS

WESTMORE OAKS ES  
ESSR III  
1504 FALLBROOK ST.  
WEST SACRAMENTO, CA 95691

CODE ANALYSIS FLOOR PLAN

Date  
03/12/2024

Application Number  
02-122280

Drawn  
Author

Checked  
Checker

Project Number  
22046

Drawing Number  
A0.6



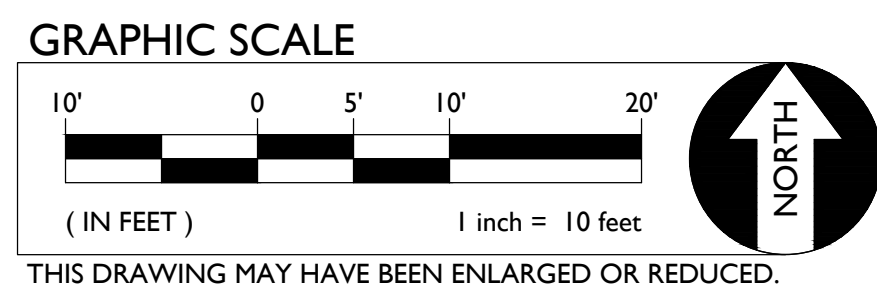
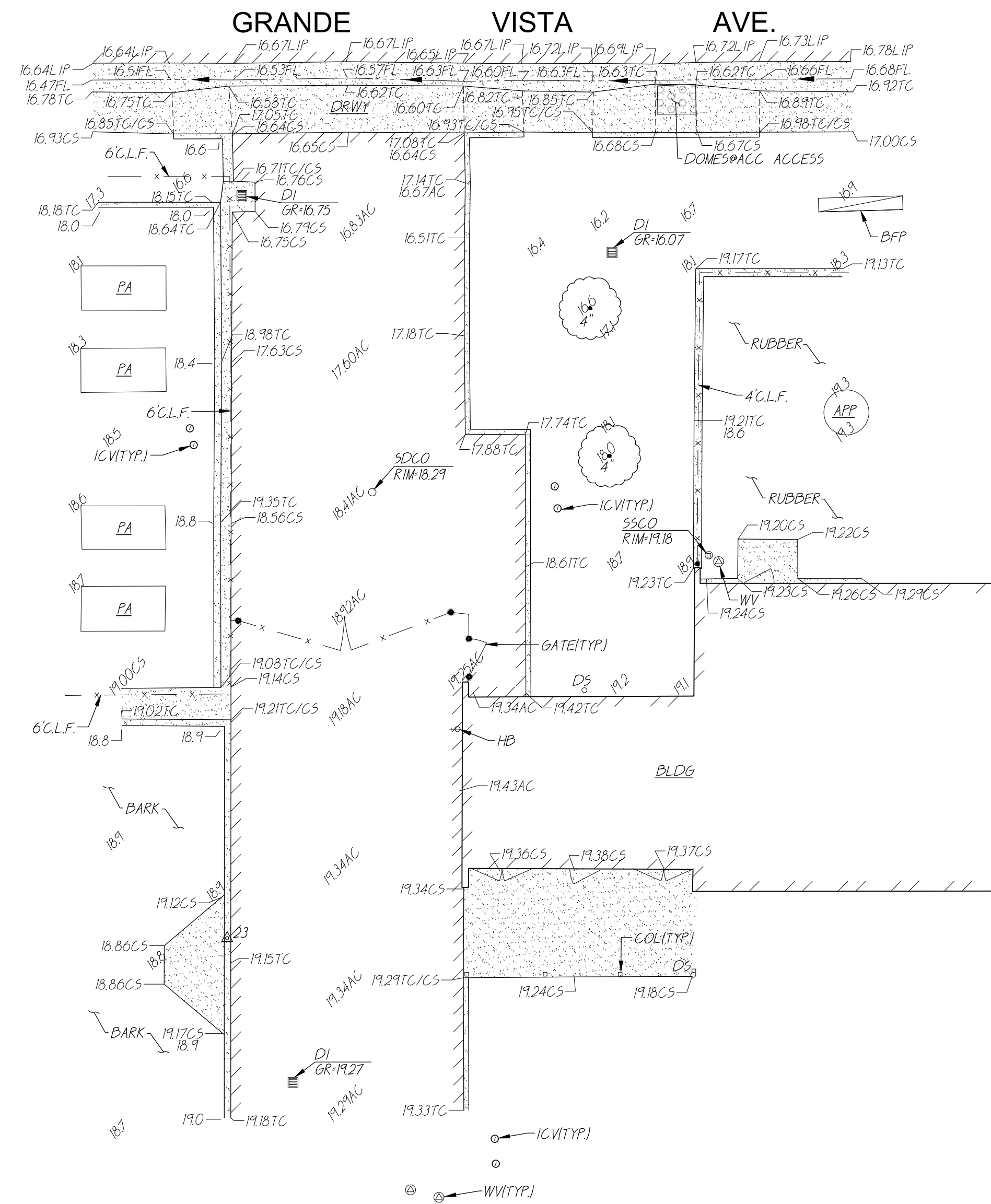
- PROPERTY LINE
- CENTERLINE
- EASEMENT
- PROPERTY CORNER FOUND AS NOTED
- PROPERTY CORNER NOTHING FOUND OR SET
- TEMPORARY BENCHMARK (SEE TBM LIST FOR INFO)
- SWALE OR DRAINAGE FLOW
- DRAINAGE FLOW
- FENCE (TYPE NOTED)
- TREE (SIZE/TYPE INDICATED)
- SLOPE
- CONTOUR
- CONCRETE SURFACE
- EDGE OF ASPHALT
- EDGE OF BUILDING
- SIGN
- POST OR BOLLARD
- GROUND ELEVATION
- ROAD SURFACE ELEVATION

NOTE: NOT ALL ABBREVIATIONS MAY BE USED ON THESE PLANS.

AC	ASPHALTIC CONCRETE
ACC	ACCESSIBLE
AD	AIR-CONDITIONING UNIT
ADN	AREA DRAIN
APN	ASSESSOR'S PARCEL NUMBER
APP	APPARATUS
DBALL	BALLETBALL POLE
BCM	BRASS CAP MONUMENT
BFP	BACK FLOW PREVENTER
BL	BLOCK
BLDG	BUILDING
BOL	BOLLARD
BOV	BLOW-OFF VALVE
BR	BRICK
BWF	BARBED WIRE FENCE
CA	COMMUNICATION
CATV	CABLE TELEVISION
CIP	CAPIED IRON PIPE
C/L	CENTERLINE
CFL	CHAIN LINK FENCE
CO	CORRUGATED METAL PIPE
CO	CLEANOUT
COL	COLUMN
CONC.	CONCRETE
COND.	CONDENSATE
CPT	CONTROL POINT FOUND
CPS	CONTROL POINT SET
CS	CONCRETE SURFACE
CS	CREATING FOUNTAIN
CG	COMPOSED GRANITE
DI	DROP INLET
DRWY	DRIVEWAY
DS	DOWNSPOUT
DWG	DRAWING
E	ELECTRIC
EWG	EDGE OF PAVEMENT
ESMT	EASEMENT
FA	FIRE ALARM
FDC	FIRE DEPARTMENT CONNECTION
FE	FINISHED FLOOR ELEVATION
FH	FIRE HYDRANT
FL	FLOWLINE
FO	FIBER OPTIC
FS	FIRE SERVICE
G	GAS
GB	GRADE BREAK
GR	GRATE
GRB	GROUND ROD BOX
GRD	GROUND ROD
GV	GAS VALVE
HB	HOSE BIBB
HDB	HOSE BIBB BOX
HDR	HEADER BOARD
HP	HIGH PRESSURE
HR	HANDRAIL
HVE	HIGH VOLTAGE ELECTRIC
HVF	HIGH VOLTAGE FENCE
IC	IN CONCRETE
ICP	IRRIGATION CONTROL PANEL
IR	IRRIGATION CONTROL VALVE
IRV	INVERT ELEVATION
IR	IRRIGATION
JP	JOINT UTILITY POLE
JT	JOINT TRENCH
LNDG	LANDING
LVE	LOW VOLTAGE ELECTRIC
M	MANHOLE
MS	MOW STRIP
MSC	METAL STORAGE CONTAINER
NTS	NOT TO SCALE
OH	OVERHEAD
OHANG	OPEN HANG
OP	OPEN IRON PIPE
P/L	PROPERTY LINE
PA	PLANTER AREA
PB	PARKING PUMPER
PH	POSTHOLE
PI	POST INDICATOR VALVE
PP	POWER POLE
PRE	PER RECORD INFORMATION
PRI	PUBLIC UTILITY EASEMENT
PVC	POLYVINYL CHLORIDE
R	RUBBER
RG	ROLLING GATE
RIM	MANHOLE RIM ELEVATION
RW	RIGHT OF WAY
RWD	RETAINING WALL
RWD	REDWOOD
RWL	RAIN WATER LEADER
SD	SIGNAL BOX
SD	STORM DRAIN
SDMH	STORM DRAIN MANHOLE
SIG	SIGNAL
SL	STREET LIGHT
SS	STREET LIGHT BOX
SS	SANITARY SEWER
SSMH	SANITARY SEWER CLEANOUT
SSW	SANITARY SEWER MANHOLE
STL	STEEL
T	TELEPHONE
TBALL	TETHER BALL POLE
TBM	TEMPORARY BENCHMARK
TDB	TOP OF CURB
TDR	TRENCH DRAIN
TEL	TELEPHONE POLE
TRW	TOP OF RETAINING WALL
UG	UNDERGROUND
UNK	UNKNOWN
V	VENT
VBALL	VOLLEYBALL
W	WATER
W	WITH
WO	WOOD
WF	WOOD FENCE
WLF	WROUGHT IRON FENCE
WRF	WOOD RAIL FENCE
WTF	TRANSFORMER
XWALK	CROSSWALK
YD	YARD DRAIN

NUMBER	DESCRIPTION	NORTHING	EASTING	ELEVATION
6	CP5 CHISELED "+"	9895.71	1077156	17.54
19	CPF NAIL+SHINER	10000.32	1101751	17.62
20	CP5 CHISELED "+"	1002153	1066255	16.59
21	CP5 CHISELED "+"	967369	1064736	18.77
22	CP5 CHISELED "+"	980780	1066029	19.02
23	CP5 CHISELED "+"	9894.92	1066029	19.12
28	CP5 CHISELED "+"	9856.38	1101789	17.72
32	CP5 CHISELED "+"	1003233	1062700	15.68
43	CPF CL INTERSEC	10000.13	10235.31	15.92
46	CP5 CHISELED "+"	9970.40	110112	17.44
47	CPF PK AT FLIGHTX	998251	10594.70	16.62

	STORM DRAIN LINE (SIZE + DIRECTION OF FLOW)
	STORM DRAIN LINE (RECORD INFORMATION)
	STORM DRAIN LINE (UNDERGROUND LOCATING)
	STORM DRAIN MANHOLE
	STORM DRAIN CLEANOUT
	DROP INLET
	AREA DRAIN
	RAIN WATER LEADER
	DOWNSPOUT
	SANITARY SEWER LINE (SIZE + DIRECTION OF FLOW)
	SANITARY SEWER LINE (RECORD INFORMATION)
	SANITARY SEWER LINE (UNDERGROUND LOCATING)
	SANITARY SEWER MANHOLE
	SANITARY SEWER CLEANOUT
	WATER LINE (SIZE INDICATED)
	WATER LINE (RECORD INFORMATION)
	WATER LINE (UNDERGROUND LOCATING)
	WATER MANHOLE
	WATER VALVE
	WATER METER
	WATER BOX
	IRRIGATION CONTROL VALVE
	FIRE HYDRANT
	BACKFLOW PREVENTER
	SPRINKLER
	HOSE BIBB
	OVERHEAD ELECTRIC LINE
	UNDERGROUND ELECTRIC LINE
	UNDERGROUND ELECTRIC LINE (RECORD INFORMATION)
	UNDERGROUND ELECTRIC LINE (UNDERGROUND LOCATING)
	ELECTRIC MANHOLE
	UTILITY POLE (WITH GUY WIRE)
	ELECTRIC METER
	ELECTRIC BOX
	STREET LIGHTING BOX
	OR LIGHT STANDARD
	SIGNAL LIGHT
	FLOOD LIGHT
	ELECTRICAL OUTLET
	GAS LINE (SIZE INDICATED)
	GAS LINE (RECORD INFORMATION)
	GAS LINE (UNDERGROUND LOCATING)
	GAS MANHOLE
	GAS VALVE
	GAS METER
	TELEPHONE LINE
	TELEPHONE LINE (RECORD INFORMATION)
	TELEPHONE LINE (UNDERGROUND LOCATING)
	STORM DRAIN BOX
	TRAFFIC SIGNAL BOX



BASIS OF BEARINGS:  
WESTMORE OAKS NO. 2,  
RECORDED IN 4 MAPS 74

NOTE:  
EXISTING UTILITIES BASED ON  
VISIBLE SURFACE STRUCTURES  
ONLY.

<div style="display: flex; justify-content: space-between;"> <div> <p><b>TITLE:</b></p> <p><b>PROJECT:</b></p> </div> <div style="text-align: center;"> <p><b>TOPOGRAPHIC SURVEY</b></p> <p><b>WESTMORE OAKS E.S. ESSR III</b></p> <p>AT</p> <p><b>1401 FALLBROOK ST.,</b></p> <p>WEST SACRAMENTO, YOLO CO., CALIFORNIA</p> <p>WASHINGTON U.S.D.,</p> </div> </div>		<p><b>WARREN CONSULTING ENGINEERS, INC.</b>          1117 WINDFIELD WAY, SUITE 110          EL DORADO HILLS, CA 95672 (916)985-1870</p>		<div style="border: 1px solid black; padding: 5px; margin-bottom: 5px;"> <p style="text-align: center; font-weight: bold; font-size: 0.8em;">IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT</p> <p style="text-align: center; font-weight: bold;">APP: 02-122280 INC:</p> <p style="text-align: center; font-weight: bold;">REVIEWED FOR:</p> <div style="display: flex; justify-content: space-around; font-size: 0.8em;"> <span>SS <input checked="" type="checkbox"/></span> <span>FLS <input checked="" type="checkbox"/></span> <span>ACS <input checked="" type="checkbox"/></span> </div> <p>DATE: <span style="float: right;">3/26/2024</span></p> </div> <table border="1" style="width: 100%; border-collapse: collapse; font-size: 0.8em;"> <tr> <td style="width: 15%;">DRAWN:</td> <td style="width: 15%;">JBT/GJS</td> <td colspan="2" style="width: 20%;">REVISIONS</td> <td colspan="2" style="width: 20%;">AGENCY APPROVAL</td> <td style="width: 10%;">A.P.N.</td> <td style="width: 10%;">058</td> </tr> <tr> <td>CHECKED:</td> <td>AT</td> <td style="width: 10%;">NO.</td> <td style="width: 10%;">DESCRIPTION</td> <td style="width: 10%;">DATE</td> <td style="width: 10%;">BY</td> <td style="width: 10%;">BENCHMARK NO.</td> <td style="width: 10%;">CI</td> </tr> <tr> <td>SURVEYED ON:</td> <td>8/13/23</td> <td></td> <td></td> <td></td> <td></td> <td>FOUND A 3" BRASS STANDARD MONUMENT OF WEST SACRAMENTO HORIZONTAL CONTROL</td> <td></td> </tr> <tr> <td>FINAL ISSUED:</td> <td>8/16/23</td> <td></td> <td></td> <td></td> <td></td> <td>30639, 1493, LOCATED C/L OF MOSSWOOD CIR. 204 SOUTHERLY OF THE C/L OF LAKEWOOD DR.</td> <td></td> </tr> </table>						DRAWN:	JBT/GJS	REVISIONS		AGENCY APPROVAL		A.P.N.	058	CHECKED:	AT	NO.	DESCRIPTION	DATE	BY	BENCHMARK NO.	CI	SURVEYED ON:	8/13/23					FOUND A 3" BRASS STANDARD MONUMENT OF WEST SACRAMENTO HORIZONTAL CONTROL		FINAL ISSUED:	8/16/23					30639, 1493, LOCATED C/L OF MOSSWOOD CIR. 204 SOUTHERLY OF THE C/L OF LAKEWOOD DR.	
				DRAWN:	JBT/GJS	REVISIONS		AGENCY APPROVAL		A.P.N.	058																														
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<p><b>SHEET NO.</b></p>		<table border="1" style="width: 100%; border-collapse: collapse; font-size: 0.8em;"> <tr> <td style="width: 15%;">HORIZONTAL SCALE:</td> <td style="width: 15%;">1"=10'</td> <td colspan="2" style="width: 20%;">VERTICAL SCALE:</td> <td colspan="2" style="width: 20%;">NA</td> <td style="width: 10%;">JOB NO.:</td> <td style="width: 10%;">23-120</td> </tr> </table>						HORIZONTAL SCALE:	1"=10'	VERTICAL SCALE:		NA		JOB NO.:	23-120																										
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## KEYNOTES

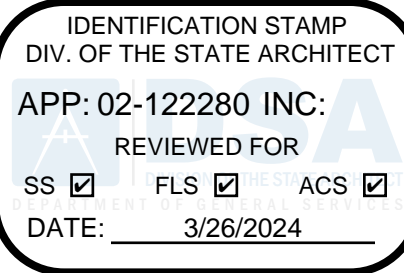
NUMBER	NOTE
10 121	(E) PARKING ENTRANCE SIGNAGE, DSA APP 02-117459
10 122	(E) FIRE LANE CURB, PAINTED RED, DSA APP 02-117459
13 303	NEW METAL SHADE STRUCTURE (SEE PG DRAWINGS)
32 222	(E) MAN GATE, DSA APP 02-117459 AND 02-117622
32 223	(E) MAN GATE, DSA APP 02-116930
32 240	(E) BICYCLE RACKS, DSA APP 02-117622

## GENERAL NOTES

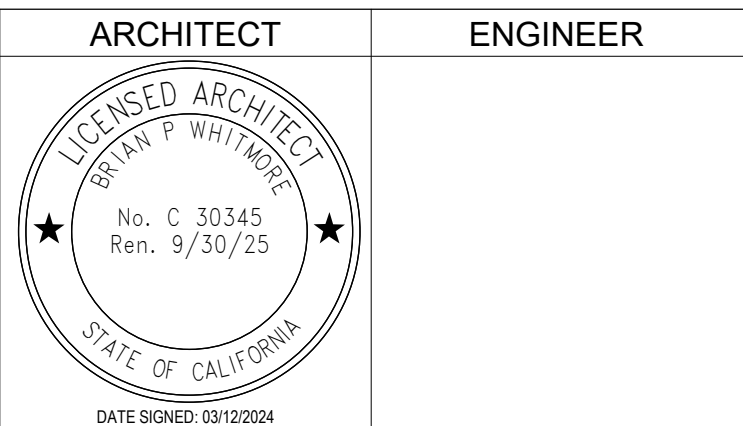
- CONTRACTOR IS RESPONSIBLE FOR 6'-0" HIGH TEMPORARY CONSTRUCTION BARRIER WITH VISION SCREEN AT STAGING, STORAGE AND CONSTRUCTION AREA WITH SIGNAGE EVERY 20'-0" TO WARN STUDENTS OF CONSTRUCTION AREA.
- CONTRACTOR TO BRING IN OFFICE TRAILER TO CONSTRUCTION AREA. CONTRACTOR SHALL ACCESS THE SITE FROM GRAND VISTA AVENUE. ANY DAMAGE TO FIRE LANE WILL BE AT THE CONTRACTOR'S EXPENSE.
- CONTRACTOR TO REPAIR BACK TO EXISTING CONDITIONS ALL LAYDOWN AREAS AT THE END OF CONSTRUCTION. THIS INCLUDES LANDSCAPE AREAS AND ANY BROKEN SPRINKLERS, VALVE BOXES, CONCRETE, ASPHALT, ETC.
- CONTRACTOR SHALL REPLACE, RECONSTRUCT AND REPAIR ALL EXISTING WORK THAT IS IMPACTED, DAMAGED, OR DESTROYED AS A RESULT OF ANY CONTRACTOR WORK INCLUDING, BUT NOT LIMITED TO, HARDSCAPING, SIDEWALKS, IRRIGATION SYSTEMS, LANDSCAPING, LAWNS, STRUCTURES AND UTILITIES - ALL TO THE SATISFACTION OF THE DISTRICT.
- WHERE ASPHALT OR CONCRETE IS BEING REPAATCHED, CONTRACTOR SHALL PROVIDE EVEN AND STRAIGHT LINE CUTS WITH 2'-FOOT STRAIGHT SLURRY SEAL SURFACE PATCH ON BOTH SIDES OF CUT.
- CONTRACTOR SHALL EXERCISE EXTREME CAUTION IN EXCAVATING AND TRENCHING ON SITE TO AVOID EXISTING DUCTS, PIPING OR CONDUITS, ETC., AND TO PREVENT HAZARDS TO PERSONNEL AND/OR DAMAGE TO EXISTING UNDERGROUND UTILITIES OR STRUCTURES WHETHER OR NOT SHOWN AND INSTALLED BY ANY OTHER CONTRACTS. THE ARCHITECT IS NOT RESPONSIBLE FOR THE LOCATION OF UNDERGROUND UTILITIES OR STRUCTURES WHETHER OR NOT SHOWN OR DETAILED AND INSTALLED BY ANY OTHER CONTRACTS. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT SHOULD SUCH UNIDENTIFIED CONDITIONS BE DISCOVERED. THESE DRAWINGS AND SPECIFICATIONS DO NOT INCLUDE THE NECESSARY ELEMENTS FOR CONSTRUCTION SAFETY.
- GATES IN PATH OF TRAVEL SHALL COMPLY WITH EXIT DOOR REQUIREMENTS WITH PROPER LEVER HARDWARE AND KICK PLATES.
- CONTRACTOR SHALL RE-ROUTE AND REPAIR ANY IRRIGATION LINES AND HEADS IN THE WAY OF NEW WORK TO ENSURE A FULLY FUNCTIONING SYSTEM AT THE END OF CONSTRUCTION.
- CONTRACTOR SHALL REQUEST A PRE-DEMOLITION MEETING WITH ARCHITECT AND DISTRICT PRIOR TO DEMOLITION OF THE EXISTING RUBBER SURFACING FOR NEW CANOPY PLACEMENT. ALL CUTS ARE TO BE STRAIGHT AND OF EQUAL SIZE FOR THE CANOPY POSTS.

## LEGEND

---	PROPERTY LINE
---	LIMIT OF WORK
---	LINE OF ROOF, SKYLIGHT, OR SOFFIT OVERHEAD - SHOWN DASHED
---	NOT IN SCOPE
+	FIRE HYDRANT



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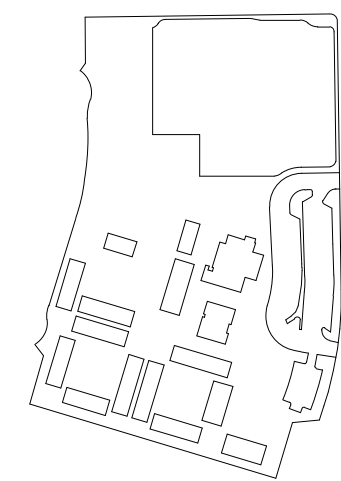


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NO.	REMARKS	DATE

DRAWING STATUS	DATE
<input type="radio"/> DSA PLAN CHECK	
<input type="radio"/> DSA BACK CHECK	
<input type="radio"/> BIDDING	
<input type="radio"/> CONSTRUCTION	

## KEY PLAN



WASHINGTON UNIFIED  
SCHOOL DISTRICT  
930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

## PROJECT STATUS

WESTMORE OAKS ES  
ESSR III  
1504 FALLBROOK ST.  
WEST SACRAMENTO, CA 95691

## SITE PLAN OVERALL

Date  
03/12/2024

Application Number  
02-122280

Drawn  
Author

Checked  
Checker

Project Number  
22046

Drawing Number  
02-122280

A1.1

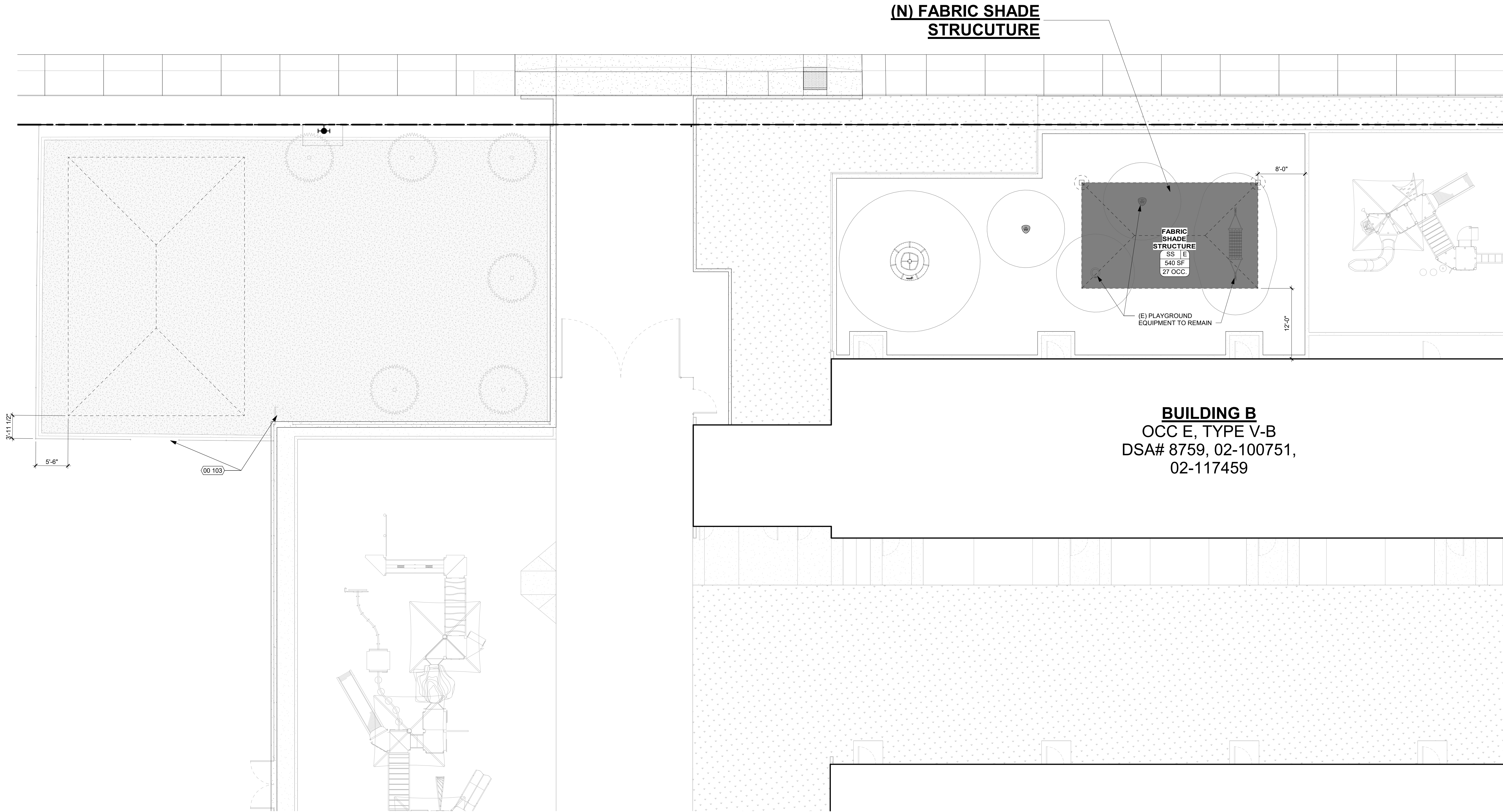
SITE PLAN

1" = 30'-0"

10



REF: 10 / A1.1



**BUILDING B**  
OCC E, TYPE V-B  
DSA# 8759, 02-100751,  
02-117459

KEY NOTES	
NUMBER	NOTE
00 103	REMOVE (E) GATE AND HARDWARE, SALVAGE TO DISTRICT

GENERAL NOTES	
1.	RUBBER SURFACING TO BE CUT NEATLY AND AS SMALL AS POSSIBLE TO INSTALL NEW FABRIC SHADE STRUCTURE FOOTINGS. CUTS FOR BOTH FOOTINGS SHOULD MATCH IN SIZE.
2.	REPLACEMENT RUBBER IS TO MATCH EXISTING RUBBER SURFACING - PLAYGROUND POURED-IN-PLACE SURFACING IN BRIGHT GREEN AND HUNTER GREEN (50/50 BLEND)

LEGEND	
<b>ROOM</b> A101   B 150 SF 1 OCC.	ROOM = ROOM NAME A101 = ROOM NUMBER B = OCCUPANCY GROUP 150 SF = FLOOR AREA IN SQUARE FEET 1 OCC = OCCUPANT LOAD (CBC TABLE 1004.5)
	BUILDING UNDER SCOPE OF WORK
	(N) SHADE STRUCTURE (UNDER THIS SCOPE OF WORK)

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122280 INC:  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☒  
DATE: 3/28/2024

STUDIO W  
ARCHITECTS

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ARCHITECT	ENGINEER

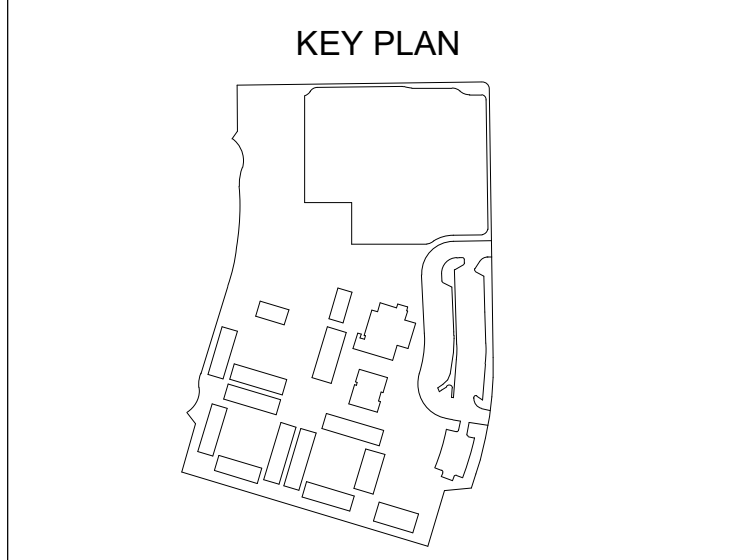
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NO.	REMARKS	DATE

DRAWING STATUS

☐ DSA PLAN CHECK  
☐ DSA BACK CHECK  
☐ BIDDING  
☐ CONSTRUCTION

DATE



WASHINGTON UNIFIED  
SCHOOL DISTRICT  
930 WESTACRE ROAD  
WEST SACRAMENTO, CA 95691

PROJECT STATUS

WESTMORE OAKS ES  
ESSR III  
1504 FALLBROOK ST.  
WEST SACRAMENTO, CA 95691

ENLARGED SHADE  
STRUCTURE PLAN

Date 03/12/2024	Project Number 22046
Application Number 02-122280	Drawing Number A1.2
Drawn Author	Checked Checker



DESIGN CRITERIA	
DESCRIPTION	DESIGN VALUES
<b>DEAD AND LIVE LOADS</b>	
ROOF LIVE LOAD	20 PSF
ROOF DEAD LOAD (SUPERIMPOSED ON FRAME)	5 PSF MAX
ROOF PANEL DEAD LOAD	M=1.1 PSF, G = 1.2 PSF, S = 1.3 PSF
COLLATERAL DEAD LOAD	M = 3.9 PSF, G = 3.8 PSF, S = 3.7 PSF
<b>ROOF LIVE LOAD, <math>L_r</math></b>	
	20 PSF
<b>ROOF SNOW LOAD</b>	
GROUND SNOW LOAD, $P_g$	20 PSF
RISK CATEGORY	II
ROOF SNOW LOAD: SLOPED, $P_s$	20 PSF
FOR SNOW LOAD CONDITIONS ONLY - SITE APPLICATION REVIEWER SHALL VERIFY THE STRUCTURE SHALL BE LOCATED AT LEAST 20 FEET FROM ANY ADJACENT STRUCTURE FOR SNOW DRIFT.	
SNOW LOAD SLOPE FACTOR, $C_e$	1.0
SNOW LOAD EXPOSURE FACTOR, $C_e$	1.0
SNOW LOAD IMPORTANCE FACTOR, $I_s$	1.0
THERMAL FACTOR, $C_t$	1.2
LOWEST ANTICIPATED SERVICE TEMPERATURE	30°
<b>WIND DESIGN</b>	
BASIC WIND SPEED (3 SECOND GUST), $V_{ult}$ , $V_{avg}$	100 MPH, 76 MPH
RISK CATEGORY	II
EXPOSURE CATEGORY	C
FACTORS: $K_d$ , $K_z$ , $K_e$	0.85, 1.0, 0.85
$q_h = 0.00256 K_d K_z K_e V^2$	18.50 PSF
$C_{hw}$ PER ASCE FIGURE 27.3-5 ROOF ANGLE 18.43° - CLEAR / OBSTRUCTED	CASE A (1.1 / -1.2) CASE B (0.01 / -0.69)
$C_{hw}$ PER ASCE FIGURE 27.3-5 ROOF ANGLE 18.43° - CLEAR / OBSTRUCTED	CASE A (-0.17 / -1.09) CASE B (-0.96 / -1.65)
$C_{hw}$ PER ASCE FIGURE 27.3-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED (< h)	CASE A (-0.8 / -1.2) CASE B (0.8 / 0.5)
$C_{hw}$ PER ASCE FIGURE 27.3-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED (> h, < 2h)	CASE A (-0.6 / -0.9) CASE B (0.5 / 0.5)
$C_{hw}$ PER ASCE FIGURE 27.3-7 PARALLEL TO RIDGE - CLEAR / OBSTRUCTED (> 2h)	CASE A (-0.3 / -0.6) CASE B (0.3 / 0.3)
COMPONENTS & CLADDING - $C_{cl}$ (PRESSURE/SUCTION) CLEAR / OBSTRUCTED	ZONE 3 - (2.29 / -2.11) / (1.0 / -3.0) ZONE 2 - (1.77 / -1.63) / (0.8 / -2.3) ZONE 1 - (1.15 / -1.05) / (0.5 / -1.5)
<b>SEISMIC DESIGN</b>	
LATERAL FORCE RESISTING SYSTEM	STEEL - ORDINARY CANTILEVER COLUMN
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE
SEISMIC IMPORTANCE FACTOR, $I_e$	1.0
SEISMIC SITE CLASS	D
$WCE_s$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, $S_s$	2.60
$WCE_s$ SPECTRAL RESPONSE ACCELERATION @ 0.2 s, $S_1$	0.90
SHORT PERIOD SITE COEFFICIENT, $F_a$	1.20
LONG PERIOD COEFFICIENT, $F_v$	1.70
FUNDAMENTAL PERIOD OF THE STRUCTURE, T (WORST CASE FOR ALL STRUCTURES)	0.152 s
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$	2.08 <input type="checkbox"/>
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, $S_{DS}$ - USED TO DETERMINE $C_s$ (WITH CAP PER ASCE 7 12.8.1.3) SOIL PROPERTIES MAY NOT BE CLASSIFIED AS SITE CLASS E	2.08 / 0.70 = 1.456 <input type="checkbox"/>
DESIGN SPECTRAL RESPONSE ACCELERATION AT 1-s PERIODS, $S_{D1}$	1.02
SEISMIC DESIGN CATEGORY	E
SITE SPECIFIC RESPONSE ANALYSIS NOT REQUIRED PER ASCE 7 11.4.8 EXCEPTION 2	$T_n = 0.49$ s <input type="checkbox"/> $T < 1.5 * T_n$
RESPONSE MODIFICATION FACTOR, R	1.25
OVERSTRENGTH FACTOR, $\Omega$	1.25
REDUNDANCY FACTOR, $\rho$	1.3
HORIZONTAL OR VERTICAL IRREGULARITIES	NONE
SEISMIC RESPONSE COEFFICIENT, $C_u$ (20° WIDE, 30° WIDE, 40° WIDE)	1.16 <input type="checkbox"/> 1.00 <input type="checkbox"/> 1.00 <input type="checkbox"/>
DESIGN BASE SHEAR, V (20° WIDE, 30° WIDE, 40° WIDE)	12.73 PSF <input type="checkbox"/> 13.41 PSF <input type="checkbox"/> 14.65 PSF <input type="checkbox"/>
ALLOWABLE SOIL BEARING FOR FOUNDATIONS	VARIABLES - SEE FOUNDATION CHARTS
FLOOD DESIGN - DESIGN IS ASSUMED TO NOT BE IN FLOOD HAZARD AREA	
IF PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED & SIGNED FROM A SOILS ENGINEER IS REQUIRED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED.	

STRUCTURAL SEPARATION		DEFLECTIONS ARE FOR (I) STRUCTURE		
ALL DEFLECTIONS SHOWN ALSO INCLUDE THE P-DELTA ROTATION PER IBC-7		SOIL CLASSES PER CBC TABLE 1806A.2		
MAXIMUM DRIFT $\delta_{h,ax}$	SIDE COLUMNS	Soil Class 5	Soil Class 4	Soil Class 3
20° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT)	(INCHES)	[ ] 2.40	[ ] 2.55	[ ] 2.65
30° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT)	(INCHES)	[ ] 2.15	[ ] 2.30	[ ] 2.40
40° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT)	(INCHES)	[ ] 2.20	[ ] 2.20	[ ] 2.30
MINIMUM SEPARATION ( $\delta_m = C_d \delta_{h,ax}$ ) $C_d = 1.25$				
20° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT)	(INCHES)	[ ] 3.00	[ ] 3.19	[ ] 3.31
30° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT)	(INCHES)	[ ] 2.69	[ ] 2.88	[ ] 3.00
40° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT)	(INCHES)	[ ] 2.75	[ ] 2.75	[ ] 2.88
MAXIMUM DRIFT $\delta_{h,ax}$	END COLUMNS	Soil Class 5	Soil Class 4	Soil Class 3
20° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT)	(INCHES)	[ ] 2.40	[ ] 2.55	[ ] 2.65
30° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT)	(INCHES)	[ ] 2.15	[ ] 2.30	[ ] 2.40
40° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT)	(INCHES)	[ ] 2.20	[ ] 2.20	[ ] 2.30
MINIMUM SEPARATION ( $\delta_m = C_d \delta_{h,ax}$ ) $C_d = 1.25$				
20° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT)	(INCHES)	[ ] 3.00	[ ] 3.19	[ ] 3.31
30° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT)	(INCHES)	[ ] 2.69	[ ] 2.88	[ ] 3.00
40° WIDE (8° EAVE HT, 10° EAVE HEIGHT, 12° EAVE HT)	(INCHES)	[ ] 2.75	[ ] 2.75	[ ] 2.88

INSTRUCTIONS FOR ARCHITECTS SUBMITTING THESE PRE-CHECKED DRAWINGS TO DSA:

BEFORE SUBMITTING THESE PRE-CHECKED DRAWINGS FOR YOUR PROJECT, FOLLOW THE STEPS BELOW TO PROPERLY DEFINE THE APPROVED OPTIONS:

STEP 1: SELECT FRAME DIMENSIONS FOR YOUR PROJECT  
-HP STRUCTURES UP TO 20' WIDE USE THE "RH 20" BASE FRAME  
-HP STRUCTURES UP TO 30' WIDE USE THE "RH 30" BASE FRAME  
-HP STRUCTURES UP TO 40' WIDE USE THE "RH 40" BASE FRAME  
-MAXIMUM WIDTH IS 40' (SEE "ARCHITECTURAL VIEWS" SHEET FOR REFERENCE)  
-THE 24", 44", 64", 84" AND 104" LENGTHS ARE SUGGESTED BECAUSE THEY ARE THE MOST COMMON (20" BAYS ARE THE MOST ECONOMIC)  
-FRAME LENGTHS ASSUME 2" OVERHANDS (UNO BY ARCHITECT - 2" MAX DIMENSION)

STEP 1	FRAME DIMENSIONS					
		SUGGESTED				OTHER
	FRAME WIDTH	[ ] 20'	[X] 30'	[ ] 40'		[ ] (40' MAX)
	FRAME LENGTH	[X] 44'	[ ] 64'	[ ] 84'	[ ] 104'	[ ] (NO MAX)

STEP 2: SELECT ROOF DECK FOR YOUR PROJECT  
-M REPRESENTS MCLEOD METAL "MULTI-RIB" ROOF PANEL  
-G REPRESENTS MCLEOD METAL "MEGA-RIB" ROOF PANEL  
-S REPRESENTS MCLEOD METAL "MEDALLION-LOCK" 16" STANDING SEAM ROOF PANEL

STEP 2	ROOF PANEL			
	ROOF PANEL TYPE	[ ] M	[ ] G	[X] S

STEP 3: IDENTIFY THE  $S_s$  ACCELERATION (g) FOR YOUR PROJECT  
- $S_s$  VALUE DETERMINES THE REQUIRED SEISMIC DESIGN FORCES  
- $S_s$  VALUE DEPENDS ON THE PROJECT'S GEOGRAPHICAL LOCATION (VALUES RANGE FROM 0.00 TO 3.73)  
-FIND  $S_s$  VALUES FOR YOUR PROJECT ON THE USGS WEBSITE (SEARCH INTERNET FOR "USGS SEISMIC DESIGN MAPS")

STEP 3	PROJECT SITE - $S_s$ ACCELERATION (g)	
	0.6	

STEP 4: IDENTIFY THE  $S_s$  REGION FOR YOUR PROJECT  
-THE REGIONS ARE DEPENDANT ON THE  $S_s$  VALUE DETERMINED IN STEP 3  
-THE  $S_s$  REGION DICTATES THE MAXIMUM DEAD LOAD PERMITTED ON THE FRAME

STEP 4	$S_s$ REGION		
	$S_s$ REGIONS		MAX DEAD LOAD
	DESCRIPTION		
	0 < $S_s$ <= 2.14		5 PSF
	2.14 < $S_s$ <= 2.50		5 PSF
	2.50 < $S_s$ <= 2.60		5 PSF

STEP 5: IDENTIFY THE ROOF DEAD LOAD FOR YOUR PROJECT  
- THE ROOF DEAD LOAD WILL ALWAYS BE INCLUDED  
- THE COLLATERAL LOAD REPRESENTS ADDITIONAL LOAD THAT CAN BE SUPPORTED BY THE FRAME  
- BE SURE THE TOTAL ROOF DEAD LOAD FOR YOUR PROJECT IS LESS THAN OR EQUAL TO THE MAX DEAD LOAD SHOWN IN STEP 4 FOR YOUR  $S_s$  VALUE  
-  $S_{ds}$  VALUE USED IN CALCULATION IS THE CAPPED  $S_{ds}$  (SEE DESIGN CRITERIA)

STEP 5	TOTAL ROOF DEAD LOAD		EXAMPLES
	ROOF DECK	1.3 PSF	M=1.1PSF; G=1.2PSF; S=1.3PSF (SEE STEP 2)
	COLLATERAL	0	LIGHTNING, FIRE SUPPRESSION, SOLAR PANELS, ETC.
STEP 5	TOTAL	1.3 PSF	ADD ROOF DECK AND COLLATERAL LOADS (MAX 5 PSF)

STEP 6: IDENTIFY THE FOUNDATION REQUIREMENTS FOR YOUR PROJECT  
-IDENTIFY SOIL CLASS FOR PROJECT SITE PER SITE SPECIFIC SOIL CONDITIONS  
-USE THIS TO SELECT CORRECT FOUNDATION SIZE ON FOUNDATION SHEET

STEP 6	FOUNDATION REQUIREMENTS		
	[ ] GEOTECHNICAL REPORT NOT REQUIRED		[ ] GEOTECHNICAL REPORT REQUIRED
	SOIL CLASS 5 (BEARING) 1500 PSF [X]	SOIL CLASS 4 (BEARING) 2000 PSF [ ]	SOIL CLASS 3 (BEARING) 3000 PSF [ ]
	SOIL CLASS 5 (LATERAL BEARING) 200 PSF/FT	SOIL CLASS 5 (LATERAL BEARING) 300 PSF/FT	SOIL CLASS 5 (LATERAL BEARING) 400 PSF/FT
	COHESION 130 PSF	FRICTION COEFFICIENT 0.25	FRICTION COEFFICIENT 0.30

- SELECT & VERIFY MINIMUM SEPARATION DISTANCE BETWEEN STRUCTURES

STEP 7: SELECT MISCELLANEOUS OPTIONS FOR YOUR PROJECT  
-MAXIMUM CLEAR HEIGHT IS 12'-0" (SEE "ARCHITECTURAL VIEWS" SHEET FOR REFERENCE)  
-MARK UP PC DRAWINGS WITH SIZE AND LOCATION OF CUTOUTS BEFORE SUBMITTING TO DSA

STEP 7	MISCELLANEOUS		DESIGN OPTIONS
	CLEAR HEIGHT	[ ] 8' [X] 10' [ ] 12' MAX	
	ELECTRICAL CUTOUTS	[ ] YES	[X] NO
	GUTTERS	[X] YES	[ ] NO

STEP 8: SELECT APPLICABLE SHEET INDEX FOR YOUR PROJECT  
-REFERENCE THE BASE FRAME (STEP 1) AND THE ROOF PANEL TYPE (STEP 2)  
-IDENTIFY THE APPLICABLE SHEET INDEX

SHEET INDEX													
STEP 8	BASE FRAME				RH 20			RH 30			RH 40		
	ROOF PANEL TYPE				M	G	S	M	G	S	M	G	S
	SELECT ONE				[ ]	[ ]	[ ]	[ ]	[ ]	[X]	[ ]	[ ]	[ ]
	GENERAL NOTES				LS1.0	LS1.0	LS1.0	LS1.0	LS1.0	LS1.0	LS1.0	LS1.0	LS1.0
	FOUNDATION PLAN				LS2.0	LS2.0	LS2.0	LS3.0	LS3.0	LS3.0	LS4.0	LS4.0	LS4.0
	FRAMING PLAN				LS2.1	LS2.1	LS2.1	LS3.1	LS3.1	LS3.1	LS4.1	LS4.1	LS4.1
	FRAME CONNECTION DETAILS				LS2.1	LS2.1	LS2.1	LS3.1	LS3.1	LS3.1	LS4.1	LS4.1	LS4.1
	ROOFING LAYOUT & DETAILS				LS2.2	LS2.2	LS2.2	LS3.2	LS3.2	LS3.2	LS4.2	LS4.2	LS4.2
	DSA 103 EXAMPLE				LS1.2	LS1.2	LS1.2	LS2.2	LS2.2	LS2.2	LS3.2	LS3.2	LS3.2
	MISC DESIGN OPTIONS				LS5.0	LS5.0	LS5.0	LS6.0	LS6.0	LS6.0	LS7.0	LS7.0	LS7.0
-													
-													
-													

STEP 9: INCLUDE APPLICABLE SHEETS WITH YOUR DSA SUBMITTAL  
-INCLUDE "MISC DESIGN OPTIONS" SHEET FOR PROJECTS WITHOUT ELECTRICAL CUTOUTS OR GUTTERS

PROJECT NAME:		SCHOOL DISTRICT:	
WESTMORE OAKS		WASHINGTON UNIFIED	
ELEMENTARY SCHOOL		SCHOOL DISTRICT	

STEP 11: CROSS OUT EXAMPLE 103 FORMS & INCORPORATE REQUIRED SPECIAL INSPECTIONS 103 FORMS THAT ARE PROJECT SPECIFIC

SITE SPECIFIC PARAMETERS  
INSTRUCTIONS: DESIGN PROFESSIONAL SHALL CHECK THE APPROPRIATE SELECTION BOXES BELOW AND ENTER THE DESIGN PARAMETERS APPLICABLE TO THE SPECIFIC PROJECT SITE

SNOW  
 $S_s = 0$  ☐  $S_s = 0$  ☐  
 $C_e = 0$  ☐

WIND  
 $V = 95$  mph < 150 mph  
 $I_t = 1.0$  ☐

EXPOSURE: c ☒ d ☐

SEISMIC  
[X] DESIGN BASED ON SITE CLASS D  
NO GEOTECHNICAL INVESTIGATION REQUIRED  
 $S_s = 0.6$   $F_a = 1.2$

[ ] DESIGN BASED ON SITE CLASS D  
GEOTECHNICAL INVESTIGATION PROVIDED

SITE CLASS: c ☐ d ☐ e ☐

$S_s =$   $F_a =$  PER ASCE 7-16 SUPPL 3, TABLE 11.4-1

[ ] DESIGN BASED ON SITE SPECIFIC GROUND MOTION HAZARD ANALYSIS  
PER CHAPTER 21 OF ASCE 7-16  
SHORT-PERIOD DESIGN SPECTRAL RESPONSE PARAMETER,  $S_{ds}$ , SHALL BE AS SPECIFIED IN GEOTECHNICAL INVESTIGATION

CGS APPROVAL REQUIRED  
NOT ELEGIBLE FOR OTC REVIEW

SITE CLASS: c ☐ d ☐ e ☐

$S_{ds} = F_a S_s = 0.6$  ( $S_{ds} = 2.08$  USED IN DESIGN, CONSERVATIVE)

[ ] SITE CLASS D  
 $S_{ds} = 2.08$  USED IN DESIGN

$C_s = 1.00$  USED IN DESIGN

SEISMIC DESIGN CATEGORY: D ☒ E ☐

\*SITE SPECIFIC  $S_{ds}$  VALUE BEFORE APPLYING REDUCTION  
ALLOWED BY ASCE 7 SECTION 12.8.1.3

ABBREVIATIONS:

ACI	AMERICAN CONCRETE INSTITUTE	MPH	MILES PER HOUR
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	M	MULTI-RIB ROOF PANEL (MCLEOD)
ASM	ASSEMBLY (INTERNAL REFERENCE)	NTS	NOT TO SCALE
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	NO	NUMBER
AWS	AMERICAN WELDING SOCIETY	OC	ON CENTER
CBC	CALIFORNIA BUILDING CODE	OSHA	OCCUPATIONAL HEALTH AND SAFETY ADMIN
CJP	COMPLETE JOINT PENETRATION	PCF	POUNDS PER CUBIC FOOT
CLR	CLEAR	PJ	PRETENSIONED JOINT
DEG	DEGREE	PLCS	PLACES
DIA	DIAMETER	PLT	PLATE
DM	DIMENSION	PSF	POUNDS PER SQUARE FOOT
DSA	DIVISION OF THE STATE ARCHITECT	PSI	POUNDS PER SQUARE INCH
EQ	EQUAL	QTY	QUANTITY
FT	FEET	REF	REFERENCE
GA	GAGE	SQ	SQUARE
IN	INCHES	SS	STANDING SEAM ROOF PANEL (MCLEOD)
KSI	KIPS PER SQUARE INCH	TYP	TYPICAL
MAX	MAXIMUM	UNO	UNLESS NOTED OTHERWISE
MIN	MINIMUM	USGS	U.S. GEOLOGICAL SURVEY
MISC	MISCELLANEOUS	W/	WITH

ARCHITECTURAL REQUIREMENTS

DESCRIPTION	DESIGN VALUES
TYPE OF CONSTRUCTION	II-B
OCCUPANCY CLASSIFICATION	A-3
NUMBER OF STORIES	1
FIRE SPRINKLER SYSTEM	NOT BY ICON/WEIGHT NOT INCLUDED IN DESIGN
MOST COMMON RH20 MIN/MAX SQ.FT (SEE STEP 1)	480/2,080
MOST COMMON RH30 MIN/MAX SQ.FT (SEE STEP 1)	720/3,120
MOST COMMON RH40 MIN/MAX SQ.FT (SEE STEP 1)	960/4,160
AREA OVER 4000 SQ.FT REQUIRES GEOHAZARD REPORT	
ALLOWABLE ARE FOR II-B / A-3 IS 9500 SQ.FT	

RELATED BUILDING CODES AND STANDARDS

TITLE 24 CODES:

2022 CALIFORNIA ADMINISTRATIVE CODE (CAC).....(PART 1, TITLE 24, CCR)  
2022 CALIFORNIA BUILDING CODE (CBC), PART 2, TITLE 24 CCR  
2022 CALIFORNIA ELECTRICAL CODE.....(PART 3, TITLE 24, CCR)  
2022 CALIFORNIA MECHANICAL CODE (CMC).....(PART 4, TITLE 24, CCR)  
2022 CALIFORNIA PLUMBING CODE (CPC).....(PART 5, TITLE 24, CCR)  
2022 CALIFORNIA ENERGY CODE.....(PART 6, TITLE 24, CCR)  
2022 CALIFORNIA FIRE CODE (CFC).....(PART 9, TITLE 24, CCR)  
2022 CALIFORNIA GREEN BUILDING STANDARDS CODE.....(PART 11, TITLE 24, CCR)  
2022 CALIFORNIA REFERENCE STANDARDS CODE.....(PART 12, TITLE 24, CCR)  
TITLE 19 CCR, PUBLIC SAFETY, STATE FIRE MARSHAL REGULATIONS

REFERENCE CODE SECTIONS FOR APPLICABLE STANDARDS:

2022 CBC, CHAPTER 35  
2022 CFC, CHAPTER 80

SCOPE OF WORK NARRATIVE

THESE DRAWINGS ILLUSTRATE THE FABRICATION AND INSTALLATION REQUIREMENTS FOR A FREE-STANDING PREFABRICATED STEEL SHADE STRUCTURE. THE ENTIRE STRUCTURAL SYSTEM IS COMPRISED OF HOLLOW STRUCTURAL STEEL MEMBERS SUPPORTED BY CONCRETE FOUNDATIONS. THE FLEXIBILITY INCLUDED HEREIN ALLOWS THE STRUCTURE TO COMPLY WITH A WIDE VARIETY OF PROJECT SITES AND LOADING REQUIREMENTS.

ICON STD RH/DSA-PC

DRAWN BY: JD

DATE: 7/25/2023

REV

REV DATE

JRMA  
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Oct 04, 2023

APPROVED  
DIV. OF THE STATE ARCHITECT  
APP: 04-122375-PC  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☒ CG ☐  
DATE: 10/10/2023

GENERAL INFO

CON  
Shelter Systems Inc

DISTINCTIVE STEEL SHELTERS  
WWW.CONSHelters.COM  
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GENERAL:

1. GENERAL NOTES AND TYPICAL DETAILS SHALL APPLY TO ALL PARTS OF THE JOB EXCEPT WHERE THEY MAY CONFLICT WITH DETAILS AND NOTES ON OTHER SHEETS. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER FOR THIS PROJECT.
2. WORK SHALL CONFORM TO THE REQUIREMENTS, AS AMENDED TO DATE, OF THE LATEST ADOPTED EDITION OF THE CBC, C.A.C. TITLE 24, AND ALL STATE AND FEDERAL REGULATIONS.
3. OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT PRIOR TO PROCEEDING WITH ANY WORK INVOLVED.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL CHECK ALL DIMENSIONS. ALL DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR THIS PROJECT AND BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
5. THESE CONSTRUCTION DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES, INCLUDING, BUT NOT LIMITED TO, BRACING, TEMPORARY SUPPORTS, AND SHORING. OBSERVATION VISIT TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER DURING THE CONSTRUCTION SHALL BE DISTINGUISHED FROM CONSTRUCTION AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ARCHITECT/ENGINEER, WHETHER OF MATERIAL OR WORK, ARE FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DOCUMENTS, BUT DO NOT GUARANTEE CONSTRUCTION.
6. ASTM DESIGNATIONS AND ALL STANDARDS REFER TO THE LATEST AMENDMENTS, EXCEPT AS AMENDED BY CBC CHAPTER 35.
7. CONFORM TO APPLICABLE CALIFORNIA CONSTRUCTION SAFETY REGULATIONS FOR ALL WORK PERFORMED DURING CONSTRUCTION. JOB SITE SAFETY IS STRICTLY THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ARCHITECT/ENGINEER OR OWNER.
8. THE ENGINEER AND THEIR CONSULTANTS SHALL HAVE NO RESPONSIBILITY FOR THE DISCOVERY, HANDLING, REMOVAL OR DISPOSAL OF HAZARDOUS MATERIALS AT THE PROJECT SITE, INCLUDING BUT NOT LIMITED TO ASBESTOS, ASBESTOS PRODUCTS, POLYCHLORINATED BIPHENYL (PCB) OR OTHER TOXIC SUBSTANCES.
9. SHOULD ANY CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, OR IF A CHANGE IN THE SCOPE OF WORK IS PROPOSED, A CONSTRUCTION CHANGE DOCUMENT DETAILING AND SPECIFYING THE REQUIRED CHANGE(S) SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK.
10. THE SCHOOL DISTRICT INSPECTOR ON RECORD SHALL INSPECT AND APPROVE THE ERECTED FRAME PRIOR TO ROOF INSTALLATION.
11. SEE REQUIREMENTS FOR LOCATION IN ANY FIRE HAZARD SEVERITY ZONE FOR WILDLAND URBAN INTERFACE AREAS (WUI) AS SPECIFIED IN THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE. PROVIDE PROTECTION AND DETAILS OF ALL AREAS COMPLYING WITH THE WUI REQUIREMENTS.
12. LOCATING THIS STRUCTURE CLOSER THAN 20 FEET TO OTHER STRUCTURES MAY AFFECT THE ALLOWABLE AREA FOR THE EXISTING CONSTRUCTION PER THE APPLICABLE VERSION OF THE CALIFORNIA BUILDING CODE.
13. VIEWS AND DETAILS ARE NOT DRAWN TO SCALE (UNLESS NOTED OTHERWISE). DO NOT SCALE THESE DRAWINGS.

STRUCTURAL AND MISCELLANEOUS STEEL:

1. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) SPECIFICATION MANUAL REFERENCED BY THE LATEST EDITION OF THE CALIFORNIA BUILDING CODE.
2. PIPE SECTIONS SHALL CONFORM TO ASTM A53, Fy = 35 KSI, GRADE B OR A501 UNLESS NOTED OTHERWISE.
3. STRUCTURAL TUBING (HSS SHAPES) SHALL CONFORM TO ASTM A-500, GRADE B (OR C), Fy = 46 KSI, MIN.
4. IF MATERIAL AVAILABILITY IS LIMITED, MEMBER THICKNESS CAN BE INCREASED BEYOND WHAT IS SHOWN IN THESE DRAWINGS (MAXIMUM INCREASE OF 1/8").
5. ALL CHANNELS, ANGLES, AND MISC. STEEL SHALL CONFORM TO ASTM A-36, Fy = 36 KSI.
6. ALL PLATE STEEL SHALL CONFORM TO ASTM A-572, Fy= 50 KSI.
7. ALL COLD FORM STEEL SHALL CONFORM TO ASTM A-653, CS = TYPE B, Fy = 50 KSI Fu = 65 KSI
8. STRUCTURAL STEEL AND DECK SHALL BE IDENTIFIED FOR CONFORMITY PER CBC 2202A.1.
9. ALL ROOF DECKS SHALL HAVE KYNAR 500 METAL COATING.
10. ALL ROOF DECKS SHALL CONFORM TO ASTM A-792, Fy = 50 KSI.
11. ALL BASE CONNECTIONS ARE A PART OF THE LATERAL FORCE RESISTING SYSTEM

NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

1. PER TITLE 24, PART 1, SECTION 4-316(a) OF THE CALIFORNIA CODE OF REGULATIONS, THIS NOTICE SHALL BE GIVEN TO DSA PRIOR TO THE APPROVAL OF PLANS AND SPECIFICATIONS.
2. FOR THE SITE SPECIFIC PROJECT, J. R. MILLER & ASSOCIATES IS NOT THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE.
3. FOR THE SITE SPECIFIC PROJECT, J.R. MILLER & ASSOCIATES' RESPONSIBILITY IS LIMITED TO THE PREPARATION OF THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC ONLY.
4. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM J.R. MILLER & ASSOCIATES' RESPONSIBILITY FOR THE SITE SPECIFIC PROJECT.
5. ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING SHALL BE DELEGATED TO A QUALIFIED ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE, BUT ARE NOT LIMITED TO, STRUCTURAL OBSERVATION OF CONSTRUCTION, REVIEW OF INSPECTION REPORTS, AND SIGNING OFF OF THE VERIFIED REPORT FOR COMPLETED WORK.
6. J.R. MILLER & ASSOCIATES WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS AND SPECIFICATIONS FOR THE SHELTERS OF THIS PC WHICH ARISE DURING PLAN REVIEW AND CONSTRUCTION.

CONSTRUCTION NOTES

1. A DSA-CERTIFIED CLASS 3 (MINIMUM) PROJECT INSPECTOR IS REQUIRED FOR THIS PROJECT.
2. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE DOCUMENT (CCD), APPROVED BY DSA, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR.
3. A "DSA CERTIFIED" PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY DSA SHALL PROVIDE CONTINUOUS INSPECTION OF WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.
4. A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED TESTS AND INSPECTIONS FOR THE PROJECT.
5. THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS ARE THAT ALL THE WORK OF THE ALTERATION, REHABILITATION OR RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24, CCR. SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHICH IS NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT (CCD), OR A SEPARATE SET OF PLANS AND SPECIFICATIONS, DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK. (SECTION 4-317(c), PART 1, TITLE 24, CCR)
6. GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS SHALL COMPLY WITH ALL LOCAL ORDINANCES

WELDING:

1. ALL WELDING SHALL COMPLY WITH AWS D1.1 SPECIFICATIONS AND SHALL BE DONE BY AWS QUALIFIED WELDERS CERTIFIED FOR THE TYPE OF WELDING TO BE PERFORMED AS REQUIRED BY DSA.
2. ALL WELDING SHALL BE DONE BY GAS METAL ARC PROCESS WITH E70XX ELECTRODES. FLUX CORE ARC WELD SHALL CONFORM TO CHARPY NOTCH TOUGHNESS RATING OF 20 ft-lb-in ( 0' ft).
3. ALL WELDING SHALL BE DONE IN THE SHOP WITH REQUIRED INSPECTION, PRE-APPROVED BY DSA, TO ENSURE PROPER MATERIAL ID AND WELDING.
4. WELD FILLER METAL MANUFACTURER SHALL PROVIDE WRITTEN CERTIFICATION OF COMPLIANCE WITH CODE AND SPECIFICATIONS.

BOLTING:

1. ALL BOLTS SHOWN ON THESE DRAWINGS ARE HOT DIPPED GALVANIZED ASTM F3125 GRADE A325 HIGH STRENGTH BOLTS (UNF), WITH THE NUTS CONFORMING TO HOT DIPPED GALVANIZED ASTM A-563 GRADE DH.
2. HIGH STRENGTH BOLTS SHALL BE VERIFIED AND INSPECTED PER CBC 1705A2.1.
3. BEFORE ERECTING THE FRAME, VERIFY ALL BOLTS AND NUTS ARE CLEAN OF DEBRIS AND BURRS – INCLUDING THE HARDWARE ALREADY FASTENED INSIDE THE MEMBERS. CHASING SOME OF THE BOLTS AND NUTS MAY BE REQUIRED.
4. HARDENED STEEL WASHERS SHALL CONFORM TO ASTM F-436.
5. THE BOLTING INSTALLATION REQUIREMENTS OUTLINED BELOW ARE CRITICAL TO THE STRUCTURE'S DESIGN AND PERFORMANCE. THE INSTALLER IS REQUIRED TO COORDINATE THIS PHASE OF CONSTRUCTION WITH THE SPECIAL BOLTING INSPECTOR AND THE INSPECTOR OF RECORD PRIOR TO THE ERECTION OF THE FRAME.  
BE INSTALLED AND INSPECTED PER THE APPLICABLE VERSION OF AISC'S  
USING HIGH-STRENGTH BOLTS\*, CBC 1705A2.1; AISC 341-16 (F); AISC 360-16 NS6.  
A) PRETENSIONED JOINTS MUST BE INSTALLED AND INSPECTED TO MEET ONE OF THE FOLLOWING REQUIREMENTS:  
1. TURN-OF-NUT PRETENSIONING: PER SECTION 8.2.1 OF THE SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS, WASHERS ARE NOT REQUIRED FOR THIS METHOD. THE NUT OR HEAD SHALL BE ROTATED AS SPECIFIED IN TABLE 8.2. THE PART NOT TURNED SHALL BE PREVENTED FROM ROTATING.  
2. CALIBRATED WRENCH: PER THE SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS, WASHERS ARE REQUIRED (NOT SUPPLIED BY KCM) THESE SHALL BE INSTALLED PER THE INSTALLATION TORQUE DETERMINED IN THE PRE-INSTALLATION VERIFICATION OF THE FASTENER ASSEMBLY PER SECTION 7. THE PART NOT TURNED SHALL BE PREVENTED FROM ROTATING.  
3. IDENTIFIED ON THE FRAME CONNECTION DETAILS WITH "PT REQUIRED"  
B) ALL OTHER JOINTS MUST BE INSTALLED AND INSPECTED TO MEET THE REQUIREMENTS OF THE SHUCK-TIGHTENED JOINTS. SHUCK TIGHT CONDITION EXISTS WHEN ALL PILES IN A CONNECTION HAVE BEEN PULLED INTO FIRM CONTACT BY THE BOLTS IN THE JOINT AND ALL OF THE BOLTS IN THE JOINT HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT REMOVAL OF THE NUTS WITHOUT THE USE OF A WRENCH.

FOUNDATIONS:

1. ALLOWABLE SOIL PRESSURES ASSUME CLASS 5 SOIL CLASSIFICATION PER CBC TABLE 1806A, UNLESS NOTED OTHERWISE. PASSIVE PRESSURE IS ASSUMED TO START 12" BELOW TOP OF FOOTING.
2. PER CBC SECTION 1803A.2, GEOTECHNICAL REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONES OR SEISMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS. ALLOWABLE FOUNDATION AND LATERAL SOIL PRESSURE VALUES MAY BE DETERMINED FROM TABLE 1806A.2.
3. FILL AND BACKFILL SHALL BE COMPACTED TO 95% OF MAX. DENSITY IN ACCORDANCE WITH ASTM TEST METHOD D-1557 OR AS RECOMMENDED BY THE GEO-TECH ENGINEER. FLOODING NOT PERMITTED.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING, ETC. NECESSARY TO SUPPORT CUT AND/OR FILL BANKS DURING EXCAVATION, AND FORMING AND PLACEMENT OF CONCRETE.
5. MINIMUM SETBACK FROM TOE OF SLOPE ON AN ASCENDING SLOPE SHALL BE 15 FEET AND MINIMUM SETBACK FROM TOE OF SLOPE ON A DESCENDING SLOPE SHALL BE 40 FEET
6. PER CBC SECTION 1803A.6, GEOHAZARD REPORTS ARE NOT REQUIRED FOR ONE-STORY LIGHT-STEEL FRAME BUILDINGS OF TYPE II CONSTRUCTION AND 4,000 SQUARE FOOT OR LESS IN FLOOR AREA AND NOT LOCATED WITHIN EARTHQUAKE FAULT ZONES OR SEISMIC HAZARD ZONES AS SHOWN ON THE MOST RECENT MAPS PUBLISHED BY THE CGS.
7. GEOHAZARD REPORTS ARE TO COMPLY WITH DSA IR A-4 PER IR-7 SECTION 1.8
8. SITE SPECIFIC GEOTECHNICAL REPORT IS REQUIRED AT THE TIME OF SITE APPLICATION IF USING OTHER THAN CLASS 5 SOIL PER DSA IR PC-7.
9. LATERAL BEARING HAS BEEN INCREASED PER CBC 1806A.3.4 FOR THE 1/2" DEFLECTION & HAS BEEN DESIGNED FOR P-DELTA EFFECTS. NO 1/3 INCREASE HAS BEEN APPLIED.
10. MINIMUM CLEARANCE BETWEEN PIERS SHALL BE 8'-0".

CONCRETE:

1. MIX DESIGN REQUIREMENTS: (NORMAL WEIGHT CONCRETE)

STRENGTH Pci (28 DAYS)	W/C RATIO (NON-AIR ENTRAINMENT)	W/C RATIO (AIR ENTRAINMENT)	SUMP (ft)	UNIT WEIGHT (NORMAL WEIGHT)
5000 PSI	0.44	0.35	3'	150 PCF

2. CONCRETE MIX DESIGN PARAMETERS ARE GOOD FOR EXPOSURE CATEGORIES F0, F1 & F2. THE AIR ENTRAINMENT FOR THESE CATEGORIES SHALL BE AS FOLLOWS: F0-0, F1-4.5, F2-6.
3. CHANGES TO THE MIX DESIGN MUST BE APPROVED BY THE ENGINEER OR ARCHITECT OF RECORD AND DSA.
4. AGGREGATES SHALL CONFORM TO THE ASTM C-33 WITH PROVEN SHRINKAGE CHARACTERISTICS OF LESS THAN 0.005. MAX AGGREGATE SIZE = 1".
5. CEMENT SHALL CONFORM TO ASTM C-150 (TYPE V) UNLESS NOTED OTHERWISE ON THE DRAWINGS.
6. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF FIVE DAYS AFTER PLACEMENT. ALTERNATE METHODS WILL BE APPROVED IF SATISFACTORY PERFORMANCE CAN BE ASSURED.
7. CONCRETE SHALL NOT FREE FALL MORE THAN FIVE FEET.
8. CONCRETE DURABILITY SHALL BE PER CBC 1904A.1, ACI 318-19, CHAPTER 19.
9. CONCRETE SHALL BE TESTED PER CBC 1903A, TABLE 1705A.3, AND ACI 318-19, SECTION 26.12.
10. NO ADMIXTURE SHALL CONTAIN CALCIUM CHLORIDE.

REINFORCING STEEL:

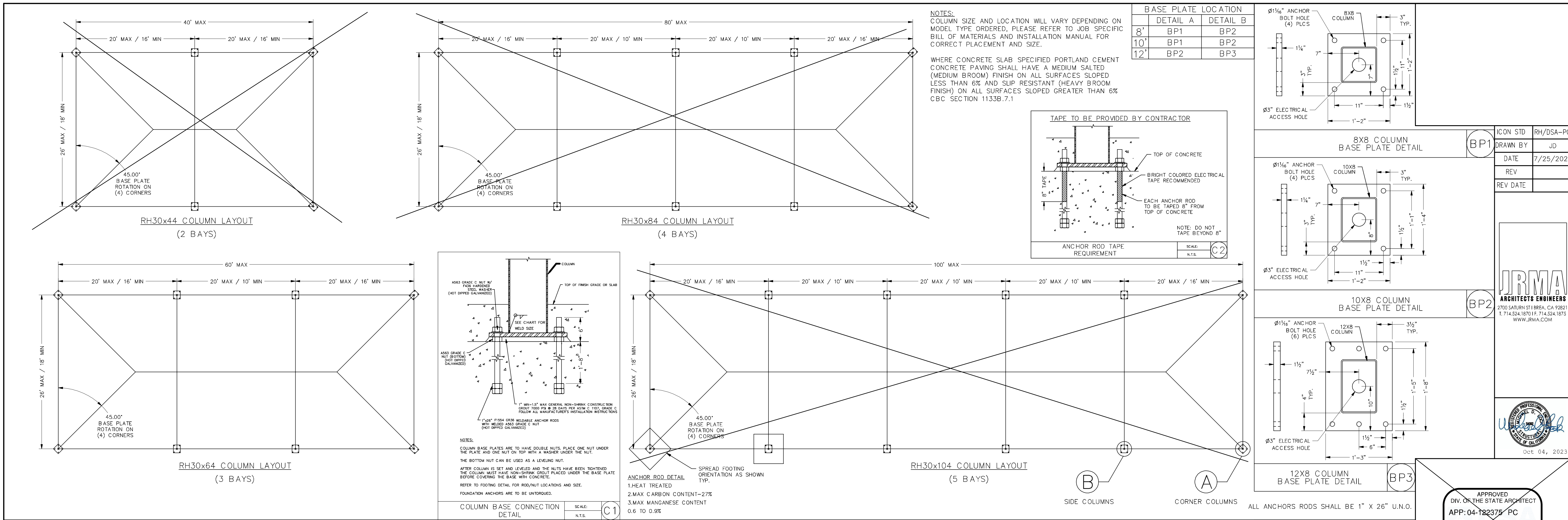
1. REINFORCING STEEL SHALL BE DEFORMED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A-615, AS FOLLOWS:  
OR 60: (#4 BARS AND LARGER)  
OR 40: (#3 BARS)  
2. DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS SHALL CONFORM TO THE ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCING CONCRETE STRUCTURES."  
3. MIN. COVER FOR CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:  
A. CAST AGAINST EARTH .....3"  
B. CAST AGAINST FORM BELOW GRADE .....2"  
C. FORMED SLABS (#11 BAR & SMALLER).....3/4"  
D. SLABS ON GRADE (FROM TOP OF SLAB).....1"  
4. BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIAL LIKELY TO IMPAIR BOND. BENDS SHALL BE MADE COLD.  
5. REINFORCING SHALL BE LAP SPLICED PER ACI 318-19, SECTION 25.5.  
6. PRIOR TO PLACING OF CONCRETE, REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE WELL SECURED IN POSITION.  
7. WELDING OF REINFORCING IS NOT ALLOWED.  
8. REINFORCING STEEL SHALL BE INSPECTED PER CBC 1705A.3.

POWDER-COAT FINISH SYSTEM:

ALL BUILDINGS THAT HAVE A POWDER-COATED FINISH SHALL MEET THE FOLLOWING SPECIFICATIONS:

1. THE STEEL FRAME (HSS SECTIONS, COLD FORMED & PLATE STEEL) SHALL BE SHOT-BLASTED TO A NEAR WHITE CONDITION PER SSPC-10 SPECIFICATIONS.
2. THE STEEL SHALL BE WASHED IN A ZINC PHOSPHATE IN AN MINIMUM THREE STAGE ELECTRO DEPOSITION PRE-TREATMENT PROCESS.
3. IMMEDIATELY FOLLOWING PRE-TREATMENT THE STEEL SHALL BE TOTALLY COATED IN AN EPOXY PRIMER TO A UNIFORM THICKNESS OF A MINIMUM OF 0.7 TO 0.9 MILS. THE E-COATING SHALL PROVIDE A MINIMUM OF 1000 HOURS OF SALT SPRAY CORROSION PROTECTION TO THE STEEL.
4. THE STEEL SHALL THEN HAVE A IGIC POLYESTER COLOR COAT APPLIED OVER THE E-COATED SURFACE.
5. THE FINISH THICKNESS OF THESE APPLICATIONS SHALL BE A MINIMUM OF 8 TO 12 MILS.
6. ALL CARBON STEEL MEMBERS (COLUMNS, BEAMS, PLATES, & COLD FORMED STEEL ETC.) NOT POWDER-COATED SHALL BE PAINTED WITH PRIME COAT PER THE "AISC CODE OF STANDARD PRACTICE" AND THE "AISC SPECIFICATION SECTION M3" UNLESS NOTED OTHERWISE).





**RH30 - PIER**

8' height - Corner Columns				8' height - Corner Columns				8' height - Corner Columns			
Soil Class 3 - 4400 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
30	134	8	6	30	99	6	6	30	99	6	6

8' height - Side Columns				8' height - Side Columns				8' height - Side Columns			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
36	144	8	6	30	132	8	6	30	118	8	6

8' Eave - 1500 psf [ ]				8' Eave - 2000 psf [ ]				8' Eave - 3000 psf [ ]			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
36	130	8	6	30	100	8	6	30	92	8	6

10' height - Corner Columns				10' height - Corner Columns				10' height - Corner Columns			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
36	130	8	6	30	100	8	6	30	92	8	6

10' height - Side Columns				10' height - Side Columns				10' height - Side Columns			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
36	130	8	6	30	100	8	6	30	92	8	6

10' Eave - 1500 psf [ ]				10' Eave - 2000 psf [ ]				10' Eave - 3000 psf [ ]			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
36	130	8	6	30	100	8	6	30	92	8	6

12' height - Corner Columns				12' height - Corner Columns				12' height - Corner Columns			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
36	130	8	6	30	100	8	6	30	92	8	6

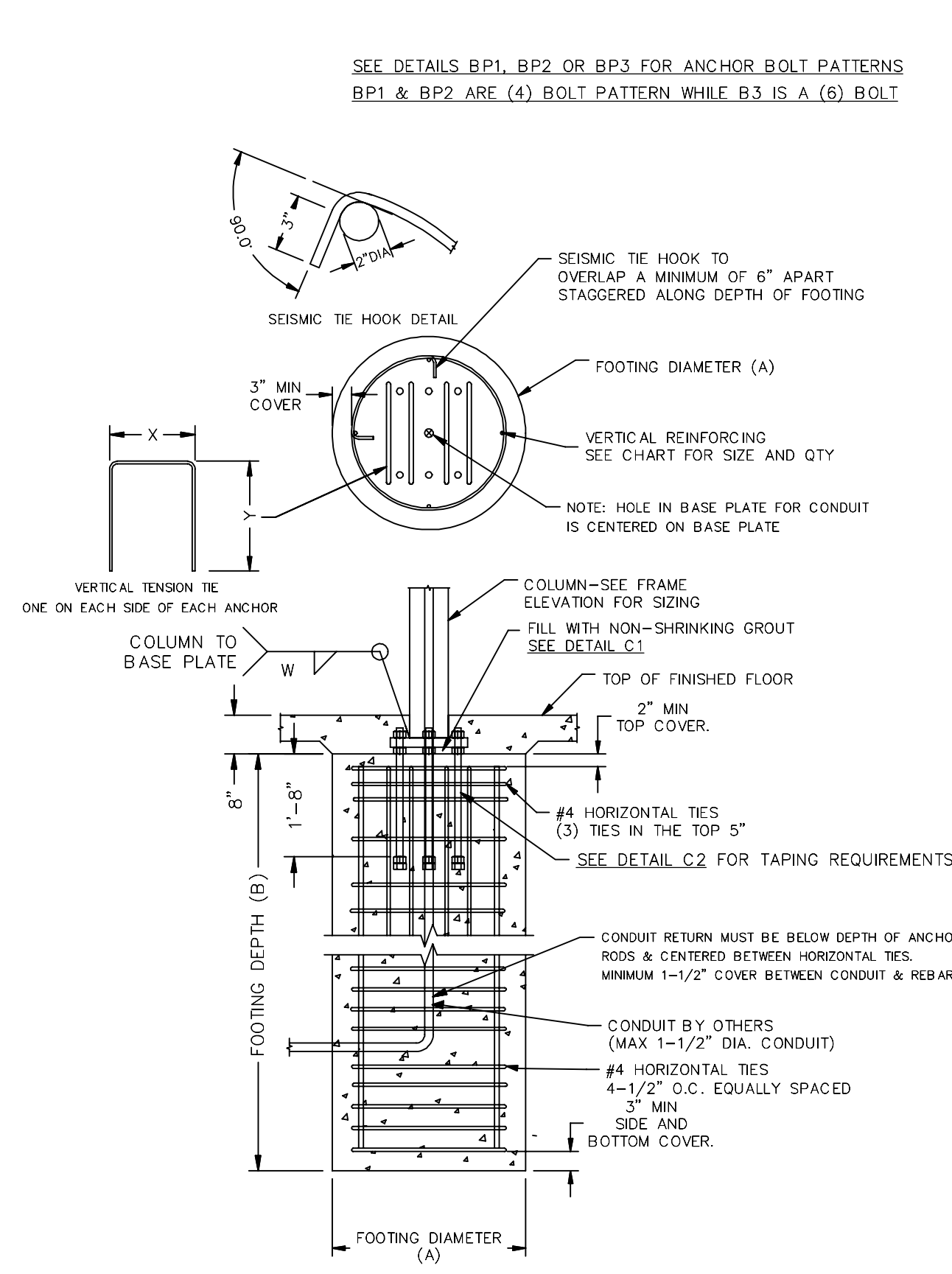
12' height - Side Columns				12' height - Side Columns				12' height - Side Columns			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
36	130	8	6	30	100	8	6	30	92	8	6

12' Eave - 1500 psf [ ]				12' Eave - 2000 psf [ ]				12' Eave - 3000 psf [ ]			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
36	130	8	6	30	100	8	6	30	92	8	6

12' height - Corner Columns				12' height - Corner Columns				12' height - Corner Columns			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
36	130	8	6	30	100	8	6	30	92	8	6

12' height - Side Columns				12' height - Side Columns				12' height - Side Columns			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
36	130	8	6	30	100	8	6	30	92	8	6

12' Eave - 1500 psf [ ]				12' Eave - 2000 psf [ ]				12' Eave - 3000 psf [ ]			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
36	130	8	6	30	100	8	6	30	92	8	6



**RH30 - SPREAD**

8' height - Corner Columns				8' height - Corner Columns				8' height - Corner Columns			
Soil Class 3 - 4400 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
60	90	8	6	57	90	6	6	57	90	6	6

8' height - Side Columns				8' height - Side Columns				8' height - Side Columns			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
64	90	8	6	63	90	6	6	63	90	6	6

8' Eave - 1500 psf [ ]				8' Eave - 2000 psf [ ]				8' Eave - 3000 psf [ ]			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
64	90	8	6	63	90	6	6	63	90	6	6

10' height - Corner Columns				10' height - Corner Columns				10' height - Corner Columns			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
66	90	8	6	63	90	6	6	63	90	6	6

10' height - Side Columns				10' height - Side Columns				10' height - Side Columns			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
66	90	8	6	63	90	6	6	63	90	6	6

10' Eave - 1500 psf [ ]				10' Eave - 2000 psf [ ]				10' Eave - 3000 psf [ ]			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
66	90	8	6	63	90	6	6	63	90	6	6

12' height - Corner Columns				12' height - Corner Columns				12' height - Corner Columns			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
81	90	8	6	79	90	6	6	79	90	6	6

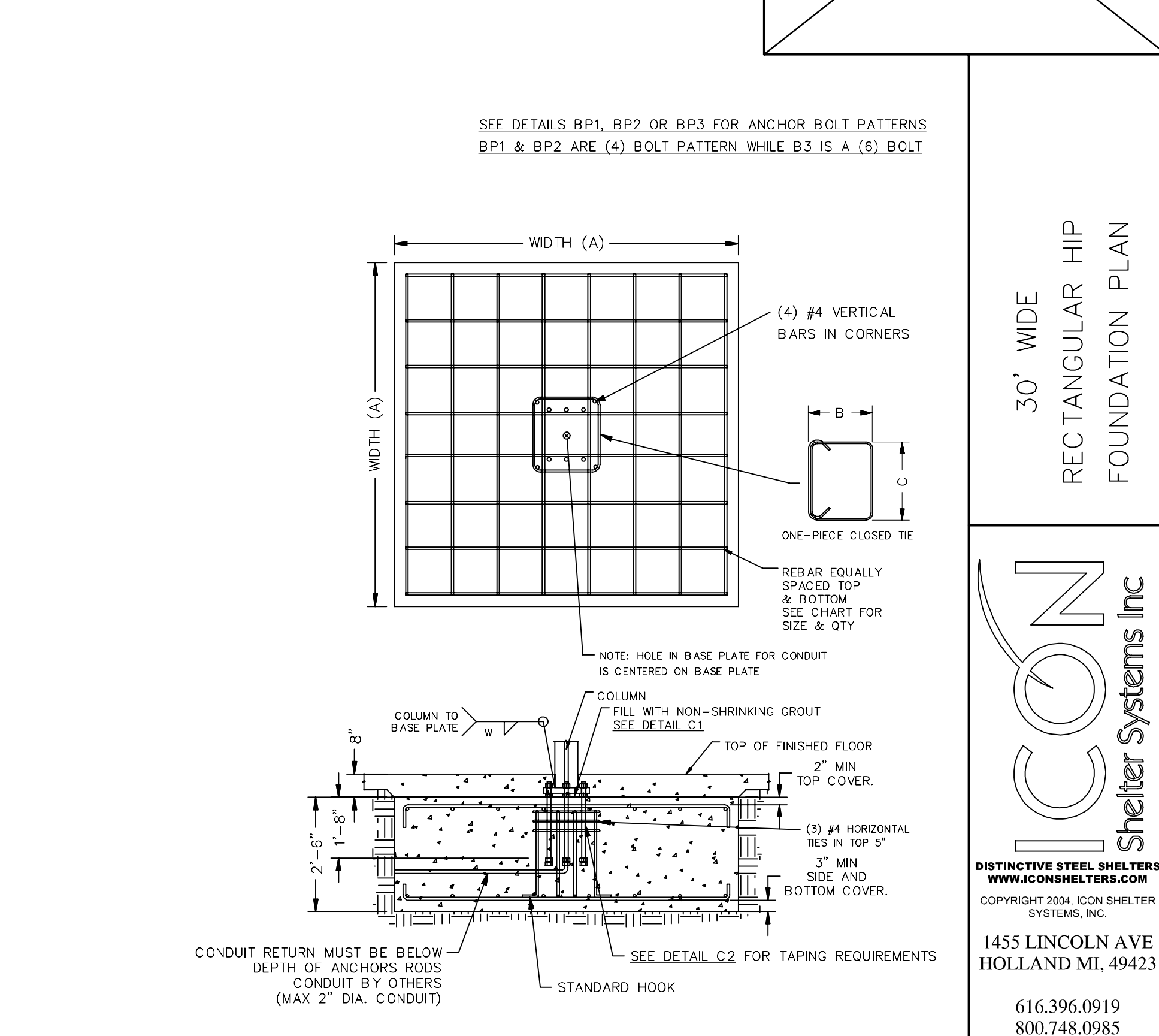
12' height - Side Columns				12' height - Side Columns				12' height - Side Columns			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
84	90	8	6	78	90	6	6	78	90	6	6

12' Eave - 1500 psf [ ]				12' Eave - 2000 psf [ ]				12' Eave - 3000 psf [ ]			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
84	90	8	6	78	90	6	6	78	90	6	6

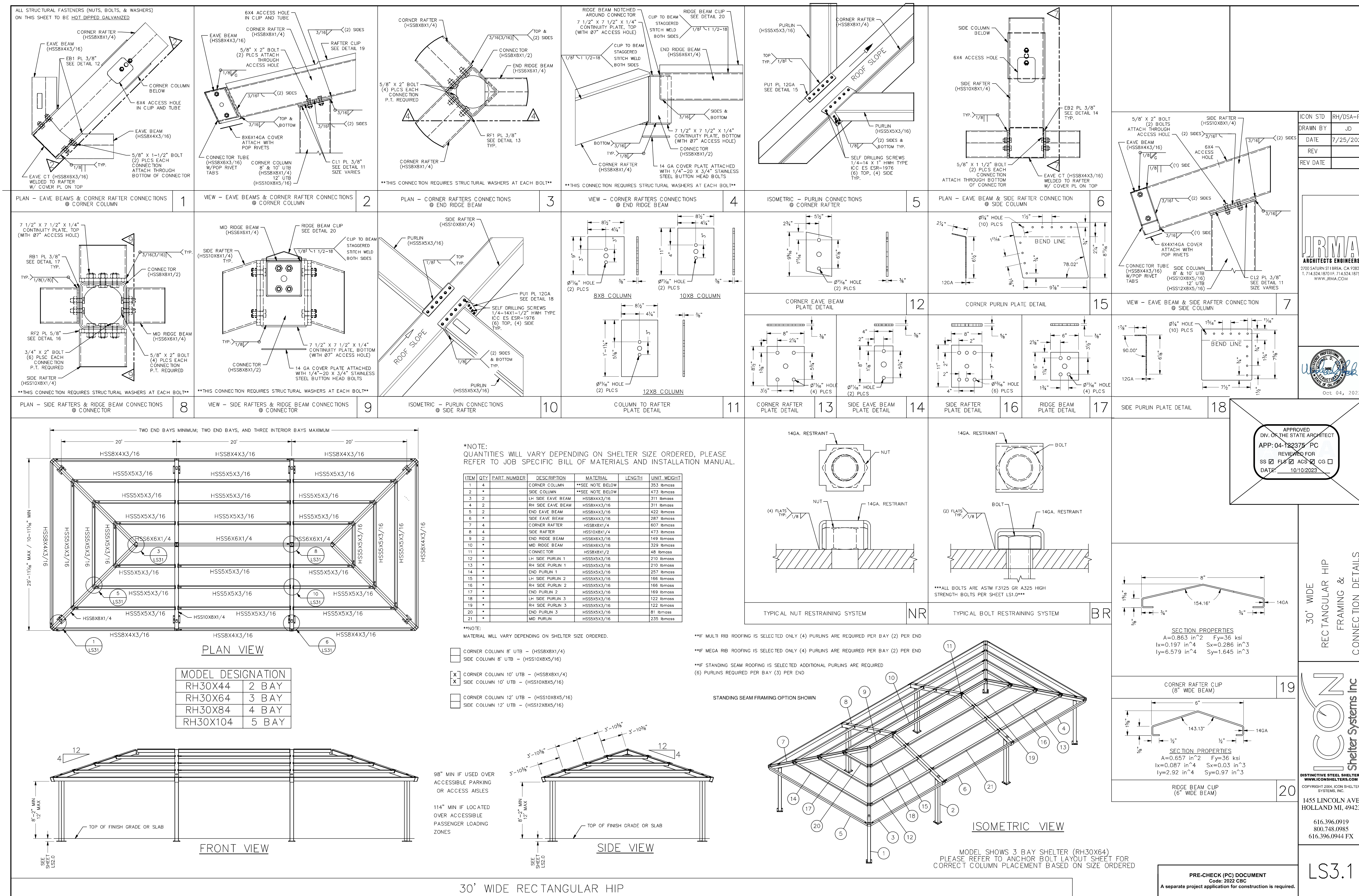
12' height - Corner Columns				12' height - Corner Columns				12' height - Corner Columns			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
84	90	8	6	78	90	6	6	78	90	6	6

12' height - Side Columns				12' height - Side Columns				12' height - Side Columns			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
84	90	8	6	78	90	6	6	78	90	6	6

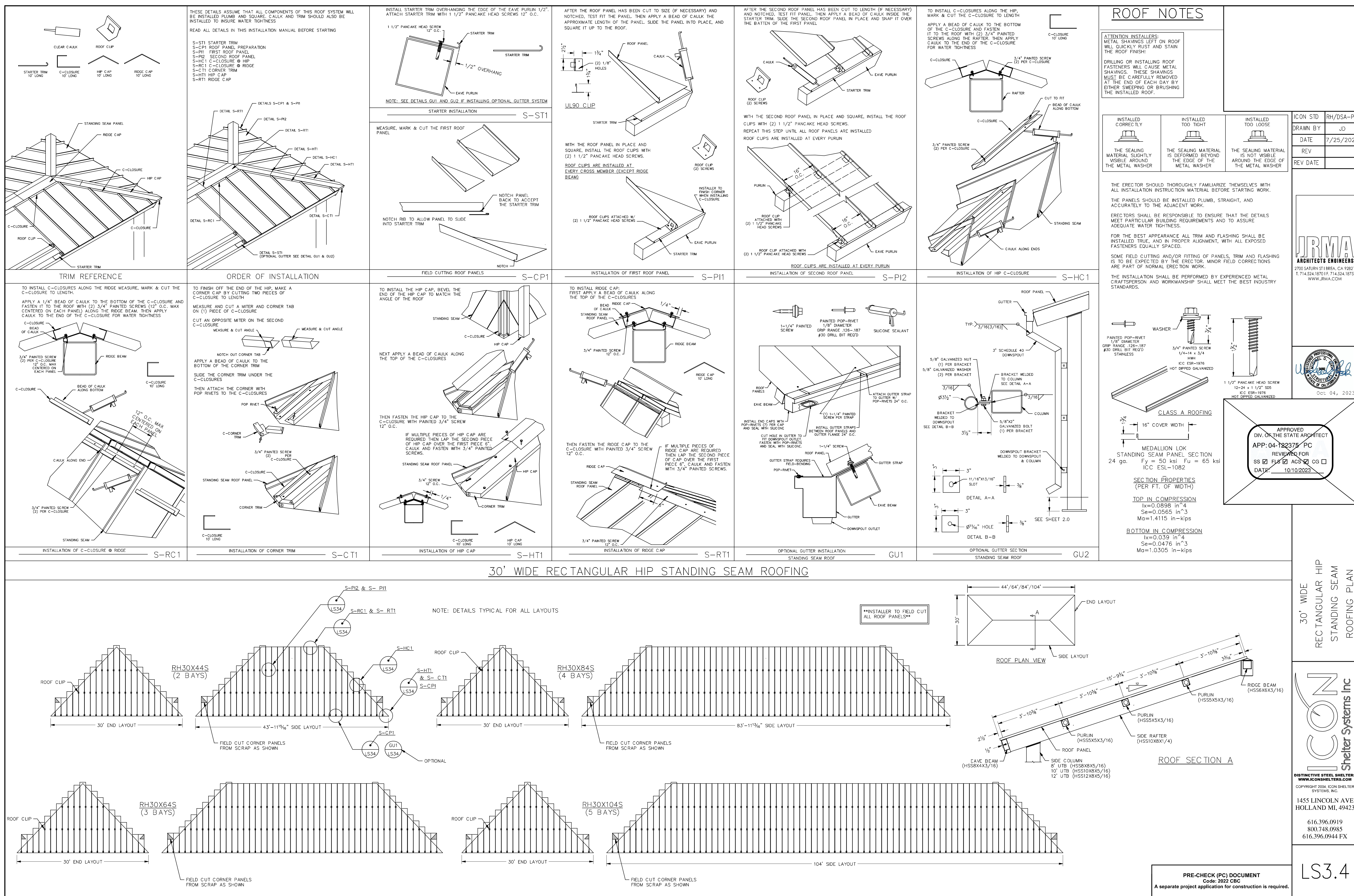
12' Eave - 1500 psf [ ]				12' Eave - 2000 psf [ ]				12' Eave - 3000 psf [ ]			
Soil Class 5 - 1500 psf Bearing				Soil Class 4 - 2000 psf Bearing				Soil Class 3 - 3000 psf Bearing			
Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size	Size (A)	Depth (B)	Vertical Rebar Qty	Rebar Size
84	90	8	6	78	90	6	6	78	90	6	6

















THESE PLANS AND SPECIFICATIONS ARE THE  
PROPERTY OF USA SHADE AND FABRIC  
STRUCTURES AND SHALL NOT BE  
REPRODUCED WITHOUT THEIR WRITTEN



**CORPORATE HEADQUARTERS**  
2580 ESTERS BLVD. SUITE 100  
DFW AIRPORT, TX, 75261  
800-966-5005

**CERTIFICATIONS:**

CLARK COUNTY MANUFACTURER  
CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER:	
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Washington U.S.D.

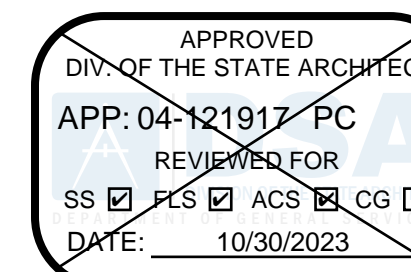
PROJECT NAME:

Westmore Oaks Elementary

## LOCATION:

1504 Fallbrook Avenue  
West Sacramento, CA 95691

**MODEL NUMBER:**



## STRUCTURE TYPE:

STRUCTURE MODEL:	DSA401J-22	
MAX. SIZE:	VARIABLES, SEE JOINED HIP UNIT SHEET	SEE SHEET 9.1-1000
MAX. AREA:	VARIABLES	
MAX. OCCUPANCY:	VARIABLES	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE. SEE PC T-3.0 & PC T-4.0

## JOINED HIP

STRUCTURE MODEL:	DSA401Q-22	
MAX. SIZE:	VARIABLES, SEE QUAD HIP UNIT SHEET	
MAX. AREA:	VARIABLES	SEE SHEET 10.1-100
MAX. OCCUPANCY:	VARIABLES	

FOR DSA 103 TESTING & INSPECTIONS SAMPLE, SEE PC T-3.0 & PC T-4.0

## QUAD HIP



DWG. **UNIT SELECTION**

SHEET	
	T-2.0

REV.



#### GENERAL NOTES

- SPECIAL INSPECTION REQUIREMENTS SHALL FOLLOW THE ATTACHED SAMPLE TEST AND INSPECTION LIST (1 & 151) APPROVED BY DSA. THE SHOP WELDING INSPECTION SHALL INCLUDE WELDING OF ALL STEEL MEMBERS AND IDENTIFICATION OF STEEL THROUGH MILL CERTIFICATE OR MATERIAL TESTING. UNCERTIFIED STEEL SHALL BE TESTED TO THE REQUIREMENTS OF CBC 2022 CHAPTER 17A. THE FIELD SPECIAL INSPECTION SHALL INCLUDE COMPRESSION CYLINDER TESTS FOR THE CONCRETE FOUNDATION.
- STRUCTURE SHALL BE IN THE LOCATION SHOWN ON THE SITE SPECIFIC DSA APPLICATION DRAWING.
- FOUNDATION DESIGN BASED ON CBC 2022, TABLE 1806A.2, SOIL CLASS 5 (ALLOWABLE FOUNDATION PRESSURE 1500 PSF)
- DESIGN PER FOLLOWING CODES: CBC 2022 (CHAPTER 35), ASCE 7-16, AISC 360-16, AISC 341-16, ACI 318-19, ASCE 55-16 & ASCE 19-16

#### STRUCTURAL STEEL

- FABRICATION OF THE STEEL STRUCTURES SHALL BE PERFORMED BY SHADE STRUCTURES OR AN AUTHORIZED LICENSEE. MATERIAL TESTING (OR MILL CERTIFICATES) AND INSPECTION OF WELDING SHALL BE CONDUCTED PER CBC 2022 SECTIONS 1704A, 1705A, 1705A.2, AND TABLE 1705A.2.1.
- ONLY CALIFORNIA LICENSED CONTRACTORS AUTHORIZED BY SHADE STRUCTURES SHALL INSTALL THE SHADE STRUCTURES.
- ALL WORK SHALL CONFORM TO CBC 2022 EDITION, TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR)
- ALL GALVANIZED STEEL TUBE PRODUCTS MANUFACTURED BY ALLIED TUBE & CONDUIT FOR THIS STRUCTURE SHALL BE AND CONFORM TO ASTM A500-15 GRADE C, IN ITS ENTIRETY.  
TYPICAL MECHANICAL PROPERTIES ARE:  
ROUND TUBE GRADE C 46,000 PSI YIELD STRESS MINIMUM / 62,000 PSI TENSILE STRESS MINIMUM
- ALL STRUCTURAL SHAPES SHALL BE COLD FORMED HSS ASTM A500 GRADE C, UNLESS OTHERWISE NOTED. TYPICAL MECHANICAL PROPERTIES ACHIEVED FOR HSS PRODUCTS:  
SQUARE AND RECTANGULAR 50,000 PSI YIELD STRESS / 62,000 PSI TENSILE STRESS  
ROUND PIPE 50,000 PSI YIELD STRESS / 62,000 PSI TENSILE STRESS

- ALL PLATES PRODUCTS SHALL COMPLY WITH ASTM A572 GRADE 50.
- STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH A.I.S.C. SPECIFICATIONS.
- ALL WELDING TO CONFORM WITH AMERICAN WELDING SOCIETY STANDARDS AND SHALL BE INSPECTED BY AN AWS/COW INSPECTOR. AWS D1.1 FOR HOT ROLLED. AWS D1.3 FOR SHEET/COILED FORMED. AWS D1.8 SEISMIC SUPPLEMENT.
- ALL FULL PENETRATION WELD SHALL BE CONTINUOUSLY INSPECTED PER AWS D1.1 & D1.8.

- SHOP CONNECTIONS SHALL BE WELDED UNLESS NOTED OTHERWISE. ALL FILLET WELDS SHALL BE A MINIMUM OF 3/16" E70XX ELECTRODES UNLESS OTHERWISE NOTED. GMAW IS ACCEPTABLE.
- ALL STAINLESS STEEL BOLTS SHALL COMPLY WITH ASTM F-593, YIELD STRENGTH= 65 KSI, TENSILE STRENGTH=100 KSI MINIMUM, ALLOY GROUP 2, CONDITION CW1. ALL NUTS SHALL COMPLY WITH ASTM F-594, ALLOY GROUP 2, CONDITION CW1. REFERRING TO RCSC, ASTM F-593 IS NOT CONSIDERED AS HIGH STRENGTH BOLTS. BOLTS, ITEM 11, SHALL BE TIGHTENED TO A SNUG TIGHT CONDITION (ST).
- ALL HIGH STRENGTH BOLTS SHALL COMPLY WITH ASTM F3125 GRADE A325 N (GALVANIZED). ALL NUTS SHALL COMPLY WITH ASTM A563DH, AND WASHERS SHALL COMPLY WITH ASTM F436, HIGH STRENGTH BOLTS. ITEM 15, SHALL BE TIGHTENED TO A SNUG TIGHT CONDITION (ST) WITH DOUBLE NUTS. ALL NUTS SHALL BE LUBRICATED WITH A LUBRICANT CONTAINING A VISIBLE DYE SO A VISUAL CHECK CAN BE MADE FOR THE LUBRICANT AT THE TIME OF THE FIELD INSTALLATION. WASHERS SHALL BE GALVANIZED PER ASTM F2329.

- ALL STRUCTURAL STEEL ITEMS FROM NOTE 5) SHALL BE POWDER COATED WITH ONE SHOP COAT (2.5 MILS MIN.) OF ZINC-RICH PRIMER, UNDERCOAT, AND FINISH COAT, OR EQUIVALENT PAINT SYSTEM. THIS COAT IS A WEATHER RESISTANT POWDER COATING BASED ON POLYESTER (TIG) (MANUFACTURED BY SHERWIN WILLIAMS, ASKO NOBEL, PPG OR TIGER DRYLAC). TO ACHIEVE OPTIMUM ADHESION, IT IS RECOMMENDED THAT THE PROPER TREATMENT AND DRYING TAKE PLACE BEFORE COATING. POLYESTER POWDER (TIG) SPECIFICATIONS SHALL BE AS FOLLOWS:  
- PENCIL HARDNESS (ASTM D-3363) - HUMIDITY (ASTM D-2247)  
- SOLVENT RESISTANCE (PCI METHOD) - 50 DBL RUBS SL SOFTNESS.

- ALL STEEL ROUND TUBING (ITEMS FROM NOTE 4) SHALL BE TRIPLE COATED FOR RUST PROTECTION USING THE IN-LINE ELECTROPLATING COAT PROCESS. TUBING SHALL BE INTERNALLY COATED WITH ZINC AND ORGANIC COATINGS TO PREVENT CORROSION AS MANUFACTURED BY ALLIED TUBE & CONDUIT.
- ALL EXPOSED STEEL FASTENERS SHALL BE STAINLESS STEEL (TYPE 304 MINIMUM), HOT DIP GALVANIZED (ASTM A153, CLASS D MINIMUM OR ASTM F2329) AS APPLICABLE, OR PROTECTED WITH CORROSION PREVENTIVE COATING THAT DEMONSTRATED NO MORE THAN 2% OF RED RUST IN MINIMUM 1,000 HOURS OF EXPOSURE IN SALT SPRAY TEST PER ASTM B117. ZINC-PLATED FASTENERS DO NOT COMPLY WITH THIS REQUIREMENT.

- CONCRETE SHALL BE SAMPLED AND TESTED PER CBC 2022 SECTION 1903A & SHALL BE INSPECTED PER SECTION 1903A.
- CONCRETE TO BE F<sub>cy</sub>= 4500 PSI, TYPE V CEMENT PLUS POZZOLAN OR SLAG CEMENT, MAXIMUM WATER/CEMENT RATIO OF 0.45, PER ACI 318-19 CHAPTER 19. (NO ADMIXTURES CONTAINING CALCIUM CHLORIDE WILL BE USED.) REINFORCING STEEL SHALL CONFORM TO ASTM A-615 GRADE 60 AND TO BE F<sub>yt</sub>= 60000 PSI, MIN. GR. 60. ALSO COATED ACCORDING TO ASTM A707/ A707M. STANDARD SPECIFICATION FOR ZINC-COATING (GALVANIZED) STEEL BARS FOR CONCRETE REINFORCEMENT.

- ALL ANCHOR BOLTS SET IN NEW CONCRETE (WHEN APPLICABLE) SHALL COMPLY WITH ASTM F-1554 GRADE 36 (GALVANIZED PER ASTM A153, CLASS D MINIMUM OR ASTM F2329).  
ANCHOR BOLTS DIAMETER NEEDS TO BE AS FOLLOWS:  
A) ANCHOR BOLT #1 1/4"

- CERTIFIED MILL TEST REPORTS ARE TO BE PROVIDED FOR EACH SHIPMENT OF REINFORCEMENT.
- ALL NON-SHRINK GROUT SHALL HAVE A MINIMUM 28 DAYS COMPRESSIVE STRENGTH OF 5000 PSI, AND SHALL COMPLY THE REQUIREMENTS OF ASTM C109, ASTM C939, ASTM C1090, ASTM C1107, WHEN APPLICABLE.
- CONCRETE EXPOSED TO FREEZING-AND-THAWING CYCLES SHALL BE AIR ENTRAINED PER ACI 318 SECTION 19.3.3.

- FABRIC SHALL BE MANUFACTURED BY MULTIKINT LTD., WHICH MEETS THE SPECIFICATIONS LISTED ON PAGE 2000, AND SHALL BE FABRICATED FROM POLYETHYLENE MATERIALS. MINIMUM SEAM LENGTH 3/4".
- THE FABRIC SHALL RETAIN 80% OF ITS TENSILE AND TEARING STRENGTH AFTER ULTRAVIOLET EXPOSURE PER ASTM G53 USING A 313 NM LIGHT SOURCE FOR 500 HOURS WHILE MOISTENED FOR 1 HOUR EVERY 12 HOURS.

- PROVIDE CERTIFICATION BY MANUFACTURER AND STATE FIRE MARSHAL TO SCHOOL'S DISTRICT INSPECTOR OF RECORD AT SITE SPECIFIC INSTALLATION. COPY OF FIRE CERTIFICATION SHALL BE SENT TO DSA.
- FABRIC SHALL REQUIRE ANNUAL INSPECTION AND MAINTENANCE BY THE DISTRICT. FIRE TEST ON FABRIC: NFPA 701 TEST 2 AND ASTM E 84 EXTENDED 30 MINUTES TEST. FLAME SPREAD INDEX (FSI): 10. SMOKE DEVELOPED INDEX (SDI): 50. FABRIC IS ACCEPTABLE FOR USE IN WILDLIFE URBAN INTERFACE AREA.

- FABRIC TOP NEEDS TO BE REMOVED IF SNOW EXCEEDING 5 PSF ARE ANTICIPATED. FABRIC TOP NEEDS TO BE REMOVED IF WINDS EXCEEDING 115 MPH ARE ANTICIPATED.
- A VISUAL INSPECTION LOOKING FOR TEAR AND ABNORMAL WEAR IN FABRIC MATERIAL AND THREAD IS REQUIRED PRIOR TO RE-INSTALLATION. USA SHADE & FABRIC STRUCTURES SHALL BE NOTIFIED IF SIGNIFICANT DAMAGE IS PRESENT BEFORE RE-INSTALLATION.

- FOR FABRIC ATTACHMENT USE 3/8" 7x19 GALV. CABLE PER ASTM A1023/A1023M, WITH A BREAKING STRENGTH VALUE OF 14,400 LBS. CABLE SHALL BE TENSIONED TO 300 LBS MINIMUM AND 500 LBS MAXIMUM. THE MAXIMUM CALCULATED CABLE ALLOWABLE CAPACITY IS 54=4909 LB.
- CABLES SHALL BE FED THROUGH THE FABRIC SLEEVES AROUND THE PERIMETER OF THE CANOPY AND TENSIONED UNTIL THE FABRIC PANELS (DESIGNED PURPOSELY UNDERSIZED) REACH A TAUT APPEARANCE. ANY LONG TERM CABLE SAG SHALL BE MINIMIZED DURING THE MAINTENANCE RE-TIGHTING VISITS AS REQUIRED.

- MAXIMUM OCCUPANT LOAD (PER CBC 2022 TABLE 1604A.5)  
K-12 250 PERSONS  
PUBLIC ASSEMBLY: 300 PERSONS  
EDUCATIONAL OCCUPANCIES  
ABOVE 12TH GRADE: 500 PERSONS

#### CBC PC DESIGN NOTES

- BUILDING CODE N/A  
FLOOR LIVE LOAD 5 PSF  
ROOF LIVE LOAD 5 PSF
- ALLOWABLE SOIL PRESSURE:  
DL + LL (CONC FTG) 1500 PSF  
DL + LL + SEISMIC (CONC FTG) 1500 PSF  
LATERAL BEARING DESIGN VALUE 100 PSF/FT BELOW NATURAL GRADE, PER TABLE 1806A.2
- TWO TIMES THE TABULAR VALUE IS USED (200 PSF/FT)  
PER CBC SECTION 1806A.3.4.  
ALLOWABLE PIER FRICTIONAL RESISTANCE 250 PSF MAXIMUM  
BASED ON SECTION 1810A.3.3.1.4 (ONE-SIXTH OF THE BEARING VALUE).  
UPLIFT FRICTIONAL RESISTANCE HAVE A SAFETY FACTOR OF 3.

- ROOF SNOW LOAD 5 PSF  
ICE LOAD ZERO PSF  
FLOOD HAZARD AREA ZONE X
- WHEN A SITE SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEER IS NEEDED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED IN THE PC ARE STILL APPLICABLE.

- WIND DESIGN DIRECTIONAL PROCEDURE: ASCE 7-16, SECTION 27.3.2  
NOTE: WIND DESIGN IS LIMITED TO UNOBSTRUCTED CLEAR FLOW CONDITION  
- BASIC DESIGN WIND SPEED (3 SEC GUST) V 115 MPH  
- ASD WIND LOAD (CBC 2022 SEC. 1603A.1.4) V 90 MPH  
- WIND EXPOSURE FACTOR C 1  
- TOPOGRAPHIC FACTOR Kzt 1  
- RISK CATEGORY II 1  
- VELOCITY PRESSURE EXPOSURE COEFFICIENT Kq 0.88  
- VELOCITY PRESSURE qz 25.32 PSF

- SEISMIC DESIGN:  
SITE CLASS D  
NOTE: UNLESS A SITE-SPECIFIC GROUND MOTION HAZARD ANALYSIS IS PERFORMED, THE SM1 VALUE INCREASED BY 50% SHALL BE LESS THAN THE DESIGN CRITERIA STATED HEREIN.

- SPECTRAL RESPONSE COEFFICIENTS  
SDS 3.00g  
SDS 2.00  
SD1 1.39

- LATERAL FORCE RESISTING SYSTEM G.2 ORDINARY CANTILEVERED COLUMN SYSTEM

- SEISMIC IMPORTANCE FACTOR Ie 1.0  
-DESIGN BASE SHEAR AT BASE V 6866 LB  
-SEISMIC RESPONSE COEFFICIENTS Cs 1.6  
-RESPONSE MODIFICATION FACTOR R 1.25  
-ANALYSIS PROCEDURE E  
-RISK CATEGORY IV 1.5  
-SEISMIC DESIGN CATEGORY D  
-SITE COEFFICIENT CATEGORY 1.2  
-REUNDANCY FACTOR p 1.3

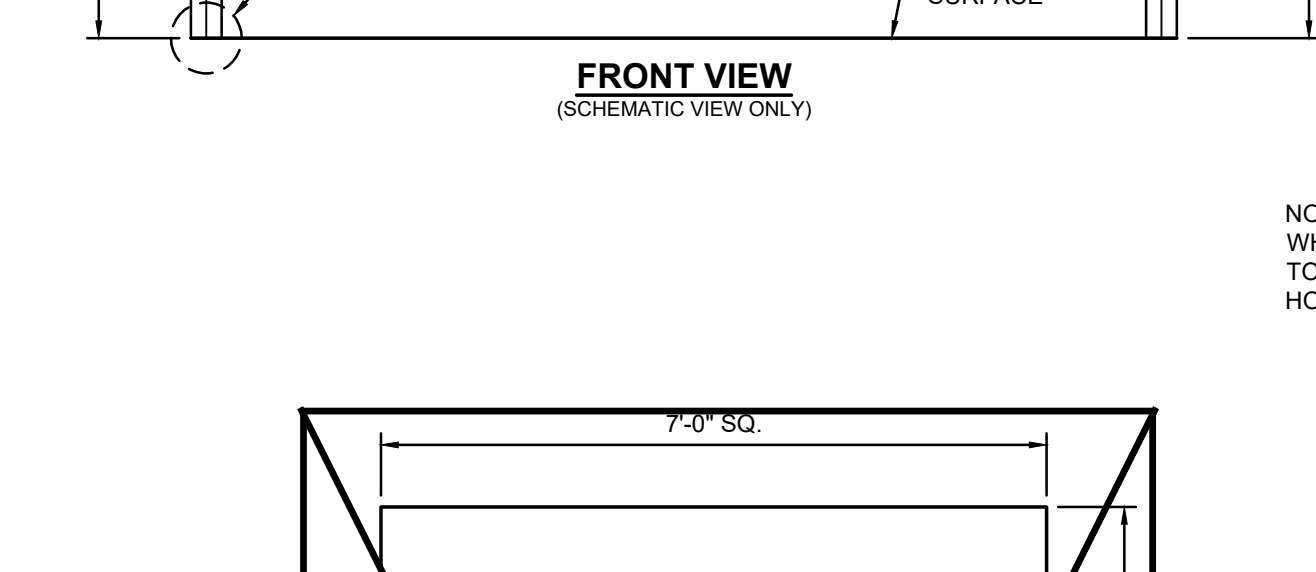
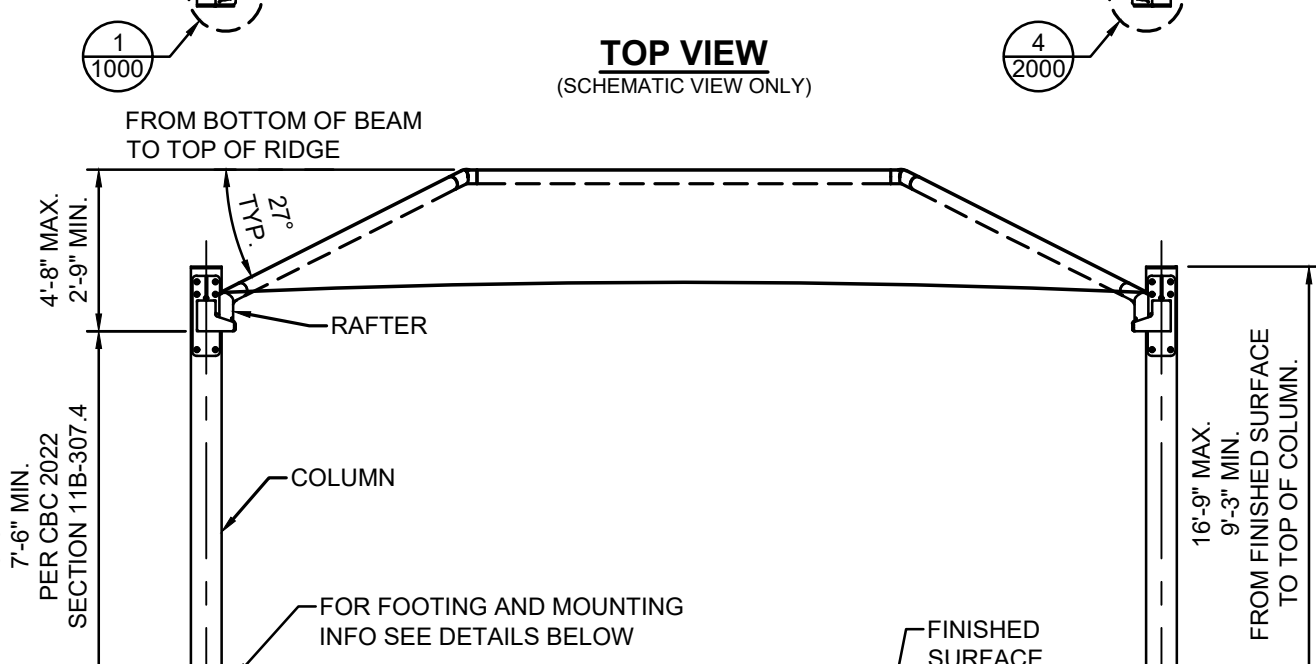
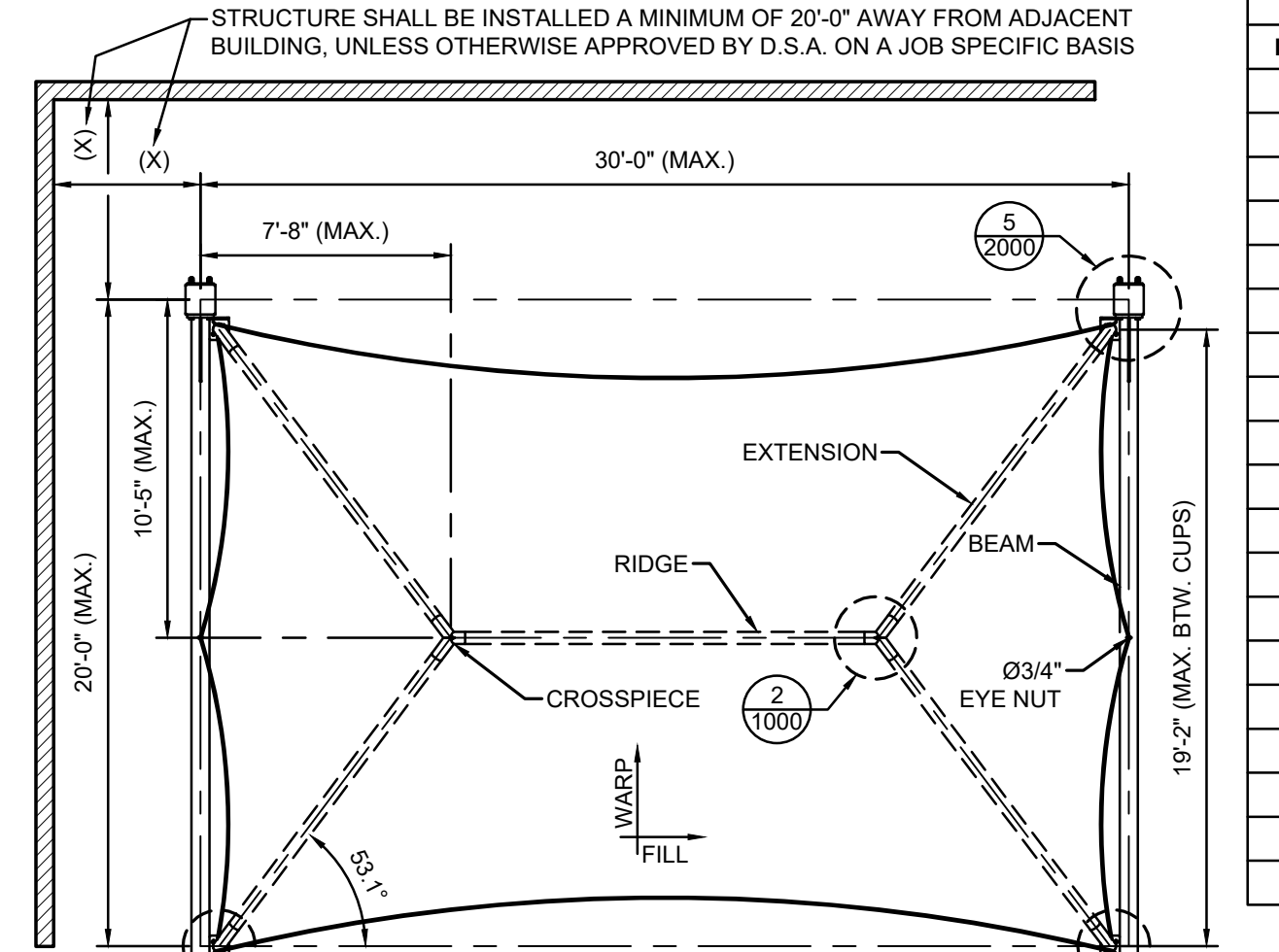
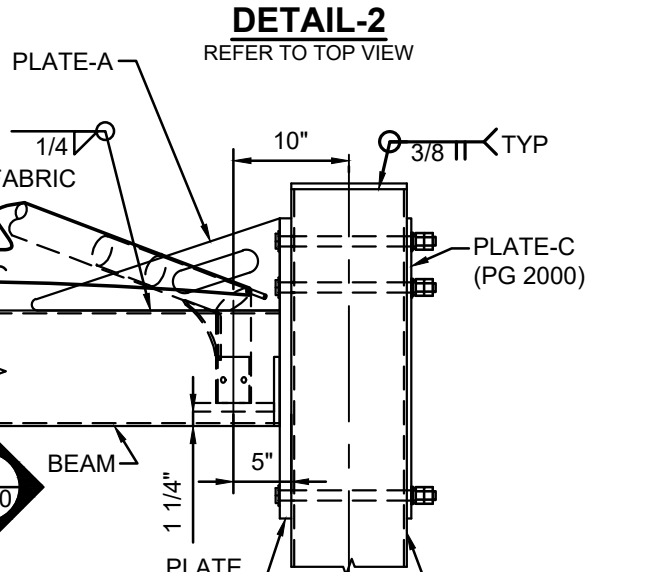
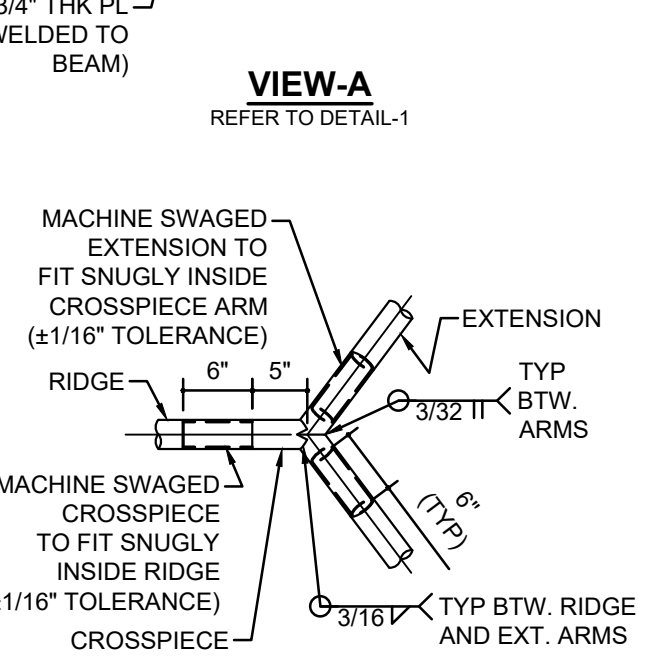
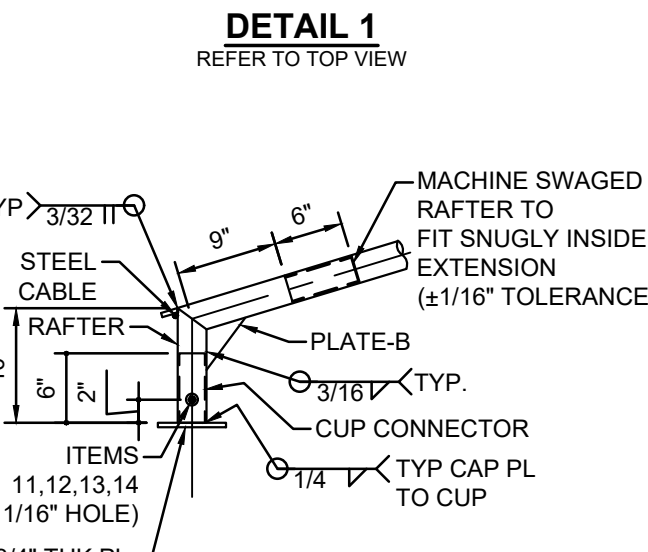
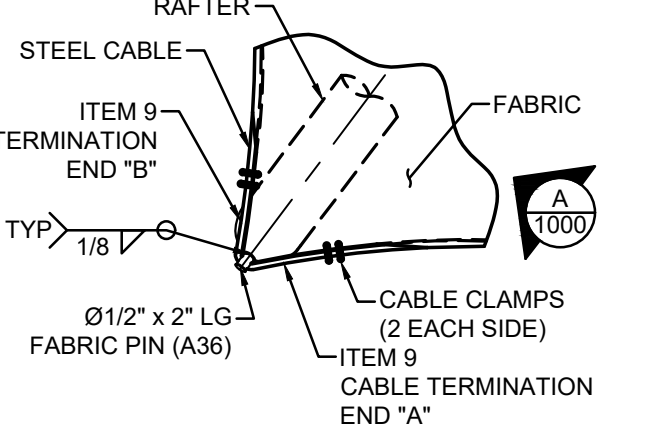
- GEHAZARD REPORT IS NOT REQUIRED FOR OPEN FABRIC STRUCTURES 1,600 SQ FT OR LESS COMPLYING WITH THE REQUIREMENTS OF IR A-4 SECTION 3.1.1. OPEN FABRIC SHADE STRUCTURES GREATER THAN 1,600 SQUARE FEET UP TO A MAXIMUM OF 4,000 SQUARE FEET AND COMPLYING WITH THE REQUIREMENTS NOTED IN IR A-4 SECTION 3.1.1 DO NOT REQUIRE A GEHAZARD REPORT PROVIDED A GEOTECHNICAL REPORT INDICATES THAT NO LIQUEFACTION POTENTIAL EXISTS.

- ARCHITECT OF RECORD TO DETERMINE IF SPECIFIC SITE IS IN GEOLOGIC HAZARD ZONE. GEHAZARD REPORT REQUIREMENTS PER DSA IR A-4.

- PC OPTIONS SHALL NOT INCLUDE LIQUEFIABLE SOIL (EXCEPTION: OPEN FABRIC SHADE STRUCTURES 1,600 SQUARE FEET OR LESS COMPLYING WITH REQUIREMENTS OF IR A-4 SECTION 3.1.1). IF STRUCTURE IS LOCATED IN AN AREA WITH LIQUEFIABLE SOIL OR SITE CLASS F, OVER-THE-COUNTER SUBMITTAL IS NOT ALLOWED AND REGULAR PROJECT SUBMITTAL IS REQUIRED. IF SITE IS NOT IN A MAPPED LIQUEFACTION HAZARD ZONE, IT MAY BE PRESUMED THAT NO LIQUEFACTION HAZARD EXISTS ON THAT SITE UNLESS A SITE-SPECIFIC GEOTECHNICAL REPORT IDENTIFIES SUCH HAZARD.

- MINIMUM FOUNDATION SETBACK LIMIT IN ADJACENT SLOPE: THE DEPTH OF REQUIRED PIER EMBEDMENT SHALL START FROM AN ELEVATION THAT CORRESPONDS WITH A HORIZONTAL CLEAR DISTANCE OF 17'-6" THAT INTERSECT WITH THE SLOPE (DAYLIGHTING). IF SETBACK LIMITS ARE SMALLER THAN CBC REQUIREMENTS, A SITE-SPECIFIC SOILS REPORT IS REQUIRED.

- MINIMUM CLASS 2 PROJECT INSPECTOR REQUIRED.



LIST OF MATERIALS			
ITEM	QTY	DESCRIPTION	MATERIAL
1	2	COLUMN	HSS 10 x 10 x 0.625
2A	1	BEAM LEFT	HSS 10 x 6 x 0.375
2B	1	BEAM RIGHT	HSS 10 x 6 x 0.375
3	4	CUP CONNECTOR (6" LG)	HSS 4.0 x 0.25
4	4	RAFTER (GALVANIZED STEEL TUBE)	4.50 GA 7 RD. TUBE (HSS 4.5 x 0.188)
5	4	EXTENSION (GALVANIZED STEEL TUBE)	4.50 GA 7 RD. TUBE (HSS 4.5 x 0.188)
6	2	CROSSPIECE (GALVANIZED STEEL TUBE)	4.50 GA 7 RD. TUBE (HSS 4.5 x 0.188)
7	1	RIDGE	4.50 GA 7 RD. TUBE (HSS 4.5 x 0.188)
8	1	FABRIC TOP	FR COLOURSHADE 1901F5
9	1	Ø3/8" CABLE	GALVANIZED STEEL
10	4	Ø3/8" CABLE CLAMP	GALVANIZED STEEL
11	4	Ø5/8"-11NC x 6" HEX BOLT (ST)	316 SS
12	4	Ø5/8"-11NC HEX NUT	316 SS
13	8	Ø5/8" FLAT WASHER	316 SS
14	4	Ø5/8" SPLIT LOCK WASHER	316 SS
15	12	Ø1"-8NC x 14 1/2" HEX BOLT (ST)	ASTM F3125 GRADE A325, GALVANIZED
16	24	Ø1"-8NC HEX NUT	ASTM A563 GALVANIZED
17	12	Ø1" SPLIT LOCK WASHER	ASTM F436 GALVANIZED
18	24	Ø1" FLAT WASHER	ASTM F436 GALVANIZED

THESE PLANS AND SPECIFICATIONS ARE THE PROPERTY OF USA SHADE AND FABRIC STRUCTURES AND SHALL NOT BE REPRODUCED WITHOUT THEIR WRITTEN PERMISSION.



CORPORATE HEADQUARTERS  
2580 ESTERS BLVD, SUITE 100  
DFW AIRPORT, TX, 75261  
800-966-5005

#### CERTIFICATIONS:

IAS CERTIFICATION No: F4-428  
CLARK COUNTY MANUFACTURER  
CERTIFICATION NUMBER (NEVADA): 355

#### CUSTOMER:

Washington U.S.D.

#### PROJECT NAME:

Westmore Oaks Elementary

#### LOCATION:

1504 Fallbrook Avenue  
West Sacramento, CA 95691

#### MODEL NUMBER:

DSA2022030-22

#### STRUCTURE TYPE: FULL CANTILEVER HIP SINGLE - DSA

SIZE: MAXIMUM  
20' x 30' x 15' MAX.

SCALE: NONE

DRAWING SIZE: D

#### PRE-CHECK (PC) DOCUMENT Code: 2022 CBC A separate project application for construction is required.

Eng. By: HH 12/01/22

Design By: OS 12/01/22

Approved By: MB 12/01/22

#### DRAWING DESCRIPTION:

#### PRODUCT INFORMATION

DWG. DSA2022030-22

SHEET 11.1-1000

REV. NC

10/18/2023





### 190/F5 Fire rated specifications

#### Standard range

Revision 0 28-Oct-12

Colour	Shade %	UV Block %	Average GSM	Average Warp break strength kgs	Average Elongation %	Average Weft break strength kgs	Average Elongation %	Average Burst Kpa	Average Burst to Mass ratio
Desert Sand	80	92	185	50	40	72	73	156	0.84
Blue	80	85	185	50	40	72	73	156	0.84
Brown	85		185	50	40	72	73	156	0.84
Green	80	85	185	50	40	72	73	156	0.84
Red	80	86	185	50	40	72	73	156	0.84
Silver	80	81	185	50	40	72	73	156	0.84
Terracotta	75	82	185	50	40	72	73	156	0.84
Yellow	80	89	185	50	40	72	73	156	0.84
			110 LB			159 LB			3258 PSF

#### Notes:

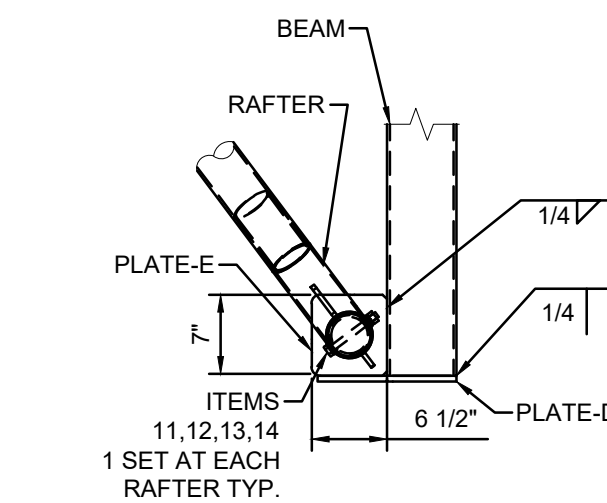
190/F5 conforms to The California State Fire Marshal Title 19 Test for Small scale Fabrics  
Tear tests are done using a 50mm wide strip and a cross head speed of 500mm/min

This report has been compiled using the mean results from all tests conducted on the given sample by our Quality Control Laboratory. The information provided is considered to be a good reflection of the relevant properties of the fabric tested. These results must only be used as an indication of the quality and characteristics of the fabric tested.  
Company cannot be held responsible or liable in any way whatsoever should this information differ to that of a registered testing institution.

Deon Joubert  
General Manager - Multiknit (Pty) Ltd

Tommy Rogers  
Managing Director - Multiknit (Pty) Ltd

CONVERSION TO  
IMPERIAL UNITS:  
185 GSM = .0378 psf  
50 KGS = 110 lb  
72 KGS = 159 lb  
156 Kpa = 3258 psf



DETAIL-4  
REFER TO TOP VIEW PG 1000

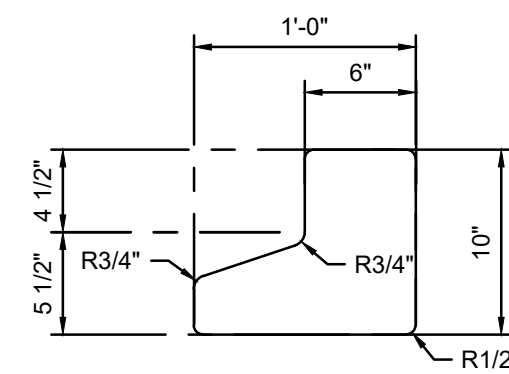


PLATE-D  
REFER TO DETAIL-4  
(1/2 THK PLATE)  
(TYP. END OF BEAM)  
(ASTZ GR 50)

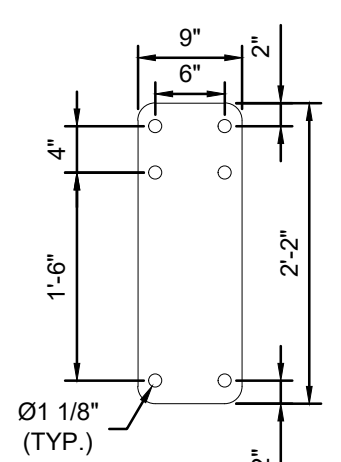
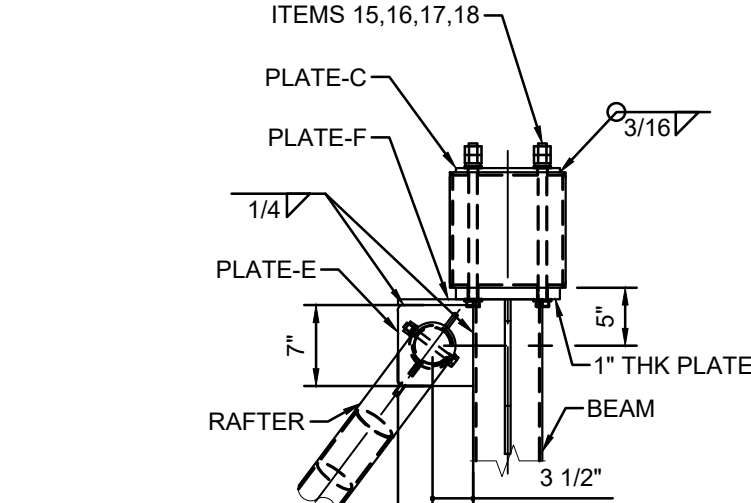
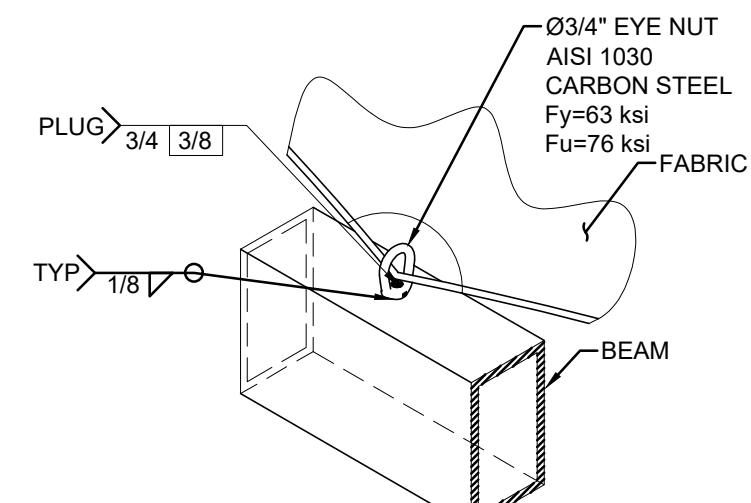


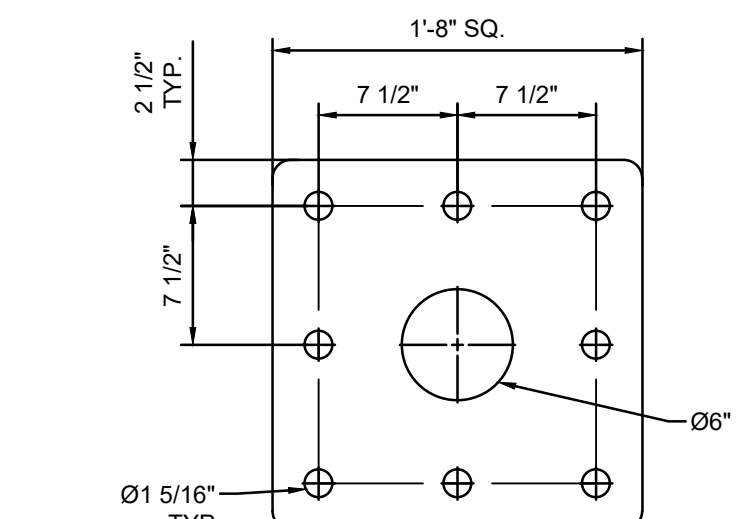
PLATE-C  
REFER TO DETAIL-4  
(3/8 THK PLATE)  
(ASTZ GR 50)



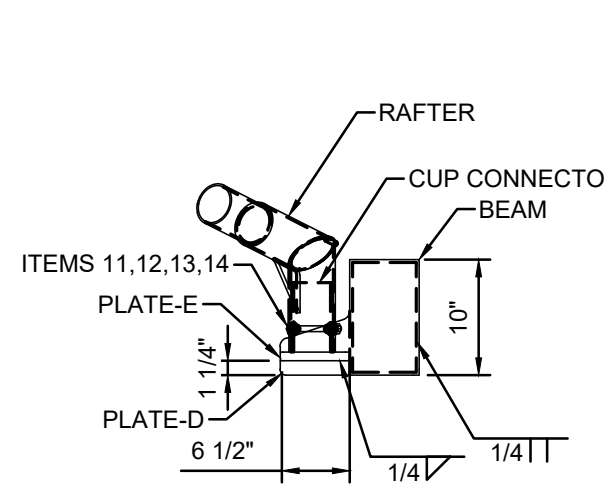
DETAIL-5  
REFER TO TOP VIEW PG 1000



ISOMETRIC VIEW 6  
REFER TO SIDE VIEW PG 1000



BASE PLATE  
(1 1/4 THK)  
(TYP. FOR RBP COLUMNS)



VIEW C  
REFER TO DETAIL-4

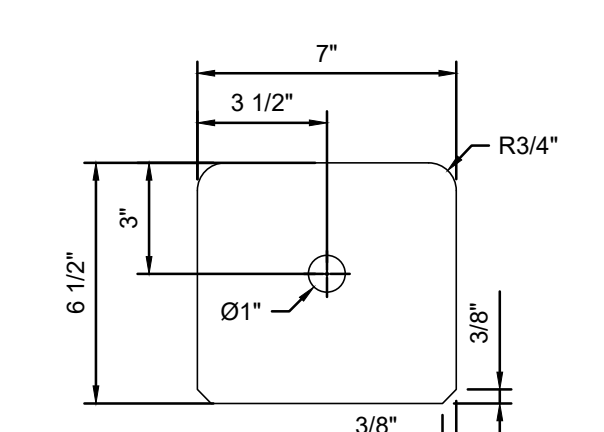


PLATE-E  
REFER TO VIEW-B PG 2000  
(3/4 THK PLATE)  
(ASTZ GR 50)

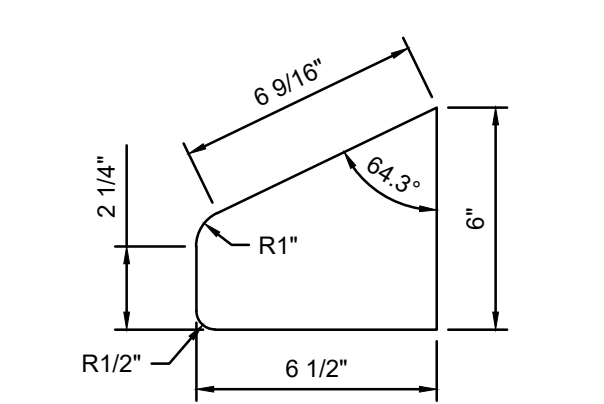
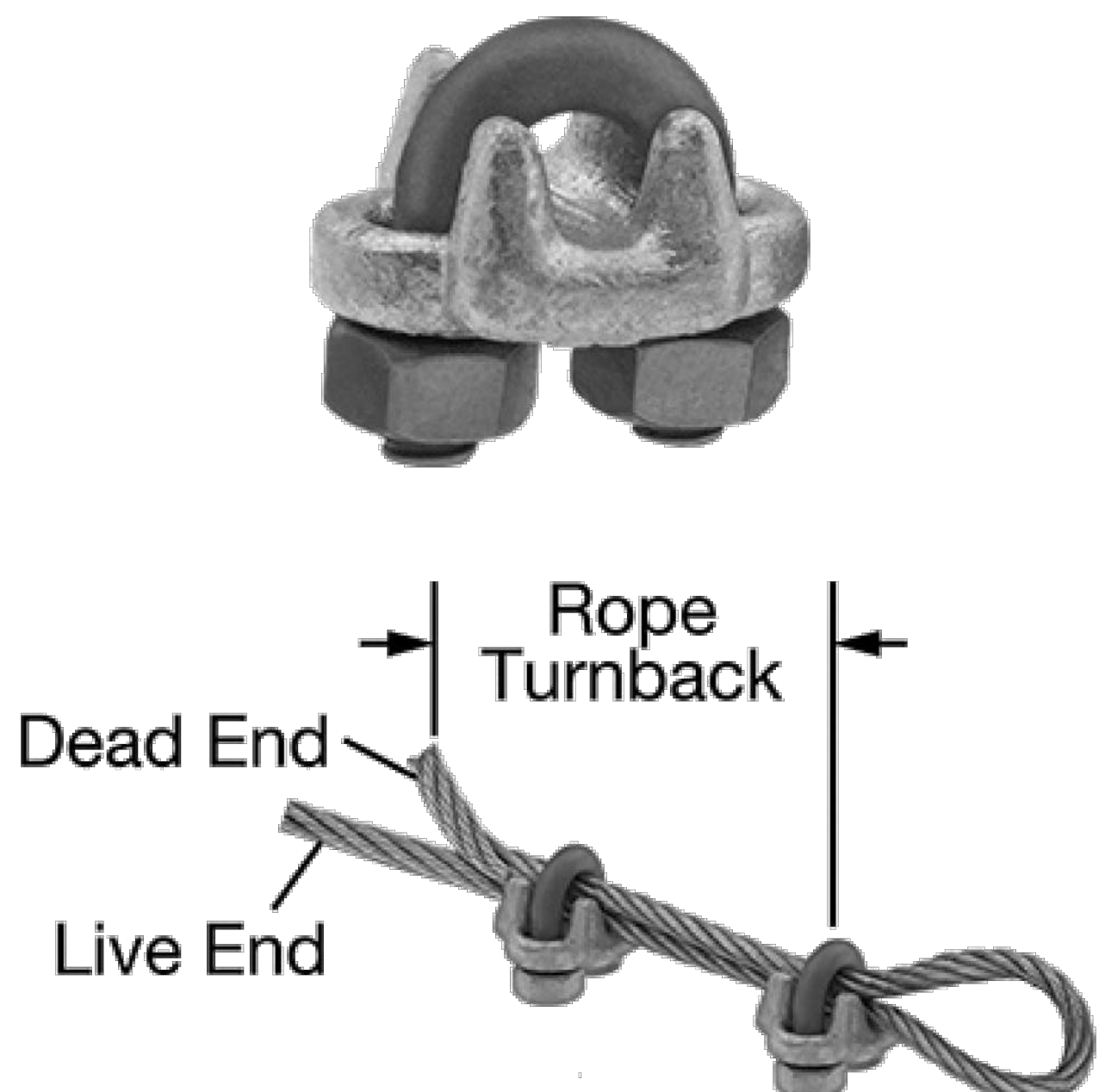
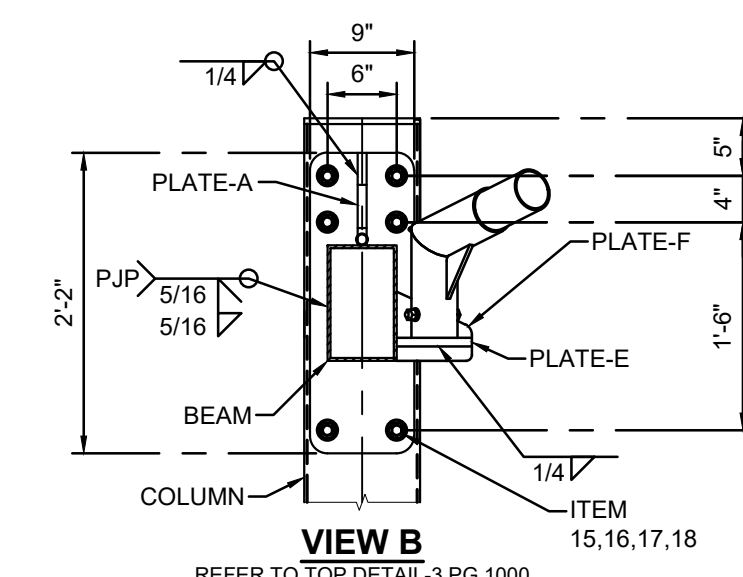


PLATE-F  
REFER TO VIEW-C PG 2000  
(1/2 THK PLATE)  
(ASTZ GR 50)



#### FORGED WIRE ROPE CLAMP

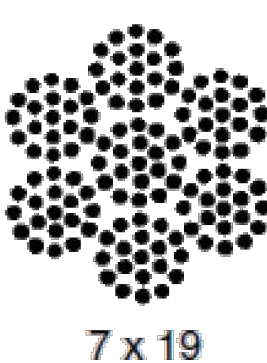
FITTING TYPE ROPE CLAMP  
FABRICATION: FORGED  
MATERIAL: GALVANIZED STEEL  
FOR WIRE ROPE DIAMETER 3/8"  
NUMBER OF CLAMPS REQUIRED: 2  
ROPE TURNBACK: 6 1/2"  
FOR WIRE ROPE CONSTRUCTION 7 x 19  
ATTACHMENT TYPE: LOOP  
CLAMP WIDTH 2", HEIGHT 1 15/16", THICKNESS 1 11/16"  
REQUIRED INSTALLATION TOOL: TORQUE WRENCH  
REQUIRED TORQUE 45 FT-LBS.  
CAPACITY 80% OF THE ROPE'S CAPACITY  
SPECIFICATIONS MET ASME B30.26, FED. SPEC. FF-C-450



### Aircraft Cable

Preformed, made in accordance with commercial specifications military and federal specification rope available.

Carbon Steel (Aircraft Cable) - Galvanized cable has the highest strength and greatest fatigue life of the materials offered. It has good to fair corrosion resistance in rural to industrial atmosphere environments. This material is most widely used for small diameter cables. Tin over galvanized cable offers greater corrosion resistance and reduced friction over pulleys.



7 x 19		Galvanized Min. Breaking Strengths (lbs)
Dia. (In)	Approx. Wt 1000 Ft/lbs	
3/32	17.	1,000
1/8	29.	2,000
5/32	45.	2,800
3/16	65.	4,200
7/32	86.	5,600
1/4	110.	7,000
9/32	139.	8,000
5/16	173.	9,800
3/8	243.	14,400



Product Marketed by:  
MULTIKNIT (PTY) LTD  
BOX 786 WHITE RIVER 1240  
MPUMALANGA SOUTH AFRICA.  
Issue Date : 05/06/2023  
Expiration Date : 06/30/2024

This product meets the minimum requirements of flame resistance established by the California State Fire Marshal for products identified in Section 13115, California Health and Safety Code. The scope of the approved use of this product is provided in the current edition of the CALIFORNIA APPROVED LIST OF FLAME RETARDANT CHEMICALS AND FABRICS, GENERAL AND LIMITED APPLICATIONS CONCERNS published by the California State Fire Marshal.

Issued By: Courtney Walker  
Fire Engineering License Manager  
Fire Engineering & Investigations Division  
Reviewed and Approved By: Patricia Setter  
Deputy State Fire Marshal III  
Fire Engineering & Investigations Division

OFFICE OF THE STATE FIRE MARSHAL  
Please visit calfire.gov/motus.org for more information on Licensing and Permitting with CAL FIRE

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USASHADE & Fabric Structures®  
CORPORATE HEADQUARTERS  
2580 ESTERS BLVD, SUITE 100  
DFW AIRPORT, TX, 75261  
800-966-5005

CERTIFICATIONS:  
IAS CERTIFICATION No: FA-428  
CLARK COUNTY MANUFACTURER  
CERTIFICATION NUMBER (NEVADA): 355

CUSTOMER:  
Washington U.S.D.

PROJECT NAME:  
Westmore Oaks Elementary

LOCATION:  
1504 Fallbrook Avenue  
West Sacramento, CA 95691

MODEL NUMBER:  
DSA2022030-22

STRUCTURE TYPE:  
FULL CANTILEVER HIP  
SINGLE - DSA

SIZE: MAXIMUM  
20' x 30' x 15'e MAX.

SCALE: NONE

DRAWING SIZE: D

PRE-CHECK (PC)  
DOCUMENT  
Code : 2022 CBC  
A separate project application for construction is required.

Eng. By : HH 12/01/22

Design By : OS 12/01/22

Approved By : MB 12/01/22

DRAWING DESCRIPTION:  
SPECIFICATIONS

DWG.  
DSA2022030-22

SHEET  
11.2-2000

REV.  
NC